

SECTION F ESTIMATE OF QUANTITIES

BID ITEM NUMBER	ITEM	QUANTITY	UNIT
009E3320	Checker	Lump Sum	LS
120E6200	Water for Granular Material	363.1	MGal
260E1010	Base Course	2,772.1	Ton
260E1030	Base Course, Salvaged	2,285.0	Ton
260E2010	Gravel Cushion	2,078.8	Ton
260E2030	Gravel Cushion, Salvaged	21,803.0	Ton
260E3500	Temporary Gravel Surfacing	1,371.0	Ton
320E1200	Asphalt Concrete Composite	3,673.4	Ton
380E0050	8" Nonreinforced PCC Pavement	60,488.6	SqYd
380E0800	PCC Shoulder Pavement	751.8	SqYd
380E3020	6" PCC Driveway Pavement	292.8	SqYd
380E3040	8" PCC Driveway Pavement	4,137.5	SqYd
380E6000	Dowel Bar	42,175	Each
380E6110	Insert Steel Bar in PCC Pavement	260	Each
831E0300	Reinforcement Fabric (MSE)	6,390	SqYd

SURFACING THICKNESS DIMENSIONS

Plans tonnage will be applied even though the thickness may vary from that shown on the plans.

At those locations where material must be placed to achieve a required elevation, plans tonnage may be varied to achieve the required elevation.

SAWING IN EXISTING SURFACING

Where new Portland Cement Concrete Pavement (PCCP) or new asphalt concrete is placed adjacent to existing asphalt concrete or PCCP, the existing pavement shall be sawed full depth to a true line with a vertical face. No separate payment shall be made for sawing.

CHECKING SPREAD RATES

The Contractor will be responsible for checking the Base Course, Gravel Cushion, Salvaged, Gravel Cushion and Asphalt Concrete Composite spread rates and taking the weigh delivery tickets as the surfacing material arrives on the project and is placed onto the roadway.

The Contractor will compute the required spread rates for each typical surfacing section and create a spread chart prior to the start of material delivery and placement. The Engineer will review and check the Contractor's calculations and spread charts. The station to station spread will be written on each ticket as the surfacing material is delivered to the roadway.

At the end of each day's shift, the Contractor will verify the following:

- All tickets are present and accounted for,
- The quantity summary for each item is calculated.
- The amount of material wasted if any, •
- Each day's ticket summary is marked with the corresponding 'computed by',
- The ticket summary is initialed and certified that the delivered and placed quantity is correct.

All daily tickets and the summary by item will be given to the Engineer no later than the following morning.

If the checker is not properly and accurately performing the required duties, the Contractor will correct the problem or replace the checker with an individual capable of performing the duties to the satisfaction of the Engineer. Failure to do so will result in suspension of the work.

The Department will perform depth checks. The Contractor will be responsible for placement of material to the correct depth unless otherwise directed by the Engineer. If the placed material is not within a tolerance of $\pm 1/2$ inch of the plan shown depth, the Contractor will correct the problem at no additional cost to the Department. Excess material above the tolerance will not be paid for. Achieving the correct depth may require picking up and moving material or other action as required by the Engineer. All costs for providing the Contractor furnished checker and performing all related duties will be incidental to the contract lump sum price for the "Checker". No allowances will be made to the contract lump sum price for "Checker" due to authorized quantity variations unless the quantities for the material being checked vary above or below the estimated quantities by more than 25 percent. Payment for the Checker will then be increased or decreased by the same proportion as the placed material quantity bears to the estimated material quantity.

BASE COURSE, SALVAGED

Base Course, Salvaged will be obtained from the stockpile site(s) provided by the Contractor from the salvaged granular material produced on this project and may be used without further gradation testing.

The Contractor will ensure the Base Course, Salvaged material contains no more than 50% salvaged asphalt mix material and at least 50% granular material (salvaged or virgin). Salvaged Asphalt Mix and Granular Base Material will be blended to the satisfaction of the Engineer.

All other requirements for Base Course, Salvaged will apply.

GRAVEL CUSHION, SALVAGED

The Gravel Cushion, Salvaged will be obtained from the stockpile site(s) provided by the Contractor from the salvaged granular material produced on this project and may be used without further gradation testing.

The Contractor will ensure the Gravel Cushion, Salvaged material contains no more than 50% salvaged asphalt mix material and at least 50% granular material (salvaged or virgin). Salvaged Asphalt Mix and Granular Base Material will be blended to the satisfaction of the Engineer.

All other requirements for Gravel Cushion, Salvaged will apply.

TEMPORARY GRAVEL SURFACING

Temporary Gravel Surfacing will be required for surfacing transitions from the new pavement to the existing pavement to allow for continued traffic flow and access to driveways during and between phases as stated in the plans.

The Temporary Gravel Surfacing used in Phase 1 will be reused in Phase 2. The Temporary Gravel Surfacing used in Phase 3 will be reused in Phase 4. Temporary Gravel Surfacing used for transitions will meet all Base Course specifications and will be compacted to the satisfaction of the Engineer. All costs to furnish, install, compact, remove & relocate the Temporary Gravel Surfacing will be incidental to the contract unit price per ton for "Temporary Gravel Surfacing".

See Section C for Temporary Gravel Surfacing notes and details.

TABLE OF TEMPORARY GRAVEL SURFACING

TEMPORARY GRAVEL SURFACING				
			GRAVEL	
PHASE	LOCATION	NUMBER	(Ton)	
	Intersecting Streets	8	272	
Phase 1	Businesses	8	168	
	Residences	10	210	
	Cross Walks	8	80	
	Tota	I Tons Phase 1	730	
	Intersecting Streets	5	170	
Phase 2	Businesses	18	378	
	Residences	5	105	
	Cross Walks	8	80	
Total Tons Phase 2 733				
	Total Tons to be Paid for Phase 1 & Phase 2733			
	Intersecting Streets	7	238	
Phase 3	Businesses	13	273	
	Residences	1	21	
	Cross Walks	8	80	
	Tota	I Tons Phase 3	612	
	Intersecting Streets	9	306	
Phase 4	Businesses	10	210	
	Residences	2	42	
	Cross Walks	8	80	
	Tota	I Tons Phase 4	638	
Total Tons to be Paid for Phase 1 & Phase 2638			638	
TOTAL GRAVEL SURFACING 1371				

TEMPORARY GRAVEL SURFACING				
			GRAVEL	
			SURFACING	
PHASE	LOCATION	NUMBER	(Ton)	
	Intersecting Streets	8	272	
Phase 1	Businesses	8	168	
	Residences	10	210	
	Cross Walks	8	80	
	Tota	I Tons Phase 1	730	
	Intersecting Streets	5	170	
Phase 2	Businesses	18	378	
	Residences	5	105	
	Cross Walks	8	80	
Total Tons Phase 2 733				
	Total Tons to be Paid for Phase 1 & Phase 2733			
	Intersecting Streets	7	238	
Phase 3	Businesses	13	273	
	Residences	1	21	
	Cross Walks	8	80	
	Tota	I Tons Phase 3	612	
	Intersecting Streets	9	306	
Phase 4	Businesses	10	210	
	Residences	2	42	
	Cross Walks	8	80	
	Tota	I Tons Phase 4	638	
Total Tons to be Paid for Phase 1 & Phase 2638				
TOTAL GRAVEL SURFACING 1371				

TEMPORARY GRAVEL SURFACING				
			GRAVEL	
			SURFACING	
PHASE	LOCATION	NUMBER	(Ton)	
	Intersecting Streets	8	272	
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TOTAL GRAVEL SURFACING 1371				

TEMPORARY GRAVEL SURFACING			
			GRAVEL
			SURFACING
PHASE	LOCATION	NUMBER	(Ton)
	Intersecting Streets	8	272
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	Total Tons to be Paid for Phase 1 & Phase 2 733		
	Intersecting Streets	7	238
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Phase 4	Businesses	10	210
	Residences	2	42
	Cross Walks	8	80
	Tota	I Tons Phase 4	638
	Total Tons to Phase 1 &	be Paid for Phase 2	638
	TOTAL GRAVE	L SURFACING	1371

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Temporary cross walks - Locations will depend on the Contractor's sequence of operations and the direction of the Engineer. Estimated 10 tons for each cross walk. Intersecting Streets - Estimated 34 tons each at 40' wide. Business Access - Estimated 21 tons for each access 24' wide. Residences - Estimated 21 tons for each access 24' wide.

STATE OF	PROJECT	SHEET	TOTAL
SOUTH			OFFEIO
DAKOTA	NH-CR 0046(69)288	F2	F38
Plotting Date	10/23/2024		

8" NONREINFORCED PCC PAVEMENT

The aggregate may require screening as determined by the Engineer.

The concrete mix used in the PCC Pavement will conform to the Special Provision for Contractor Furnished Mix Design for PCC Pavement.

In lieu of an automatic subgrader operating from a preset line, a motor grader or other suitable equipment may be used to trim the gravel cushion to final grade prior to placement of concrete. There will be no direct payment for trimming of the gravel cushion for PCC pavement. The trimming will be considered incidental to the related items required for PCC Pavement.

A construction joint will be sawed whenever new concrete pavement is placed adjacent to existing concrete pavement.

The transverse construction joints will be handled in accordance with Standard Plate 380.15.

The location of joints, as shown and designated on the PCC Pavement Joint Layout(s) are only approximate locations to be used as a guide and to afford bidders a basis for estimating the construction cost of the joints. The final locations of the joints are to be designated by the Engineer during construction.

The entire surface of the mainline paving will be a heavy carpet drag. The surface of the mainline paving will receive a heavy carpet drag to within 2 or 3 feet of the face of the curb. All other areas will be textured as directed by the Engineer.

Unless specified otherwise in the PCC Pavement Joint Layout Sheets or elsewhere in the plans, the typical joint spacing for 8" Nonreinforced PCC Pavement will be 13'.

See Standard Plate 380.04 for placement of Dowel Bars.

The transverse contraction joints will be perpendicular to the centerline. In multilane areas the transverse contraction joints will be perpendicular to the centerline and be in a straight line across the entire width of pavement. In special situations the Engineer may pre-approve transverse contraction joints that do not meet these requirements. All nonconforming transverse contraction joints will be removed at the Contractor's expense. Any method of placement that cannot produce these requirements will not be allowed.

The location of joints, as shown and designated on the PCC Pavement Joint Layout(s) are only approximate locations to be used as a guide and to afford bidders a basis for estimating the construction cost of the joints. The final locations of the joints are to be designated by the Engineer during construction.

The Nonreinforced PCC Pavement will be tested using the 10' straight edge as per Specifications 380.3.O.1.

SAW AND SEAL JOINTS

Longitudinal and transverse joints will be sawed and sealed.

Joint sealing will conform to Section 380.3 P.

Longitudinal and transverse joints will be sealed with Hot Poured Elastic Joint Sealer.

PCC SHOULDER PAVEMENT

In lieu of an automatic subgrader operating from a preset grade line, a motor grader or other suitable equipment may be used to bring the gravel cushion to final grade prior to placement of the concrete.

The outside shoulder may be poured monolithic with the mainline pavement.

Provide a heavy carpet drag finish, a metal-tine finish will not be required on the shoulders. A metal-tine finish may be applied to the shoulders poured monolithic with the mainline.

If the shoulders are poured monolithic with the mainline pavement a sawed joint with tie bars will be constructed between the mainline pavement and the shoulders.

PAVEMENT SMOOTHNESS

The following locations will be tested for smoothness with a Contractor furnished and operated 25 foot California style profilograph in accordance with the Special Provision for PI PCC Pavement Smoothness with 0.2" Blanking Band:

SD46 - Sta. 11+00 to Sta. 124+66 - Driving Lanes

Turning lanes including center turn lane and side streets will be tested using the 10' straight edge as per Specifications 380.3.O.1

CURING OF CONCRETE

Portland Cement Concrete Pavement, Concrete Curb & Gutter, Concrete Gutter, and Concrete Fillet will be cured with Linseed Oil Base Emulsion Compound. All costs for Curing of Concrete will be incidental to the contract unit price per various Portland Cement Concrete bid items.

TABLE OF 8" NONREINFORCED PCC PAVEMENT

Location			8" NONREINFORCED PCC PAVEMENT
Sta	to	Sta.	(SqYd)
11+00	to	56+34	27,171.5
56+34	to	119+36	26,779.1
119+36	to	124+66	2,210.9
		Total:	56,161.5

TABLE OF PCC SHOULDER PAVEMENT

Location			PCC SHOULDER PAVEMENT
Sta	to	Sta.	(SqYd)
119+36	to	124+66	751.8
		Total:	751.8

PAVEMENT

Location	Stationing	8" NONREINFORCED PCC PAVEMENT
		(SqYd)
394th Avenue - North	28+79	222.6
394th Avenue - South	28+79	87.1
Lane Street - North	32+77	218.0
Old Highway 50 - North	42+40	476.7
Walnut Avenue SW - South	42+40	242.0
Birch Avenue SW - South	48+15	107.3
Washington Avenue NW - North	56+36	109.3
High Avenue NW - North	60+00	107.9
High Avenue SW - South	60+00	423.4
West Avenue NW - North	63+66	108.8
West Avenue SW - South	63+66	109.0
Main Street - North	67+50	137.0
Main Street - South	67+50	435.3
East Avenue SE - South	71+32	109.1
Grant Avenue SE - South	74+98	108.9
Grant Avenue NE - North	75+13	128.1
Sheridan Avenue SE - South	78+70	130.8
Front Avenue NE - North	81+87	479.2
Front Avenue SE - South	81+87	493.2
Tenth Avenue NE - North	95+16	0.0
East Harr Avenue - South	99+31	93.4
Total:		4,327.1

	STATE OF	PROJECT	SHEET	TOTAL
	SOUTH			ONEETO
DAK	DAKOTA	NH-CR 0046(69)288	F3	F38
	Plotting Date	10/23/2024		

TABLE OF 8" INTERSECTING ROADS NONREINFORCED PCC

TABLE OF 8" PCC PAVEMENT FOR DRIVEWAYS

Location	8" PCC DRIVEWAY PAVEMENT
	(SqYd)
Sta. 16+36 R	191.1
Sta. 19+10 R	149.9
Sta. 23+90 R	261.6
Sta. 27+04 R	194.4
Sta. 30+78 L	199.3
Sta. 30+78 R	184.2
Sta. 32+77 R	186.0
Sta. 35+88 R	210.6
Sta. 36+55 L	163.8
Sta. 44+36 L	19.4
Sta. 45+23 R	37.6
Sta. 47+64 L	40.1
Sta. 49+09 L	56.3
Sta. 50+46 R	45.4
Sta. 50+73 L	50.1
Sta. 52+84 R	88.4
Sta. 52+94 L	51.3
Sta. 57+34 L	20.9
Sta. 58+64R	122.9
Sta. 60+18, 105' R	53.2
Sta. 61+82 R	20.8
Sta. 66+00 L	49.0
Sta. 66+49 R	76.1
Sta. 67+14, 64' L	49.6
Sta. 67+85, 64' L	102.9
Sta. 68+93 L	35.6
Sta. 69+39 R	61.6
Sta. 72+80 L	22.4
Sta. 73+80 L	44.2
Sta. 75+90 L	81.1
Sta. 76+98 L	84.7
Sta. 77+93 L	44.6
Sta. 78+56 L	10.8
Sta. 88+19 L	21.4
Sta. 89+34 R	34.0
Sta. 89+61 L	19.3
Sta. 91+77 L	20.9
Sta. 111+50 L	295.1
Sta. 113+46 L	295.1
Sta. 118+54 R	441.8
Total:	4,137.5

TABLE OF 6" PCC PAVEMENT FOR DRIVEWAYS

Location	6" PCC DRIVEWAY PAVEMENT
	(SqYd)
Sta. 13+45 L	87.4
Sta. 71+85 L	23.6
Sta. 76+60 R	17.4
Sta. 79+97 L	43.3
Sta. 84+19 R	23.3
Sta. 85+69 R	22.2
Sta. 86+60 R	14.2
Sta. 87+29 R	27.4
Sta. 89+34 R	34.0
Total:	292.8

TABLE OF DOWEL BARS

Location	1 1/4" Bars
US 46	
Bars in Mainline - 12 bar	39,802
Bars in intersecting streets - 12 bar	2,373
Total Dowel Bars:	42,175

STEEL BAR INSERTION

The Contractor will insert the Steel Bars (11/4 inch x 18 inch epoxy coated plain round dowel bars) into drilled holes in the existing concrete pavement. An epoxy resin adhesive must be used to anchor the steel bar in the drilled hole.

The steel bars will be cut to the specified length by sawing or shearing and will be free from burring or other deformations.

Epoxy coated plain round steel bars will be inserted on 12 inch centers in the transverse joint. The first steel bar will be placed a minimum of 3 inches and a maximum of 9 inches from the outside edge of the slab.

TABLE OF STEEL BAR INSERTION

LOCATION	1-1/4" x 18" Plain Round Dowel Bars
Sta. 11+00	37
Sta. 28+79 - 85' L	35
Sta. 32+77 - 82' L	36
Sta. 42+40 - 100' L	40
Sta. 67+50- 88' R	70
Sta. 124+66	42
Total:	260

	STATE OF SOUTH DAKOTA	PROJECT	SHEET	TOTAL SHEETS	
		NH-CR 0046(69)288	F4	F38	
	Plotting Date:	10/23/2024			

ALKALI SILICA REACTIVITY

Fine aggregate will conform to Section 800.2 D Alkali Silica Reactivity (ASR) Requirements.

Below is a list of known fine aggregate sources and the average corresponding 14-day expansion values (as of 9-18-2024):

Source	Location	Expansion Value
Bachman	Winner, SD	0.335*
Bitterman	Delmont, SD	0.316*
Concrete Materials	Corson SD	0 146
Concrete Materials - Vellek Pit	Yankton, SD	0.411**
Croell	Hot Springs, SD	0.089
Croell	Wasta, SD	0.212
Emme Sand & Gravel	Oneil, NE	0.217
Fisher S&G – Blair Pit	W of Vale, SD	0.171
Fisher S&G - Mickelson Pit	E of Nisland, SD	0.129
Fisher S&G - Vallery Pit	Nisland, SD	0.110
Fisher S&G	Rapid City, SD	0.092
Fisher S&G	Spearfish, SD	0.053
Fisher S&G	Wasta, SD	0.159
Fuchs	Pickstown, SD	0.275*
Henning – Tilstra Pit	Ash Creek, MN	0.199
Higman	Hudson, SD	0.187
Jensen	Herried, SD	0.276*
L.G. Everist	Akron, IA	0.257*
L.G. Everist	Brookings, SD	0.297*
L.G. Everist – Ode Pit	E Sioux Falls, SD	0.222
L.G. Everist – Nelson Pit	NE Sioux Falls, SD	0.156
L.G. Everist	Hawarden, IA	0.211
L.G. Everist	Summit, SD	0.184
Mark's S&G – Moerke Pit	Underwood, MN	0.165
Morris – Birdsall	Blunt, SD	0.229
Morris - Leesman	Blunt, SD	0.231
Morris - Richards Pit	Onida, SD	0.188
Morris - Shawn's Pit	E of Sturgis, SD	0.186
Northern Concrete Agg.	Rauville, SD	0.113
Northern Concrete Agg.	Luverne, MN	0.154
Opperman - Gunvordahl Pit	Burke, SD	0.363*
Opperman - Cahoy Pit	Herrick, SD	0.307*
Opperman - Jones Pit	Burke, SD	0.321*
Opperman - Randall Pit	Pickstown, SD	0.230
Pete Lien & Sons	Creston, SD	0.158
Pete Lien & Sons	Oral, SD	0.157
Pete Lien & Sons	Wasta, SD	0.255*
	Scottsbiutt, NE	0.277*
Thorpe Pit	Britton, SD	0.098
Valley S&G – Van Beek Pit	Rock Valley, IA	0.228
Wagner Building Supplies	Pickstown (Wagner), SD	0.251*
Winter Brothers- Whitehead Pit	Brookings, SD	0.197

* These sources will require Type II cement with a fly ash content of 25% in the concrete mix.

** These sources will not be used.

The Department will use the running average of the last three or fewer known expansion test results for determining acceptability of the source. These expansion results are reported in the preceding table. Additional testing,

when requested by the Contractor, will be performed by the Department at the Contractor's expense.

The values listed in the table are intended for use in bidding. If a previously tested pit by SDDOT with a test value less than 0.250 is discovered after letting to be 0.250 or greater, then the Department will accept financial responsibility if higher costs are incurred due to higher percent of fly ash requirement.

MANHOLE BOX-OUT DETAILS

The Contractor will construct box-outs for all manholes in the 8" Nonreinforced PCC Pavement according to the Box-Out Detail. Locations of Proposed Manholes and water valve boxes are shown on the PCC Pavement Joint Layout Sheets.

ASPHALT CONCRETE COMPOSITE

Asphalt Concrete Composite placed as a final surfacing will include MC-70 Asphalt for Prime placed at the rate of 0.30 gallons per square yard. The Asphalt for Prime will be applied to the Base Course or Base Course, Salvaged for the full width of the bottom layer of Asphalt Concrete Composite plus one foot additional on the outside shoulder.

Asphalt Concrete Composite placed as the temporary surfacing for the traffic diversion estimated at 3,189.4 tons will not require the placement of MC-70 Asphalt for Prime or Blotting Sand for Prime prior to the Asphalt Concrete Composite placement. Asphalt for Flush Seal SS-1h or CSS-1h and Sand for Flush Seal will not be required for the temporary Asphalt Concrete Composite surfacing for the traffic diversion.

Asphalt for tack SS-1h or CSS-1h will be applied prior to each lift of Asphalt Concrete Composite. Asphalt for tack will be applied at a rate of 0.09 gallons per square yard on existing pavement or milled asphalt concrete surfaces and at a rate of 0.06 gallons per square yard on primed base course or new asphalt concrete pavement. The Asphalt for tack will be applied for the full width of the bottom layer of Asphalt Concrete Composite plus one-half foot additional on the outside shoulder.

	STATE OF	PROJECT	SHEET	TOTAL SHEETS
DAKOTA		NH-CR 0046(69)288	F5	F38
	Plotting Date:	10/23/2024		

							STATE OF	PROJECT	SHEET
							SOUTH DAKOTA	NH-CR 0046(69)288	F6
BLE OF MATERIALS QUANTITIES		1		I	1	، ب	Pletting Date:	10/23/2024	I
	WATER FOR	GRAVEL CUSHION,	BASE	BAGE		ASP	HALT		
LOCATION	GRANULAR	SALVAGED/ GRAVEL	COURSE,			CONC	CRETE		
	MATERIAL	CUSHION	SALVAGED	COURSE	SURFACING	COMF	POSITE		
						1st Lift	Top Lift		
Station to Station	(MGal)	(Ton)	(Ton)	(Ton)	(Ton)	(Ton)	(Ton)		
			(TON)	(1011)					
11+00.00 to 56+34.00	113.1	9425.0							
56+34.00 to 119+36.00	122.6	10 21/ 0							
110+36.00 to 124+66.00	10.4	870.0							
119-30.00 10 124-00.00	10.4	870.0							
30/th Avenue North	0.9	72.0							
394th Avenue - North	0.9	72.0				10.0	10.0		
Lang Street North	0.9	68.0				19.0	19.0		
	1.5	122.0				+			
Old Highway 50 - North	1.5	77.0							
Direb Avenue SW - South	0.9	77.0				0.0	0.0		
BIRCH AVENUE SVV - SOUTH	0.7	55.0				9.0	9.0		
vvasnington Avenue NVV - North	0.8	63.0				11.0	11.0		
High Avenue NW - North	0.5	44.0				5.0	5.0		
High Avenue SW - South	2.1	1/2.0				28.0	28.0		
West Avenue NW - North	0.5	44.0				5.0	5.0		
West Avenue SW - South	0.6	51.0				8.0	8.0		
Main Street - North	1.4	118.0				40.0	40.0		
Main Street - South	1.6	131.0							
East Avenue SE - South	0.8	70.0				16.0	16.0		
Grant Avenue SE - South	0.6	51.0				8.0	8.0		
Grant Avenue NE - North	0.7	56.0				9.0	9.0		
Sheridan Avenue SE - South	0.7	58.0				9.0	9.0		
Front Avenue NE - North	1.9	162.0				20.0	20.0		
Front Avenue SE - South	2.0	168.0				23.0	23.0	1	
Tenth Avenue NE - North	0.4	34.0						1	
Harr Avenue - South	0.6	50.0							
8" PCC DRIVEWAY PAVEMENT (40)	12.0	960.0							
6" PCC DRIVEWAY PAVEMENT (9)	0.9	63.0							
ASPHALT CONCRETE COMPOSITE DRIVEWAYS (2)	1.0			84.0		32.0	32.0		
				01.0		02.0	02.0		
GRANULAR DRIVEWAYS (30)	3.0	360.0							
TRAFFIC CONTROL QUANTITIES									
9+55.00 to 10+00.00	0.1		8.2			19	21		
10+00.00 to 20+25.00	3.6		297.7			86.4	86.4		
20+25.00 to 28+52.43	7.8		646.4			21/ 1	21/ 1		
$\frac{20+23.00}{20+32.43}$	<u> </u>		420.8			122.2	122.2		
$\frac{20+32.43}{40+72.43} = \frac{40+72.43}{40+72.43}$	5.5		439.0			133.3	133.3		
$\frac{40+72.43}{40+72.43} = \frac{10}{42+62.47}$	0.8		00.0			20.0	20.0		
42+02.47 10 05+50.00	9.9		ŏ∠4.4			250.0	250.0		
	3.0	249.8		550 (47.3	41.3	1	
00+50.00 TO 81+82.69	6.6			552.4		167.5	167.5	{	
81+82.69 to 108+26.39	11.4			952.8		288.9	288.9		
108+26.39 to 112+46.39	2.5			210.4		66.9	66.9		
112+46.39 to 125+66.00	11.3			938.4		308.6	308.6		
125+66.00 to 126+42.55	0.4			34.1		8.5	9.3		
TEMPORARY GRAVEL SURFACING - TRAFFIC CONTROL	16.5				1,371.0			l	
T	otals 363.1	23,881.8	2,285.0	2,772.1	1,371.0	3,6	73.4		





IN PLACE TYPICAL SECTIONS





	STATE OF	PROJECT	SHEET	TOTAL SHEETS
7	SOUTH DAKOTA	NH-CR 0046(69)288	F9	F38
3.67		0.5" - C & G, In Place		
	Base Cours	e, In Place		

TYPICAL SURFACING SECTIONS







(STATE OF	PROJECT	SHEET	TOTAL SHEETS
Ref (SOUTH DAKOTA	NH-CR 0046(69)288	F10	F38
		Transitions:		
	STATE OF SOUTH DAKOTA PROJECT SHEET DAKOTA NH-CR 0046(69)288 F10 Transitions: Sta. 9+55 to Sta. 16+15 * 6' to 5.5' *** 12' to 11' *** 0' to 11' **** 0' to 11'			

Crown Shift: Center to 5.5' L: Sta. 11+00 to Sta. 12+00







Sta. 56+34 to Sta. 57+10 * 11'

Sta. 57+10 to Sta. 58+84 * 11' to 2.5'

Sta. 56+34 to Sta. 57+10 ** Crown 5.5' Lt.

Sta. 57+10 to Sta. 57+90 ** Crown shift 5.5' Lt. to Center





























...\05JN_PCC Layouts - 2D.dg



...\05JN_PCC Layouts - 2D.dgn



... \05JN_PCC Layouts - 2D.d















Plot Scale -





otted From - TRPR13462

File _ \cmix05.IN\StdPlateSectionF









LONGITUDINAL	CONSTRUCTION JOINT WITH TIE BARS	
Sawed Joint filled with F Poured Elastic Joint Sea	(Drilled in Bars) lot	nickness
In place PCC Pave placed on previous or current project Drilled The tie bars we the in place F	Metal Recess Strip 9" (Min.) Hole No.5 Epoxy Coated Deform vill be embedded a minimum depth of 9 inches into PCC Pavement	ned Tie Bar
LONGITUDINAL	CONSTRUCTION JOINT WITH TIE BARS (Inserted or Formed in Bars)	
Sawed Joint filled with H Poured Elastic Joint Se		
H In place PCC Pave placed on the curre project ★ A	ement ent 15" **	
GENERAL NOTES (For the details abov	e):	ned Tie Bar
The epoxy coated deformed tie bars will	be spaced in accordance with the following tables:	
	TIE BAR SPACING 30" MAXIM	NUM
TIE BAR SPACING 48" MA	Joint Spacing Tie Ba	ars
Transverse Contraction Nu	mber of 5' to 7' 2	
Joint Spacing Ti	e Bars 7.5' to 9.5' 3	
6.5' to 10'	2 10 to 12 4 12.5' to 14.5' 5	
10.5' to 14 14 5' to 18'	<u>4</u> <u>15' to 17'</u> 6	
18.5' to 22'		
The tie bars will be placed a minimum	of 15 inches from transverse contraction joints.]
The required number of tie bars as sho spaced tie bars will be spaced a maxin spaced a maximum of 30 inches cente spacing will apply to tie bars within eac	wn in the table will be uniformly spaced within each par num of 48 inches center to center for a female keyway a r to center for a vertical face and male keyway. The ma h panel.	nel. The uniform and will be aximum tie bar
The keyway illustrated in the above de	tails depict a female keyway.	
The keyway is optional and is not requ metal recess strip will be used. When	ired. When concrete pavement is formed and a keyway concrete pavement is slip formed, a metal recess strip i	/ is provided, a s not required.
 ★ The vertical placement tolerance for ar ★★The transverse placement (side shift) to longitudinal joint line. 	by part of the tie bar will be \pm T/6. Derance will be \pm 3 inches when measured perpendicul	ar to the November 19, 202
S D D	PCC PAVEMENT LONGITUDINAL	plate number 380.20
Published Date: 2025	JUNNIS WIIN HE DANS	Sheet 1 of 2



		PI	lotting Date:	10/23/2024	ן	
SAWED L	ONG	ITUDINAL JOINT WITH TIE (Poured Monolithically)	BARS			
Sawed Joint filled wit Poured Elastic Joint filled New PC	h Hot Sealer	vement 15" * * Line of Fractu No. 5 Epoxy Coated Deformed T	re Fie Bars	vement		
	-	T = Pavement Thickness				
GENERAL NOTES (For the deta The epoxy coated deformed tie b	ail abo bars w	ove): /ill be spaced in accordance with the	e following	g table:		tion Loss
	TIE	BAR SPACING 48" MAXIMUMansverse ContractionNumber ofJoint SpacingTie Bars6.5' to 10'210.5' to 14'314.5' to 18'418.5' to 22'5				Jonniv OK INI StdDlata So
The tie bars will be placed a min	imum	of 15 inches from the transverse co	ntraction	joints.		
The required number of tie bars a maximum space of 48 inches o within each panel.	as sho center	own in the table will be uniformly spa to center. The maximum tie bar spa	aced with acing will	in each panel with apply to tie bars		
The first saw cut to control crack sawing for widening the saw cut sealer is necessary.	ing wi to prc	ill be a minimum of 1/3 the thickness wide the width for the installation of	s of the pa the hot po	avement. Additional oured elastic joint		
 The vertical placement tolerance The transverse placement (side longitudinal joint line. 	for a shift)	ny part of the tie bar will be \pm T/6. tolerance will be \pm 3 inches when me	easured p	perpendicular to the		
				November 19, 2022	2	
	5 D D D	PCC PAVEMENT LONGIT	UDINAL	PLATE NUMBER 380.20		





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Published Date: 2025

	STATE OF SOUTH		PROJECT	SHEET	TOTAL SHEETS
	DAKOTA Plotting Date:	10/23	CR 0046(69)288	F37	F38
	. Iotany Date.	10/20		1	
(Concrete Curb and	Addified C	uttor)			
	viounieu G	uller			
PCC Paveme	ent	~~>			
T:	=Pavemen	t Thickne	SS		
			*.		
		ent			
^ ^ 15" ** ^ ^			* *		
Line of Fracture					
Coated Deformed Tie	Bars				
ced 48 inches center to	center.				
s from existing transver	se contrac	tion joints			
ced monolithically with	the PCC p	avementi	if the mainline		
	a a f 11				
dth for the installation of	the hot po	oured elas	Additional tic joint		
	1				
versely at each mainline and gutter will be sawed	transverse and seale	e contract ed same a	ion joint. The is the transverse		
	-				
ed for the type of gutter of will be constructed at the	or curb and e same slo	d gutter to ope as the	be constructed. mainline		
ne tie bar will be ± T/6. ill be ± 3 inches when m	neasured r	erpendicu	ular to the		
	F		March 31, 2024		
VEMENT LONGITUDINA	L CONSTRU	JCTION	PLATE NUMBER		
INTS WITH CONCRETE	GUTTER ()R	380.21		
CONCREIE CORB AND	GUITEK		Sheet 2 of 2		



