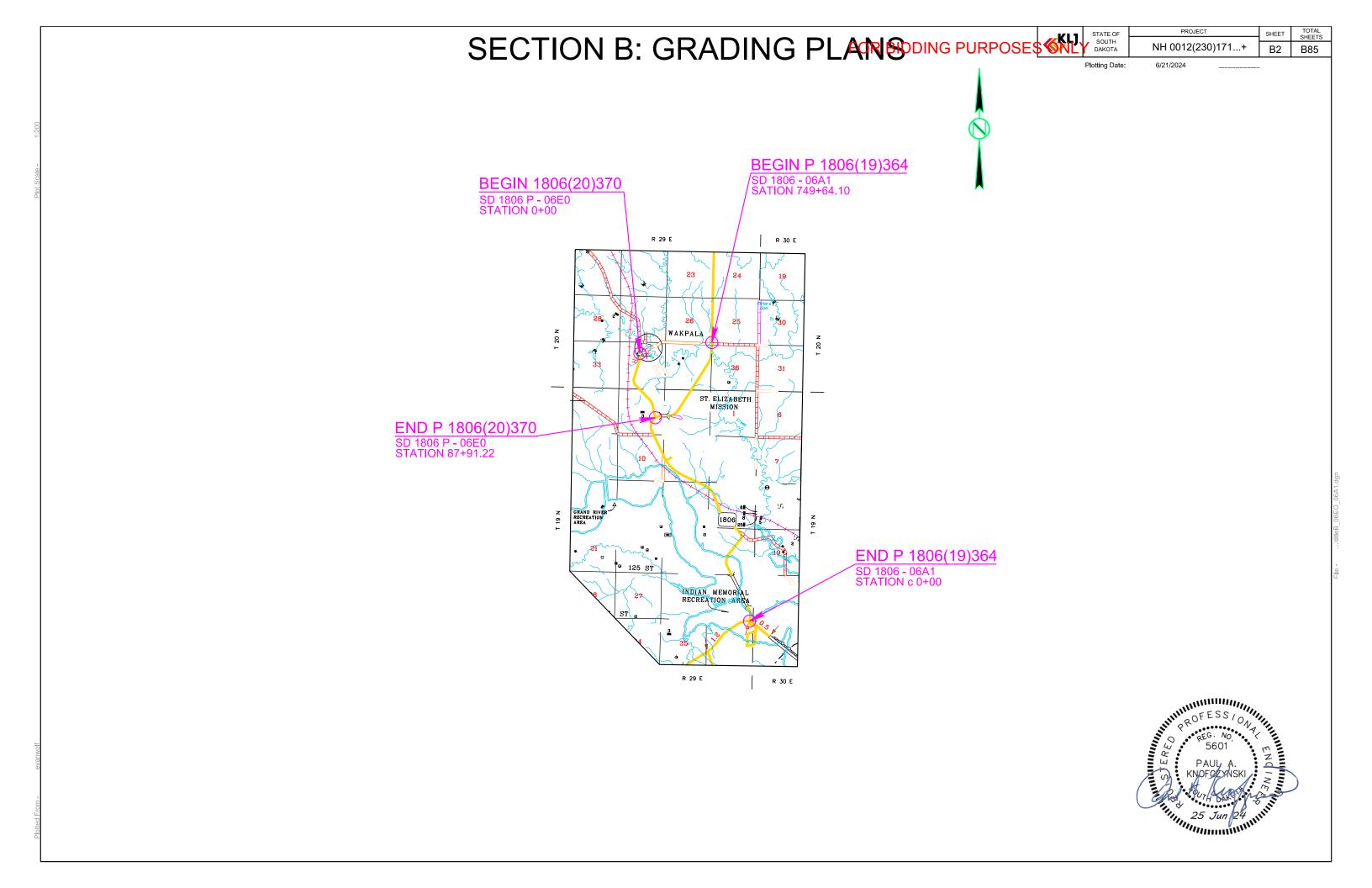
SECTION B: GRADING PLAN SOUTH SOUTH DAKOTA TOTAL SHEETS PROJECT SHEET NH 0012(230)171...+ В1 B85 Plotting Date: **INDEX OF SHEETS** B1-B3 General Layout with Index Estimate with General Notes and Tables B4-B12 B13-B20 Pipe Quantities Typical Grading Sections
Horizontal Alignment Data B21-B23 B24-B26 BEGIN NH 0012(230)171 Control Data B27 US 12 - 05TY STATION 176+72.2 B28 Legend Plan and Profile Sheets B29-B39 B40 Proposed Landslide Contours B41-B55 Guardrail Sheets B56-B60 Special Detail Sheets B61-B85 Standard Plates R 28 E R 29 E END NH 0012(230)171 US 12 - 05TY STATION c 268+01.75 R 29 E

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SECTION B: GRADING PLAN SOUTH SOUTH DAKOTA

Plotting Date:

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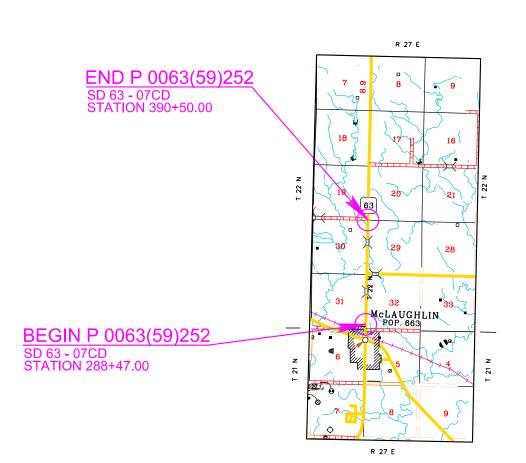
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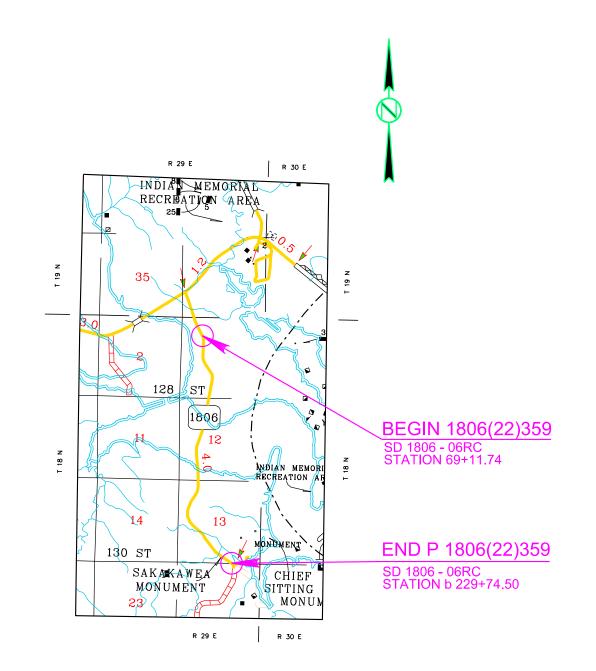
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TOTAL SHEETS

B85

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### **SECTION B ESTIMATE OF QUANTITIES**

#### 05TY-Section B

BID ITEM NUMBER	ITEM	QUANTITY	UNIT
009E0010	Mobilization	Lump Sum	LS
009E3210	Construction Staking	15.015	Mile
009E3250	Miscellaneous Staking	15.015	Mile
009E3280	Slope Staking	0.528	Mile
009E3301	Engineer Directed Surveying/Staking	50.0	Hour
009E4200	Construction Schedule, Category II	Lump Sum	LS
110E0135	Remove Delineator	7	Each
110E0500	Remove Pipe Culvert	46	Ft
110E0510	Remove Pipe End Section	19	Each
110E0595	Remove Cattle Pass End Section	6	Each
110E0600	Remove Fence	61	Ft
110E0730	Remove Beam Guardrail	1,400.0	Ft
110E7500	Remove Pipe for Reset	392	Ft
110E7510	Remove Pipe End Section for Reset	40	Each
120E0600	Contractor Furnished Borrow	16,432	CuYd
120E6100	Water for Embankment	249.2	MGal
250E0020	Incidental Work, Grading	Lump Sum	LS
450E0143	24" RCP Class 3, Furnish	8	Ft
450E0150	24" RCP, Install	8	Ft
450E0203	48" RCP Class 3, Furnish	8	Ft
450E0210	48" RCP, Install	8	Ft
450E0213	54" RCP Class 3, Furnish	8	Ft
450E0220	54" RCP, Install	8	Ft
450E2008	18" RCP Flared End, Furnish	8	Each
450E2009	18" RCP Flared End, Install	8	Each
450E2028	36" RCP Flared End, Furnish	2	Each
450E2029	36" RCP Flared End, Install	2	Each
450E2036	48" RCP Flared End, Furnish	2	Each
450E2037	48" RCP Flared End, Install	2	Each
450E2040	54" RCP Flared End, Furnish	3	Each
450E2041	54" RCP Flared End, Install	3	Each
450E2200	24" RCP Sloped End, Furnish	7	Each
450E2201	24" RCP Sloped End, Install	7	Each
450E4768	24" CMP 14 Gauge, Furnish	30	Ft
450E4770	24" CMP, Install	30	Ft
450E5310	24" CMP Sloped End, Furnish	2	Each
450E5311	24" CMP Sloped End, Install	2	Each
450E8900	Cleanout Pipe Culvert	16	Each
450E9000	Reset Pipe	392	Ft
450E9001	Reset Pipe End Section	40	Each
462E0250	Cellular Grout	164.4	CuYd
620E0020	Type 2 Right-of-Way Fence	61	Ft
620E0520	Type 2 Temporary Fence	140	Ft

<sup>\* -</sup> Denotes Non-Participating

BID ITEM NUMBER	ITEM	QUANTITY	UNIT
620E1020	2 Post Panel	2	Each
630E0500	Type 1 MGS	650.0	Ft
630E1500	Type 1 Guardrail Transition	10	Each
630E2017	MGS MASH Flared End Terminal	10	Each
632E2510	Type 2 Object Marker Back to Back	97	Each
700E0210	Class B Riprap	37.3	Ton
720E1010	PVC Coated Bank and Channel Protection Gabion	54.0	CuYd
831E0110	Type B Drainage Fabric	214	SqYd

#### 06RC-Section B

BID ITEM NUMBER	ITEM	QUANTITY	UNIT
009E0010	Mobilization	Lump Sum	LS
009E3210	Construction Staking	3.554	Mile
009E3230	Grade Staking	0.106	Mile
009E3250	Miscellaneous Staking	3.554	Mile
009E3280	Slope Staking	0.106	Mile
009E3301	Engineer Directed Surveying/Staking	25.0	Hour
009E4200	Construction Schedule, Category II	Lump Sum	LS
110E0500	Remove Pipe Culvert	226	Ft
110E0510	Remove Pipe End Section	7	Each
110E0600	Remove Fence	687	Ft
120E0010	Unclassified Excavation	18,143	CuYo
120E0600	Contractor Furnished Borrow	19,054	CuYo
120E6100	Water for Embankment	308.7	MGa
250E0020	Incidental Work, Grading	Lump Sum	LS
450E0143	24" RCP Class 3, Furnish	186	Ft
450E0150	24" RCP, Install	186	Ft
450E2200	24" RCP Sloped End, Furnish	4	Each
450E2201	24" RCP Sloped End, Install	4	Each
450E4768	24" CMP 14 Gauge, Furnish	50	Ft
450E4770	24" CMP, Install	50	Ft
450E5015	24" CMP Elbow, Furnish	4	Each
450E5016	24" CMP Elbow, Install	4	Each
450E5306	18" CMP Sloped End, Furnish	1	Each
450E5307	18" CMP Sloped End, Install	1	Each
450E5310	24" CMP Sloped End, Furnish	2	Each
450E5311	24" CMP Sloped End, Install	2	Each
450E8014	24" RCP to CMP Transition, Furnish	2	Each
450E8015	24" Pipe Transition, Install	2	Each
* 450E8900	Cleanout Pipe Culvert	5	Each
620E0020	Type 2 Right-of-Way Fence	687	Ft
620E0520	Type 2 Temporary Fence	1,244	Ft
620E1020	2 Post Panel	8	Each
632E2510	Type 2 Object Marker Back to Back	18	Each
680E0224	4" PVC Outlet Pipe	100	Ft
680E0440	4" Slotted Corrugated Polyethylene Drainage Tubing	280	Ft
680E2000	Concrete Headwall for Underdrain	1	Each
680E2500	Porous Backfill	118.0	Ton
720E1010	PVC Coated Bank and Channel Protection Gabion	9.0	CuYo
831E0110	Type B Drainage Fabric	30	SqYc

#### \* - Denotes Non-Participating

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06E0-Section B

BID ITEM NUMBER	ITEM	QUANTITY	UNIT	
009E0010	Mobilization	Lump Sum	LS	
009E3210	Construction Staking	1.665	Mile	
009E3250	Miscellaneous Staking	1.665	Mile	
009E3301	Engineer Directed Surveying/Staking	25.0	Hour	
009E4200	Construction Schedule, Category II	Lump Sum	LS	
110E0135	Remove Delineator	5	Each	
110E0510	Remove Pipe End Section	1	Each	
110E0600	Remove Fence	88	Ft	
110E7500	Remove Pipe for Reset	40	Ft	
110E7510	Remove Pipe End Section for Reset	3	Each	
250E0020	Incidental Work, Grading	Lump Sum	LS	
450E2028	36" RCP Flared End, Furnish	1	Each	
450E2029	36" RCP Flared End, Install	1	Each	
450E8300	Culvert Joint Cleaning	198.0	Ft	
450E8305	Repair Culvert Joint	198.0	Ft	
450E8310	Chemical Grout Void Fill	12.0	Gal	
* 450E8900	Cleanout Pipe Culvert	9	Each	
450E9000	Reset Pipe	40	Ft	
450E9001	Reset Pipe End Section	3	Each	
620E0020	Type 2 Right-of-Way Fence	88	Ft	
620E0520	Type 2 Temporary Fence	312	Ft	
620E1020	2 Post Panel	4	Each	
632E2510	Type 2 Object Marker Back to Back	30	Each	

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#### 07CD-Section B

BID ITEM NUMBER	ITEM	QUANTITY	UNIT
009E0010	Mobilization	Lump Sum	LS
009E3210	Construction Staking	3.731	Mile
009E3250	Miscellaneous Staking	0.100	Mile
009E3301	Engineer Directed Surveying/Staking	25.0	Hour
009E4200	Construction Schedule, Category II	Lump Sum	LS
110E0730	Remove Beam Guardrail	910.0	Ft
120E0600	Contractor Furnished Borrow	140	CuYd
250E0020	Incidental Work, Grading	Lump Sum	LS
* 450E8900	Cleanout Pipe Culvert	8	Each
630E0500	Type 1 MGS	362.5	Ft
630E1500	Type 1 Guardrail Transition	8	Each
630E2017	MGS MASH Flared End Terminal	8	Each
632E2510	Type 2 Object Marker Back to Back	12	Each

<sup>\* -</sup> Denotes Non-Participating



<sup>\* -</sup> Denotes Non-Participating

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06A1-Section B

BID ITEM NUMBER	ITEM	QUANTITY	UNIT
009E0010	Mobilization	Lump Sum	LS
009E3210	Construction Staking	6.859	Mile
009E3230	Grade Staking	0.316	Mile
009E3250	Miscellaneous Staking	6.859	Mile
009E3280	Slope Staking	0.316	Mile
009E3301	Engineer Directed Surveying/Staking	25.0	Hour
009E4200	Construction Schedule, Category II	Lump Sum	LS
110E0135	Remove Delineator	4	Each
110E0500	Remove Pipe Culvert	82	Ft
110E0510	Remove Pipe End Section	11	Each
110E0600	Remove Fence	88	Ft
110E0730	Remove Beam Guardrail	1,040.0	Ft
110E7510	Remove Pipe End Section for Reset	1	Each
120E0010	Unclassified Excavation	13,054	CuYd
120E0600	Contractor Furnished Borrow	10,035	CuYd
120E2000	Undercutting	6,252	CuYd
120E6100	Water for Embankment	149.4	MGal
250E0020	Incidental Work, Grading	Lump Sum	LS
270E0112	Salvage Granular Material	1,313.0	Ton
450E0143	24" RCP Class 3, Furnish	60	Ft
450E0150	24" RCP, Install	60	Ft
450E0183	36" RCP Class 3, Furnish	100	Ft
450E0190	36" RCP, Install	100	Ft
450E2028	36" RCP Flared End, Furnish	4	Each
450E2029	36" RCP Flared End, Install	4	Each
450E2200	24" RCP Sloped End, Furnish	3	Each
450E2201	24" RCP Sloped End, Install	3	Each
450E4600	24" RCP Arch Sloped End, Furnish	1	Each
450E4601	24" RCP Arch Sloped End, Install	1	Each
450E4758	18" CMP 14 Gauge, Furnish	64	Ft
450E4760	18" CMP, Install	64	Ft
450E5310	24" CMP Sloped End, Furnish	5	Each
450E5311	24" CMP Sloped End, Install	5	Each
450E5406	18" CMP Safety End, Furnish	2	Each
450E5407	18" CMP Safety End, Install	2	Each
450E8300	Culvert Joint Cleaning	240.0	Ft
450E8305	Repair Culvert Joint	240.0	Ft
450E8310	Chemical Grout Void Fill	12.0	Gal
450E8900	Cleanout Pipe Culvert	22	Each
450E9001	Reset Pipe End Section	1	Each
462E0250	Cellular Grout	30.2	CuYd
464E0100	Controlled Density Fill	17.2	CuYd
620E0020	Type 2 Right-of-Way Fence	88	Ft

<sup>\* -</sup> Denotes Non-Participating

BID ITEM NUMBER	ITEM	QUANTITY	UNIT
620E0520	Type 2 Temporary Fence	233	Ft
620E1020	2 Post Panel	4	Each
630E0500	Type 1 MGS	300.0	Ft
630E1500	Type 1 Guardrail Transition	12	Each
630E2017	MGS MASH Flared End Terminal	12	Each
632E2510	Type 2 Object Marker Back to Back	53	Each
720E1010	PVC Coated Bank and Channel Protection Gabion	16.5	CuYd
831E0110	Type B Drainage Fabric	53	SqYd
831E1010	Geogrid Reinforcement	4,034	SqYd

#### **GRADING OPERATIONS**

Water for Embankment is estimated at the rate of 15 gallons of water per cubic yard of Embankment minus waste. The estimated quantity of Water for Embankment is 249.2 MGal for 05TY, 149.4 MGal for 06A1, and 308.7 MGal for 06RC. All costs associated will be incidental to the contract unit price per MGal of "Water for embankment".

For embankment soil with an optimum moisture of 20% or greater, the Density Specification (Percent of Maximum Dry Density) will be 92% to 98% and the Moisture Specification (Percent of Optimum Moisture) will be -2% to +3%.

Special ditch grade and other sections of the roadway different than the typical sections will be constructed to the limits shown ion the cross sections. If significant changed to the cross sections are necessary during construction, the Engineer will contact the Designer for the proposed change.

A copy of the surfacing/ Subgrade investigation for these projects is available from the Pierre Region and Mobridge Area offices.

#### **TABLE OF EXCAVATION QUANTITIES BY BALANCES**

06RC	Total Excavation (CuYd)	Contractor Furnished Borrow Exc. (CuYd)
Landslide Repair and Underdrain Installation	18143	19054
06RC Total	18143	19054

06A1	Excavation (CuYd)	Undercut (CuYd)	Contractor Furnished Borrow Exc. (CuYd)	Total Excavation (CuYd)
Grading at Guardrail Locations			165	
Heave Repair MRM 367.300 to MRM 367.545	5369	5069	7732	10438
Base Course Reinforcement MRM 369.817 to MRM 369.873	1433	1183	2063	2616
06A1 Total	6802	6252	9960	13054

#### UTILITIES

The Contractor will contact the involved utility companies through South Dakota One Call (1-800-781-7474) prior to starting work. It will be the responsibility of the Contractor to coordinate work with the utility owners to avoid damage to existing facilities.

Utilities are not planned to be affected on this project. If utilities are identified near the improvement area through the SD One Call Process as required by South Dakota Codified Law 49-7A and Administrative Rule Article 20:25, the Contractor will contact the Engineer to determine modifications that will be necessary to avoid utility impacts.

A potential waterline at MRM 370.00+0.530 on SD 1806 was identified. The Contractor must utilize One-Call as per Section 5.6 of the Specifications to avoid any utility conflicts.

## PROCEDURES FOR DETERMINING UNCLASSIFIED EXCAVATION QUANTITY

Unclassified Excavation will not be measured but will be paid for as plans quantity. Removal and replacement of the topsoil will not be measured but will be paid for at the contract lump sum price for Remove and Replace Topsoil.

The Excavation quantities from individual balances and the table above have been reduced by the volume of in place concrete pavement and asphalt pavement that will be removed.

(CuYd)

#### TABLE OF UNCLASSIFIED EXCAVATION

Excavation		` 680Ź
Undercut		6252
	Total	13054
06RC		(CuYd)
		` ,
Excavation	. <del>-</del>	18143
	Total	18143

06A1



#### **SALVAGE GRANULAR MATERIAL**

In the heave repair areas, the Contractor will be required to salvage enough existing granular base material to provide for a 6" lift of temporary surfacing which will be utilized until the placing the asphalt concrete surfacing for the project. The temporary surfacing is estimated to require 1313 tons of salvaged material. Costs associated with salvaging and stockpiling the material for use as temporary surfacing is incidental to the contract unit price per ton for "Salvage Granular Material".

#### **UNDERCUTTING**

The undercut depth for the Heave Repair and Base Course Reinforcement areas will be 3 feet.

The plan shown quantity will be the basis of payment. However, if there are additional areas of undercut other than what is shown in the plans, the Engineer will direct removal of these areas and the additional areas will be measured according to the Engineer.

#### **TABLE OF UNDERCUTTING**

#### 06A1

MRM to	MRM	Quantity
367.300	367.550	5069
369.817	369.873	1183
	Total	6252

## HEAVE REPAIR 06A1

From MRM 367.300 to MRM 367.550 the earthen subgrade will be undercut 3 feet below the earthen subgrade surface at the heave areas specified in the table below. The undercut material or other suitable material, as directed by the Engineer, will then be replaced, and compacted to the density specified for the section being constructed. The undercut will utilize a 10:1 taper from the top of the subgrade to the bottom of the undercut.

#### **CONTRACTOR FURNISHED BORROW EXCAVATION**

The Contractor will provide a suitable site for Contractor furnished borrow excavation material. The Contractor is responsible for obtaining all required permits and clearances for the borrow site. The borrow material will be approved by the Engineer. The plans quantity for "Contractor Furnished Borrow Excavation" as shown in the Estimate of Quantities will be the basis of payment for this item.

Restoration of the Contractor furnished borrow excavation site will be the responsibility of the Contractor.

#### TABLE OF CONTRACTOR FURNISHED BORROW

05TY	
Location	(CuYd)
Inslope Flattening	14578
US12/SD20 Turn Lane Widening	1537
US12/SD1806 Turn Lane Widening	148
Pipe Work	121
Grading at Guardrail Locations	48
05TY Total	16432
<b>06A1</b>	
Location	(CuYd)
Grading at Guardrail Locations	240
Heave Repair	7732
MRM 367.300 to MRM 367.545	
Base Course Reinforcement MRM 369 817 to MRM 369 873	2063
06A1 Total	10035
06RC	
Location	(CuYd)
Landslide Repair and Underdrain Installation	19054
06RC Total	19054
07CD	
Location	(CuYd)
Grading at Guardrail Locations	140
07CD Total	140

#### **CELLULAR GROUT**

The Contractor will submit a proposed grouting procedure to the Engineer at least two weeks prior to beginning this work.

Bulkheads will be constructed at each end of the pipe. Each bulkhead will be constructed to withstand the pressure of the grouting operation. The bulkhead will extend from the end of the existing pipe inward a minimum depth of 18 inches and will be free from leaks.

Pressure grouting will be done to ensure all the voids are filled including all breaks or holes in and around the existing pipe.

The grout will be a cellular grout (grout with pre-generated foam) with a minimum 28-day compressive strength of 100 pounds per square inch. If water is not present within the pipe a low-density grout with a minimum of 30 pounds per cubic foot wet density may be used. When it is not possible to dewater the existing pipe, a high-density grout with a minimum of 70 pounds per cubic foot will be used which may include approved sand. The foaming agent used will meet the requirements of ASTM C869 when tested in accordance with ASTM C796.

Both of the cellular grout mix designs will be submitted to the SDDOT Concrete Engineer for approval prior to use. The mix design submittal will include the base cement slurry mix per cubic yard, expansion factor from the foaming agent, and the cellular grout wet density (pounds per cubic foot).

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The Contractor will install a bypass valve adjacent to the location where the pressure grouting hose is attached for obtaining samples to be checked for wet density. The wet density of the cellular grout will be checked by the Contractor to verify the proper minimum wet density before the cellular grout filling operations begin and at a minimum once every two hours during production. The SDDOT will document the results of the density checks.

Cellular grout will be wasted until the cellular grout meets the minimum wet density required; however, if 0.5 cubic yards or more of base cement slurry is wasted trying to meet density requirements, then that quantity will not be included for payment.

If grout holes are utilized, cylindrical wooden plugs or other approved plugs will be inserted to plug holes until the grout has set. After the plugs are removed the holes will be filled with concrete.

The quantity of cellular grout was estimated based on volume of the existing pipe and voids outside the existing pipe.

The quantity of base cement slurry ordered will be approved by the Engineer. The quantity of base cement slurry needed will be calculated to the nearest tenth of a cubic yard using the approved mix design, expansion factor of the foaming agent, and estimated amount of cellular grout. The quantity for payment to the nearest tenth of a cubic yard of "Cellular Grout" is a calculated quantity based on the amount of base cement slurry used on the project to the nearest tenth of a cubic yard, expansion factor of the foaming agent, and approved mix design.

All costs for furnishing and installing the cellular grout including bulkhead construction, inlet bevel construction, and incidentals necessary to satisfactorily complete the work will be included in the contract unit price per cubic yard for "Cellular Grout".

#### **TABLE OF CELLULAR GROUT**

<b>05TY</b> Station	Type of Work	Quantity (CuYd)
a 177+75	Plug Cattle Pass	40
a 242+00	Plug Cattle Pass	62.2
b 27+25	Plug Cattle Pass	62.2
	05TY Total:	164.4
06A1		
845+35	Fill Pipe	30.2
	06A1 Total	30.2

The quantity at each location includes an additional 15% to account for void volume outside the existing pipe.

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#### **CONTROLLED DENSITY FILL FOR PIPE**

Controlled density fill will be in conformance with Section 464 of the Specifications.

The controlled density fill will be placed between the pipes from the base of pipe elevation to the haunch of the pipes and extend to the end of the end section.

#### TABLE OF CONTROLLED DENSITY FILL FOR PIPE

<b>06A1</b> Station		Quantity (CuYd)
751+99		17.2
	06A1 Total:	17.2

#### TABLE OF PVC COATED BANK AND CHANNEL PROTECTION GABIONS AND DRAINAGE FABRIC

Station/ MRM	L/R	PVC Coated Bank and Channel Protection Gabion (CuYd)	Type B Drainage Fabric (SqYd)
05TY		(Gara)	(OqTu)
a 114+28	R	4.5	15
a 262+75	R	6.0	19
a 292+80	R	4.5	15
a 296+06	R	4.5	15
a 342+65	R	6.0	19
a 384+07	R	4.5	15
a 403+87	Ĺ	12.0	34
b 104+43	R	12.0	34
05TY T	otal	54.0	166
06A	1		
MRM	R	4.5	15
370.00+0.067			
MRM	R	6.0	19
369.00+0.860			
MRM	L	6.0	19
366.00+0.444			
06A1 To	tal	16.5	53
06RC	;		
b 38+16	L	4.5	15
b 46+24	L	4.5	15
06RC To	otal	9.0	30

#### **CORRUGATED METAL PIPE**

Corrugated metal pipes will have 2 \(^2\_3\)-inch x \(^1\_2\)-inch corrugations for 42-inch and smaller round pipe and 48-inch and smaller arch pipe unless otherwise stated in the plans. Corrugated metal pipes will have 3-inch x 1-inch or 5-inch x 1-inch corrugations for 48-inch and larger round pipe and 54-inch and larger arch pipe unless otherwise stated in the plans.

## FOR BIDDING PURPOSES ONLY DAKOTA

Areas within the project have soils that are highly corrosive to steel Corrugated metal pipe in these areas will be polymer coated 14 gauge steel as specified in the Table of Pipe Quantities. Any required connection bands, elbows, tees, crosses, wyes, reducers, and transitions will also be polymer coated. The connection bands will be 24 inches wide. All polymer coated corrugated metal pipe and components will be in conformance with AASHTO M245. Riveted pipe will not be allowed.

All damage to the polymer coating will be repaired in accordance with the manufacturer's recommendations prior to installation of the pipe.

All costs associated with the polymer coating including repair of polymer coating will be incidental to the corresponding CMP contract items.

Metal pipe end sections connected to polymer coated CMP will be aluminumcoated (Type 2) in accordance with AASHTO M36 as specified in the Table of Pipe Quantities. All costs associated for gauge, coating, and connections will be incidental to the corresponding CMP End Section contract items.

#### MAINLINE CROSS PIPE REPLACEMENT

Pipe culverts will be installed in accordance with the following notes and as shown on the Pipe Installation Detail.

This work will be completed prior to beginning cold milling on the project. Pipe replacements will be completed half-width at a time to maintain traffic.

After the existing pipe has been removed, the new pipe culvert will be undercut to a minimum depth one 1 foot. The depth of undercut is an estimate and the actual depth necessary will be determined during construction. The Engineer will determine how much undercut will be done in accordance with Section 421 of the specifications but will not reduce the undercut to less that 1 foot in depth.

Select fill material for backfilling the undercut will conform to the gradation requirements of Base Course in Section 882. If groundwater is encountered during construction, the select fill material for backfilling the undercut area and Class B bedding will conform to the gradation requirements of Section 421.2 A. until backfill placement is above the groundwater level. The Engineer will process a CCO to provide for compensation to the Contractor for the added cost of the changed material. All other requirements of Section 421 will apply.

Pipe Culverts will be bedded in accordance with Section 450.3 F.2, Class B Bedding with the following exceptions. The excavated area will extend 2 feet from the outermost diameter on both sides of the pipe with the back of the excavated area being sloped 2:1 upward to the top of the roadway surface. Select fill material for Class B Bedding will conform to the gradation requirements of Base Course in section 882.

After the minimum testing requirements of M.S.T.R. Section 4.1F.3.a.1 (SDDOT Materials Manual) have been met, the minimum density testing requirements will be one test per zone. Each zone from the top of the pipe to the top of the subgrade will be 2 feet in depth. Moisture testing will remain as per M.S.T.R.

The remainder of the pipe culvert excavation will be backfilled with soils taken from the pipe removal excavation or other suitable material as approved by the Engineer. The Backfill with be benched into 2:1 excavation slope. Compaction of the backfill material will be governed by the Specified Density Method.

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After the new pipe has been backfilled to the top of the subgrade, a 12" depth of Base Course and 5" (2-2.5" lifts) depth of asphalt concrete composite will be placed as a patch matching the existing asphalt concrete.

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All costs to remove and dispose of asphalt concrete pavement, including full depth saw cutting of the asphalt concrete pavement, will be incidental to the contract until price per square yard to Remove Asphalt Concrete Pavement (See Section F). All excavation necessary for Class B Bedding and the pipe installation will be incidental to the contract unit price per foot for the corresponding pipe installation contract items. Pipe culvert undercut is not anticipated.

The select fill material used for backfilling the pipe culvert undercut and the Class B bedding will be paid for at the contract unit price per ton for Base Course. The 3" layer of bedding material to form the cradle in the pipe foundation will be incidental to the corresponding pipe installation contract items. The cost for asphalt concrete composite installed over the pipe replacement will be paid for at the contract unit price per ton for Asphalt Concrete Composite.

#### REINFORCED CONCRETE PIPE

High sulfate levels are likely to be encountered on this project. The type of cement will be either a Type II or a Type V with 20% to 25% Class F Modified Fly Ash substituted for cement in accordance with Section 605 of the Specifications. The Water/Cementitious material ratio will not exceed 0.45 as defined in Section 460.3 C of the Specifications. The mix will be as per the fabricator's design; however, minimum compressive strength will not be less than 4500 psi at 28 days. The pipe must be marked in an acceptable way to designate meeting requirements for sulfate resistance.

#### **INSLOPE TRANSITIONS**

Inslope transitions will be required at various pipe locations. Refer to Standard Plate 120.05 for details.



#### **REMOVE & RESET PIPE**

The Contractor will tie each section of pipe to the adjacent sections with tie bolts conforming to Standard Plate 450.18. All costs for drilling holes, furnishing, and installing the tie bolt assembly will be incidental to the corresponding pipe bid item.

Existing tie bolts, if any, may be salvaged and reused if condition is acceptable to the Engineer.

#### **INCIDENTAL WORK, GRADING**

PCN/Station	L/R	Remarks
06A1/845+30	R	Remove Ditch Block
05TY, 06A1, 06RC, 06E0 & 07CD	L/R	Minor grading to reestablish drainage – See Pipe Table

#### TEMPORARY EXCAVATION FOR PIPE REPLACEMENT

A temporary 2:1 excavation slope will be required at Station a 114+52 for the replace 30' CMP. The temporary slope will become unstable over the long-term. However, the slope should remain globally stable over the short-term during construction if measures are taken to divert runoff away from the slope and construction activities are sequenced to minimize the amount of time the temporary slope is left exposed and unsupported. Regular monitoring of the temporary slope is required during construction. If the temporary slope becomes unstable, excavation will cease, and the slope will be evaluated by the Engineer.

#### **TABLE OF RIPRAP**

#### 05TY

Station	L/R	Class B Riprap Quantity (Ton)	Type B Drainage Fabric (SqYd)
b 28+13	L	37.3	48
05TY T	otals	37.3	48

#### **GENERAL GEOLOGY**

Materials that will be encountered consist of the Pierre Shale and Pierre Shale derived embankment. The South Dakota Geologic Survey describes the Pierre Shale formation as outlined below:

The Pierre Shale consists of blue-gray, fissile to blocky shale with persistent beds of bentonite, black organic shale, and light-brown chalky shale. Contains minor sandstone, conglomerate, and abundant carbonate and ferruginous concretions.

#### **CLASSIFICATION OF EXCAVATION**

Most of the material encountered should be able to be excavated using conventional methods associated with normal Unclassified Excavation. All materials encountered during the construction of this project, regardless of their nature or the manner in which they are excavated, will be considered Unclassified Excavation.

#### BASE COURSE REINFORCEMENT

Mainline has been distorted by landslide activity around SD 1806 MRM 369.845. This work will be completed prior to beginning cold milling on the project.

Correct the mainline profile by removing the existing surfacing and reconstructing the subgrade from MRM 369.812± to MRM 369.878±. After the asphalt and base course has been removed, undercut the subgrade 3 feet. The undercut will be tapered at 10:1 at each end of the excavation resulting in a full depth excavation from MRM 369.817± to MRM 369.873±. Reconstruct the subgrade and replace the surfacing section.

The base course portion of the surfacing section will be reinforced with geogrid from MRM 369.812± to MRM 369.878± (350 feet). After the subgrade has been rebuilt, 4 inches of base course will be placed and compacted in preparation for geogrid placement. Place biaxial geogrid followed by 8 inches of base course. Place an additional layer of biaxial geogrid followed by the remaining 8 inches of base course. Install base course and geogrid according to the Installation Procedure.

#### Installation Procedure

- 1) Level and compact the first lift of granular material.
- 2) Remove any protrusions that might damage the geogrid prior to placing the geogrid.
- 3) The geogrid can be rolled out parallel to the centerline. The geogrid may be cut and realigned to prevent the propagation of wrinkles as the geogrid is unrolled.
- 4) All seams in the geogrid will be overlapped at least 2 feet and shingled to prevent granular material being forced between the geogrid layers.
- 5) No equipment will be allowed directly on geogrid. The Geogrid must be backfilled with a minimum of 4 inches of granular material before equipment will be allowed to operate the grid from reinforced area.
- 6) The geogrid should be kept as taut as possible prior to backfilling.
- Damaged areas may be repaired by placing additional geogrid over the damaged area. The geogrid patch will cover the damaged area plus 2 feet minimum in all directions as directed by the Engineer.
- 8) Granular material will be dumped at least 20 feet behind the leading edge of the fill and pushed into place with a loader or dozer.
- 9) Granular material will be placed in 4-inch maximum lifts and compacted as per the Specified Density Method.

#### Geogrid Specification

The Geogrid will be biaxial grid of single layer constriction. Vibratory welded, integrally formed or woven and coated geogrids will be acceptable. Grids with laser welded junctions will not be allowed. The geogrid will be certified by the supplier to meet the following specification prior to installation:

Property	Test	MARV
•	ASTM D6637	850 lb/ft MD and XD
Tensile Strength		
(Ultimate)		

Approximately 4,034 square yards of Geogrid will be required. Geogrid will be Paid for at the contract unit price per square yard. Payment quantities will be based on the area covered plus 15%. Overlaps are accounted for by the additional 15%. Payment will be full compensation for furnishing and installing the geogrid only. Granular backfill materials will be paid for under a different item.

### FOR BIDDING PURPOSES ONL

STATE OF	PROJECT	SHEET	TOTAL SHEETS
Y SOUTH DAKOTA	NH 0012(230)171+	B8	B85

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#### LANDSLIDE DEBRIS EXCAVATION (PCN 06RC)

Landslide Debris Excavation will be required at the locations shown on the cross sections. It is anticipated that most of the excavated Landslide Debris can be used in the construction of embankment. The Landslide Debris Excavation limits will not exceed those shown on the cross sections unless directed by the Engineer. A temporary 1.5:1 backslope is required to excavate the Landslide Debris and reconstruct the inslopes. The temporary slope will be unstable over the long-term. However, the slope should remain globally stable over the short-term during construction if measures are taken to divert runoff away from the slope and construction activities are sequenced to minimize the amount of time the temporary backslope is left exposed and unsupported. Regular monitoring of the temporary slope is required during construction. If the temporary slope becomes unstable, excavation will cease, and the slope will be evaluated by the Engineer.

Landslide Debris Excavation will be paid for as Unclassified Excavation.

#### **UNDERDRAINS (PCN 06RC)**

An underdrain will be required to capture water from local seeps and improve subgrade and embankment foundation conditions. The underdrain will be installed as per the following:

An underdrain will be installed at the toe of the temporary Landslide Debris Excavation slope from Station b 25+20±to Station b 28+00±. The underdrain will consist of 4-inch Slotted Corrugated Polyethylene Tubing placed in a 2-foot-wide by 3-foot-deep trench backfilled with 3 feet of Porous Backfill. The underdrain will outlet through 100 feet of 4-inch PVC Outlet Pipe placed in a 2-foot-wide trench of variable depth backfilled with soil. The underdrain outlet pipe will tee into the underdrain at Station b 25+95±and daylight at an Outlet Headwall at approximately Station b 26+05, 219' Rt. as directed by the Engineer.

Estimate of Quantities:

4-inch Corrugated Slotted Polyethylene Tubing	280 feet
4-inch PVC Outlet Pipe	100 feet
Porous Backfill	118 ton
Headwalls (See Standard Plate No. 680.01)	1 each



#### UNDERDRAIN CONSTRUCTION (PCN 06RC)

The 4-inch PVC Outlet Pipe will be Schedule 40 PVC Pipe conforming to ASTM D1785 designed as PVC 1120, PVC 1220, or PVC 2120. Pipe sections will be connected using a PVC Solvent Cement conforming to ASTM D2564. All labor, tools, equipment, and incidentals necessary for the installation of the PVC Outlet Pipe will be incidental to the contract unit price per foot for 4-inch PVC Outlet Pipe.

Care will be taken to ensure that the underdrain and outlet pipes are not damaged during construction. Sufficient cover material is to be placed over the pipes before compaction equipment is allowed over the underdrain system. Damaged pipe will be replaced by the Contractor at no additional cost to the Department.

The underdrain locations are given based on the best information available to the Geotechnical Engineering Activity. Actual field conditions may require that adjustments be made by the Engineer during construction to provide for sufficient drainage. The Geotechnical Engineering Activity will be available for onsite assistance if necessary.

Underdrain trenches will be graded to maintain a minimum of .01ft/ft. or 1% drop from beginning to outlet. The Contractor will ensure all segments of the drainage tubing and outlet pipe are positively connected and remain soil tight during installation of the underdrain system.

Underdrain headwall will be cleared of topsoil, straw, or other debris after seeding operations have been completed. The as built headwall location will be recorded and submitted to the Engineer. Each headwall location will be identified by GPS coordinated and Station and Offset. The headwall location will be cataloged in the Mobridge Area office for reference in post construction maintenance.

#### **EMBANKMENT CONSTRUCTION (PCN 06RC)**

Embankment construction will not begin until all compressible materials have been excavated from the embankment footprint to the satisfaction of the Engineer. A suitable embankment foundation consists of compacted soil which does not pump, rut, or otherwise displace when traveled over with construction equipment. Each embankment will be benched into the existing slopes in accordance with Section 120.3.B.2 of the Specifications.

#### REINFORCED CONCRETE PIPE JOINT REPAIR AND VOID GROUTING

The Contractor will provide a notarized statement, from the Manufacturer, that the products used for culvert joint repair meet the specified requirements, along with the Manufacturer's current product specification and installation instructions.

The Contractor will be an Approved Contractor of the Manufacturer of the specified product and will provide written certification from the Manufacturer attesting to their Approved Contractor status.

All product documentation and Contractor submittals must be submitted to the Engineer prior to or at the preconstruction conference. The Contractor must have the Engineer's approval prior to commencing any of this work.

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The Contractor will follow the Manufacturer's installation instructions and specifications throughout the repair process

Temperature of the specified products is critical from the point of pumping to the point of injection. All polyurethanes react faster at higher temperatures. Drum heaters and heated hoses are required when ambient or ground temperatures are below 70 degrees Fahrenheit. The optimum hose temperature will vary with the weather conditions and the particular job site conditions with the minimum hose temperature being 75 degrees Fahrenheit and the maximum hose temperature being 95 degrees Fahrenheit and the drum temperature not to exceed 90 degrees Fahrenheit.

The Contractor will provide worker and inspector safety protective gear in accordance with the manufacturer, including but not limited to chemical goggles, face shields, eye wash system and NBR gloves.

The Contractor will provide safe storage and handling of materials prior to delivery and at the project site. All material installation, handling and storage will be in accordance with the Manufacturer's recommendations.

The Contractor will visit the project to determine the extent of culvert joints to be cleaned and filled, prior to bidding.

Culvert Joint Cleaning and Repair Culvert Joint quantities will be based upon the following table showing circumference of joints based upon culvert size and shape.

Pipe	Round Pipe	Arch Pipe
Diameter	Circumference per Joint	Circumference per Joint
(ln)	(Ft)	(Ft)
36	9.4	
42	11.0	11.0
48	12.6	
54	14.1	
60	15.7	
66	17.3	
72	18.8	19.0
78	20.4	
84	22.0	

#### **CULVERT JOINT CLEANING**

This work will consist of cleaning of the culvert joints, washing the entire culvert and joints with a high-pressure washer, and if needed, wire brush cleaning of each joint to be repaired as directed by the Engineer. The entire culvert will be clean and dry and most notably the specified joints will be thoroughly cleaned to the satisfaction of the Engineer using a power washer with water pressure of at least 2500 psi. The culvert must be in a clean condition so that no deleterious material is trapped in the joints that are being repaired. The Contractor will dispose of all debris removed from the culverts during the cleaning operation as approved by the Engineer.

All costs for equipment, material and labor for the culvert joint cleaning work will be incidental to the contract unit price per foot for Culvert Joint Cleaning. Culvert Pipe Cleaning will be measured to the nearest 0.1 foot of joint which is cleaned for joint repair.

#### **REPAIR CULVERT JOINT**

The culvert joints will be repaired in accordance with the Chemical Grout Manufacturer's directions to prevent future infiltration/exfiltration of soils and water and to keep the chemical grout from expanding back into the structure during injection.

The culvert joint will be repaired with a sealant comprised of water reactive hydrophilic polyurethane resin and dry oil free oakum. All grout will be injected under such pressure so as not to damage the existing drainage structure or roadway structure.

The Contractor will submit to the Engineer for approval a detailed procedure for the installation of the polyurethane grout.

The work will include, but is not limited to sealing each pipe joint with a hydrophilic polyurethane grout meeting the following specifications:

GEL FOAM II (Saturated Oakum Rope Joint Packing) as manufactured by Green Mountain International, LLC or equal.

ULTRA (Single Component Grout for Joint Injection) as manufactured by Green Mountain International, LLC or equal.

Excess grout and oakum will be trimmed from the interior face of the joint prior to applying the UV Protection (Gel Coat). The epoxy gel coat compound will be as recommended by the Manufacturer for both surface sealing and protecting the hydrophilic grout from UV exposure. The epoxy gel compound will be mixed and handled in accordance with the Manufacturer's recommendations and will meet the following requirements:

Epoxy gel sealant compounds manufactured by Green Mountain Grouts, LLC or equal.

All costs for all equipment, material and labor required to complete the work will be incidental to the contract unit price per foot for Repair Culvert Joint. Completion of the work includes initial saturated oakum rope packing of each joint, follow up injection of grout into the back side of each joint, trimming the excess grout and oakum from the interior face of the joint, application of the epoxy gel coat and site clean-up. Payment will be made per 0.1 foot of culvert joint repaired.



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## REINFORCED CONCRETE PIPE JOINT REPAIR AND VOID GROUTING (CONTINUED)

#### **DUAL COMPONENT CHEMICAL GROUT FOR VOID FILLING**

The external voids surrounding the culvert will be filled with an injected high expansion chemical grout compound. Holes will be strategically drilled as required and grout injected throughout the structure to effectively fill all voids that have developed outside of the structure due to the infiltration of external soils and materials into the culvert and "piping" (water running outside and under the structure due to separated joints). It is the Contractor's responsibility to locate reinforcing bars and conduit prior to drilling any grout holes. All grout will be injected under such pressure so as not to damage the existing drainage structure or roadway structure. All joints will be appropriately cleaned and sealed, with appropriate recommended cure time, prior to the injection of the void grouting. After completion of the void filling, all holes will be properly sealed.

The typical method consists of placing a layer of chemical grout behind or around the structure. The Contractor will submit for approval by the Engineer a detailed grouting plan showing the spacing, orientation and depth of the grout holes, as well as type of polyurethane grout to be used, range of gel times, equipment, mixing procedures, recommended injection pressure, technique for monitoring grout travel and any other pertinent information. The grouting plan should address the prevention of overfilling and prevention of damage to structures or roadway. The Contractor will submit this detailed procedure for the installation of the expansion grout to the Engineer for approval. The holes are drilled with a rotary percussion hammer drill using a sharp masonry bit with a minimum diameter of 3/8 inch to a maximum diameter of 5/8 inch. Care must be taken to prevent holes from causing damage to reinforcing bars or utility conduits. Drilled holes should be vacuumed and flushed. Use injection grout and methods as recommended by Manufacturer.

Injection can be monitored by either applicator's visual inspection or by pumping a specific amount of injection grout into each hole. The work will start at the inlet end of the pipe and proceed downstream to the outlet. Inject bottom row every other hole. When material appears at the adjacent port, discontinue injection at entry port and begin injection at the adjacent port. Continue injection process section by section from bottom of pipe to top of pipe in a continuous manner to next pipe section. Injection pressure will vary from 200 psi to 3000 psi depending on the width of the joint, thickness of the structure, and condition of the concrete.

The Contractor must supply the Engineer with three (3) prior job references of projects where they have successfully injected urethane resin for subgrade void filling applications, or soil stabilization.

- In lieu of three (3) prior job references the Contractor will:
- a) Obtain hands on training from the supplier on the installation Procedures, and
- b) Have the supplier on site to provide training to Contractor's staff. Supplier will be present for at least two complete pipe culvert repairs and until the Engineer is satisfied that Contractor's staff is competent in performing this work.

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The chemical grout will be a dual component hydrophobic polyurethane grout compound which is non-flammable and non-toxic when cured.

The chemical grout mixture will have expansion properties listed in the data sheets of greater than eighteen (18) times its original volume and cure to rigid closed cell polyurethane foam. The grout will expand to fill any voids and must bond to the exterior surface of the structure. The chemical grout will be Mountain Grout U 4.0 dual component polyurethane grouts as manufactured by Green Mountain International LLC or equal.

All costs for equipment, material, and labor required to fill external voids surrounding the culvert will be incidental to the contract unit price per gallon for Chemical Grout Void Fill. Any overfilling of voids that results in damage to overlying pavement, highway user ride quality, or drainage structure integrity will be corrected and paid for by the Contractor. All corrections will be approved by the Engineer. Payment will be to the 0.1 gallon of chemical grout used, prior to expansion of the material.

A calibrated metering device will be used to measure the chemical grout and to assure proper mixing ratio of components.

After the grout cures, excess material will be removed flush with the pipe interior wall and the pipe left clean.

#### **TEMPORARY FENCE**

The Contractor will verify the location of the temporary fence with the landowner prior to installation of the fence.

#### **REMOVE OBJECT MARKERS**

At locations shown in the Table for Mainline Culvert Work, where Object Markers will be removed, cost for removing the existing Object Markers will be paid for the contract unit price per each for Remove Delineator.



### TABLE OF FENCE QUANTITIES

		Remove Fence	Right-of-Way Fence	Fence Panel	Temporary Fence
	Side	rence	Type 2	2 Post	Type 2
Station to Station	(L/R)	Ft	Ft	Each	Ft
			05TY		
a 165+95 to a 166+56	R	61	61	2	140
05TY	Total:	61	61	2	140
			06A1		
845+53 to 845+68	L	16	16	2	233
845+77 to 846+26	L	72	72	2	
06A1	Total:	88	88	4	233
			06E0		
66+42 to 66+87	R	45	45	2	158
66+44 to 66+87	L	43	43	2	154
06E0	Total:	88	88	4	312
			06RC		
b 24+24 to b 30+50	R	515	515	4	1014
b 37+74 to b 38+56	L	82	82	2	115
b 45+79 to b 46+69	L	90	90	2	115
06RC	Total:	687	687	8	1244

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	STATE OF	PROJECT	SHEET	TOTAL SHEETS
1L	Y SOUTH DAKOTA	NH 0012(230)171+	B11	B85

### **TABLE OF GUARDRAIL**

	Remove Beam Guardrail	Type 1 MGS	Type 1 Guardrail Transition	MGS MASH Flared End Terminal
Location	(Ft)	(Ft)	(Ft)	(Each)
	05TY			
Structure No. 16-665-200 (05TY)				
Structure Lt.	250	137.5	2	2
Structure Rt.	250	87.5	2	2
Structure No. 16-666-216 (05TY)				
Structure Lt.	300	137.5	2	2
Structure Rt.	300	137.5	2	2
Structure No. 65-000-020 (05TY)				
Structure Lt.	300	150	2	2
05TY Total:	1400	650	10	10
	07CI	)		
Structure No. 16-580-084 (07CD)				
Structure Lt.	225	100	2	2
Structure Rt.	225	87.5	2	2
Structure No. 16-580-075 (07CD)				
Structure Lt.	230	87.5	2	2
Structure Rt.	230	87.5	2	2
07CD Total:	910	362.5	8	8
	06A <sup>2</sup>	1		
Structure No. 16-737-253 (06A1)				
Structure Lt.	160	50	2	2
Structure Rt.	160	50	2	2
Structure No. 16-732-234 (06A1)				
Structure Lt.	200	50	2	2
Structure Rt.	200	50	2	2
Structure No. 16-720-217 (06A1)				
Structure Lt.	160	50	2	2
Structure Rt.	160	50	2	2
06A1 Total:	1040	300	12	12



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Revised: 10/8/24 EJW

# TABLE OF CONSTRUCTION STAKING FOR PROJECTS NH 0012(230)171, P 1806(19)364, P 1806(20)370, P 1806(22)359, & P 0063(59)252

(See Special Provision for Contractor Staking)

Roadway and Description	Begin Station	End Station	Number of Lanes	Length (Ft)	Length (Mile)	Lane Factor	*Sets of Stakes	Construction Staking Quantity	Slope Staking Quantity	** Grade Staking Quantity	Misc. Staking Quantity
								(Mile)	(Mile)	(Mile)	(Mile)
				05TY							
US 12 (2 Lanes AC Pavement)	176+72	249+37	2	7265	1.376	1	0	1.376			1.376
US 12 (2 Lanes AC Pavement)	254+93	278+05	2	2313	0.438	1	0	0.438			0.438
US 12 (3 Lanes AC Pavement)	a 0+00	a 17+00	3	1700	0.322	1.5	0	0.483			0.483
US 12 (4 Lanes AC Pavement)	a 17+00	a 30+00	4	1300	0.246	2	0	0.492			0.492
US 12 (3 Lanes AC Pavement)	a 30+00	a 47+00	3	1,700	0.322	1.5	0	0.483			0.483
US 12 (2 Lanes AC Pavement)	a 47+00	a 55+28	2	828	0.157	1	0	0.157			0.157
US 12 (2 Lanes AC Pavement)	a 57+12	a 64+00	2	688	0.130	1	0	0.130			0.130
US 12 (3 Lanes AC Pavement)	a 64+00	a 79+00	3	1,500	0.284	1.5	0	0.426			0.426
US 12 (4 Lanes AC Pavement)	a 79+00	a 97+00	4	1800	0.341	2	0	0.682			0.682
US 12 (3 Lanes AC Pavement)	a 97+00	a 119+00	3	2200	0.417	1.5	0	0.625			0.625
US 12 (2 Lanes AC Pavement)	a 119+00	a 218+00	2	9900	1.875	1	0	1.875			1.875
US 12 (3 Lanes AC Pavement)	a 218+00	a 242+00	3	2400	0.455	1.5	0	0.682			0.682
US 12 (2 Lanes AC Pavement)	a 242+00	a 262+00	2	2000	0.379	1	0	0.379			0.379
US 12 (3 Lanes AC Pavement)	a 262+00	a 299+50	3	3750	0.710	1.5	0	1.065			1.065
US 12 (2 Lanes AC Pavement)	a 299+50	a 327+17	2	2767	0.524	1	0	0.524			0.524
US 12 (2 Lanes AC Pavement)	a 328+63	a 339+00	2	37	0.007	1	0	0.007			0.007
US 12 (3 Lanes AC Pavement)	a 339+00	a 404+48.8	3	6548.8	1.240	1.5	0	1.860			1.860
US 12 (4 Lanes AC Pavement)	b 20+40	b 23+71	4	331	0.063	2	0	0.125			0.125
US 12 (2 Lanes AC Pavement)	b 23+71	b 82+00	2	5829	1.104	1	0	1.104			1.104
US 12 (3 Lanes AC Pavement)	b 82+00	b 115+78	3	3378	0.640	1.5	0	0.960			0.960
US 12 (2 Lanes AC Pavement)	b 208+31	b 240+05	2	3174	0.601	1	0	0.601			0.601
US 12 (3 Lanes AC Pavement)	b 240+05	b 253+09	3	1304	0.247	1.5	0	0.370			0.370
US 12 (2 Lanes AC Pavement)	b 253+09	b 262+02	2	893	0.169	1	0	0.169			0.169
US 12/SD 20 Intersection Widening	a 397+86	b 34+65		2088			0		0.395		
US 12/SD 1806 Intersection Widening	b 238+00	b 245+00		700			0		0.133		
							05TY Subtotals:	15.015	0.528		15.015
				06A1							
SD1806 (2 Lanes AC Pavement)	749+64.10	c 0+00	2	36217.95	6.859	1		6.859			6.859
Heave Repair	1018+13	1031+33		1320		1	1		0.250	0.250	
Base Course Reinforcement	885+25	881+76		349		1	1		0.066	0.066	
							06A1 Subtotals:	6.859	0.316	0.316	6.859
				06RC							
SD1806 (2 Lanes AC Pavement)	67+11.74	b 229+74.50	2	18763.44	3.554	1		3.554			3.554
Landslide Repair	b 24+40	b 30+00		560		1	1		0.106	0.106	
							06RC Subtotals:	3.554	0.106	0.106	3.554
				06E0							
SD1806P (2 Lanes AC Pavement)	0+00	87+91.22	2	8791.22	1.665	1		1.665			1.665
							06E0 Subtotals:	1.665			1.665
				07CD							
SD63 (2 Lanes AC Pavement)	288+47	390+50	2	9850.89	1.866	1		1.866			1.866
·							07CD Subtotals:	1.866			1.866
							Total:	28.959	0.950	0.422	28.959

- \* 1 = Top of Granular Material Blue Top Stakes Only (Asphalt Concrete Pavement)
- \*\* Grade Staking Quantity = (Length) x (Lane Factor) x (Sets of Stakes)



# TABLE OF PIPE QUANTIFICES ING PURPOSES ONL STATE OF DAKOTA

## 05TY

						Per Origi	nal Plans	Remo Re	ve for set	Rese	t	ı	Remov	e		F	Pipe			ed End		Flared E	nd Secti	ions			Furnished	ion	de	Fabric	Work,		Marker	
ert #						In Place Culvert Size and Type		Pipe	End Section	Pipe	End Section	Pipe	End Section	Cattle Pass End Section	24"CMP	24" RCP	48" RCP	54" RCP	24" CMP	24" RCP	18" RCP	36" RCP	48" RCP	54" RCP	Cleanout Pipe Culvert	Cellular Grout	- E	B&C Protection Gabions	Class B Riprap	Type B Drainage	Incidental Wo	Remove	New Back to Back	Repair Comments
Culv	HWY	MRM	Disp	Station	Side			(Ft)	(Each)	(Ft) (E	ach)	(Ft)	(Each)	(Each)	(Ft)	(Ft)	(Ft)	(Ft)	(Each)	(Each	(Each	) (Each	) (Eacl	n) (Each)	(Each)	(CuYd)	(CuYd)	(CuYd)		(SqYd	) (LS)	(Each)	(Each)	
48	US 12	172	0.003	811+20	Lt Rt	24" CMP	Flared Flared																			1						1 1	1 1	-
49	US 12	172	0.003	180+97	Lt	18" RCP	Flared	8	1	8	1																				Х		1	_
					Rt Lt		Flared Flared		1		1		1								1					1							1	
50	US 12	172	0.061	183+98	Rt	18" RCP	Flared						1								1												1	
51	US 12	172	0.132	188+00	Lt Rt	18" RCP	Flared Flared	24		24			1								1				1	-				-			1 1	-
F2	UC 12	172	0.216	107.55	Lt	10" DCD	Flared	24		24			1								1												1	
52	US 12	172	0.316	197+55	Rt	18" RCP	Flared														1												1	1
53	US 12	172	0.389	201+60	Lt Rt	24" RCP	Flared Flared	8 16		8 16			1							1													1 1	-
54	US 12	172	0.482	206+27	Lt	24" RCP	Flared	8		8			1							1													1	_
					Rt Lt		Flared Flared	16		16			1							1					1							1	2	
55	US 12	172	0.681	217+10	Rt	7' x 7' RCBC	Flared																		1							1	2	1
56	US 12	174	0.062	a 12+00	Lt	24" RCP	Flared		1		1																						1	-
					Rt Lt		Flared Flared		1		1																						1	
57	US 12	174	0.591	a 40+00	Rt	18" RCP	Flared		1		1																						1	1
58	US 12	175	0.130	a 67+72	Lt Rt	30" RCP	Flared Flared	8	1		1																				Х		1 1	Right reset FES and re-grade around to achieve drainage
59	US 12	175	0.239	a 73+00	Lt	18" RCP	Flared		1		1																				Х		1	Reset FES and re-grade to
39	03 12	1/3	0.239	a 73+00	Rt	10 NCF	Flared		1		1											_				-				_			1	achieve drainage
60	US 12	175	0.853	a 105+75	Lt Rt	18" RCP	Flared Flared	8	1		1														1								1	-
61	US 12	176	0.039	a 114+28	Lt	24" CMP	Flared																										1	
					Rt I†		Flared Flared	8	1	8	1	30	1		30				1	-						+		4.5		15		1	1 1	- HIMMING ESS/ONLY
62	US 12	176	0.210	a 124+00	Rt	24" RCP	Flared	8	1		1	寸																					1	REG. NO.
63	US 12	176	0.434	a 135+00	Lt Rt	7' x 7' RCBC	Wingwalls			_		$\dashv$						-	-	-										-		1	2	PAUL AZ ZO
	LIC 42	170	0.022	- 101:55	I t	24" 505	Wingwalls Flared	8	1	8	1	$\dashv$							+							1						1	1	MNOFCZYNSKI
64	US 12	1/6	0.933	a 161+55	Rt	24" RCP	Flared	8	1	8	1																						1	THE WAY STATE
65	US 12	177	0.027	a 165+90	Lt Rt	24" RCP	Flared Flared	16	1	16	1	$\dashv$	1						$\vdash$	1						1				-		1	1 1	PAUL A. PAUL A
66	US 12	177	0.245	a 177+50	Lt	24" RCP	Flared		1		1		_																				1	
	03 12	1,,	5.245	4 177130	Rt I+	24 1101	Flared		1	-	1	_		1				-	-	-						-				-		1	1	
67	US 12	177	0.252	a 177+75	Lt Rt	4'x6' RCP	Flared Flared					$\dashv$		1					+							40	22					1		Plug Cattle Pass
			PROJEC	T 05TY SUB	BTOTALS			152	19	152	19	30	11	2	30	0	0	0	1	5	6	0	0	0	5	40	22	4.5		15	0	4	42	

# TABLE OF PIPE QUANTITUES ING PURPOSES ONL STATE OF DAKOTA

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## 05TY

						Per Origi	nal Plans		ove for eset	Re	eset		Remov	e		P	ipe		Slope Sect		FI	ared End	l Section	ıs			ished	ioi	œ.	Fabric	ork,		Marker	
Culvert #						In Place Culvert Size and Type		Pipe	End Section	Pipe	End Section	Pipe	End Section	Cattle Pass End Section	24"CMP	24" RCP	48" RCP	54" RCP	24" CMP	24" RCP	18" RCP	36" RCP	48" RCP	54" RCP	Cleanout Pipe Culvert	Cellular Grout	Contractor	B&C Protection Gabions	Class B Riprap	Type B Drainage	Incidental Wo Grading	Remove Delineator	New Back to Back	Repair Comments
3	HWY	MRM	Disp	Station	Side			(Ft)	(Each)	(Ft)	(Each)	(Ft)	(Each)	(Each)	(Ft)	(Ft)	(Ft)	(Ft)	(Each)	(Each)	(Each)	(Each)	(Each)	(Each)	(Each)	(CuYd)	(CuYd)	(CuYd)		(SqYd)	(LS)	(Each)	(Each)	
8	US 12	177	0.378	a 185+65	Lt	24" RCP	Flared	8	1	8	1																					—	1	
+					Rt		Flared	8	1	8	1																					$\vdash$	1	
1	US 12	177	0.534	a 194+00	Lt Rt	24" RCP	Flared Flared	16		16			1							1												$\vdash$	1	
+					Lt		Flared	16	1	16	1									1												$\vdash$	1	
	US 12	177	0.742	a 205+20	Rt	24" RCP	Flared	8	1	8	1																					$\vdash$	1	
1				215 22	Lt	1011 000	Flared		1		1																					<u> </u>	2	
	US 12	177	0.983	a 216+00	Rt	42" RCP	Flared		1		1																						2	
	US 12	178	0.420	a 241+91	Lt	36" RCP	Flared	32	1	32	1																							
	U3 12	1/8	0.439	a 241+91	Rt	SU KLP	Flared		1		1																							
	US 12	178	0.451	a 242+00	Lt	4'x6' RCP	Flared							1												62.2	42					—		Plug Cattle Pass
4				3 = 12 100	Rt		Flared							1												72.2						<del></del>		
.	US 12	178	0.529	a 245+00	Lt	24" CMP	Flared																									—		
+					Rt		Flared															_										₩	1	
,	US 12	178	0.879	a 262+75	Lt Rt	36" RCP	None None	8		8			1									1						6.0		10		$\vdash$	2	
+					Lt		Flared						1									1						6.0		19		$\vdash$	2	
5	US 12	179	0.012	a 267+15	Rt	24" CMP	Flared																									$\vdash$	1	
十					Lt		Flared						1						1													<del>                                     </del>	1	
7	US 12	179	0.370	a 287+56	Rt	24" CMP	Flared												_													<u> </u>		
		470		222.22	Lt	2.411.02.45	Flared																										1	
8	US 12	179	0.491	a 292+80	Rt	24" CMP	Flared																					4.5		15				
9	US 12	179	0.501	a 298+06	Lt	24" CMP	Flared																		1								1	
	03 12	173	0.531	a 238+00	Rt	24 CIVIF	Flared		1		1																	4.5		15		<u> </u>		
	US 12	179	0.659	a 301+75	Lt	24" CMP	Flared																								Х	—	1	Left regrade around FES
+					Rt		Flared																		1							—	1	
	US 12	179	0.729	a 306+35	Lt	24" RCP	Flared	8	1	8	1																					—	1	
+					Rt		Flared																		1							<del></del>	1	
	US 12	179	0.945	a 316+70	Lt Rt	Twin 9' x 7' RCBC	Wingwalls Wingwalls	_																								1	2	
+					Lt		Flared	1	1	$\parallel \parallel$	1				1																Х	<del>                                     </del>	2	
3	US 12	180.08	0.255	a 342+65	Rt	30" RCP	Flared	8	1	8	1																	6.0		19	^	$\vdash$	1	LT reset FES and regrad
十		_			Lt		Flared		<u> </u>																			0.0		-		$\vdash$	1	
4	US 12	180.08	0.425	a 351+50	Rt	24" CMP	Flared								1 1																		1	
_	UC 13	100.00	0.570	- 250:75	Lt	24" 6545	Flared																										1	, miniming
5	US 12	180.08	0.578	a 359+75	Rt	24" CMP	Flared																		1									- HILL ROFESS/
5	US 12	180 09	0.589	a 363+85	Lt	24" CMP	Flared						_																					REG. NO.
$\perp$	03 12	100.00	0.363	u 303+03	Rt	Z- CIVIF	Flared																									—		PAUL A. KNOFCZYNS  ASOLITANIA
,	US 12	180.08	0.639	a 366+50	Lt	24" RCP	Flared	16	1	16															1							ــــــ	1	PAUL A.
$\perp$					Rt		Flared	16	1	16															1								1	- I S NIVOFCOMS
			PROJEC	T 05TY SUB	TOTALS			144	14	144	14	0	4	2	0	0	0	0	1	1	0	2	0	0	6	62.2	42	21		68	0	2	33	- The Williams

# TABLE OF PIPE QUANTITUES ING PURPOSES ONLY SOUTH DAKOTA

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## 05TY

						Per Origii	nal Plans	Remo Re	ve for	Re	eset		Remove			Pi	ipe		Slope	ed End tions	F	lared En	d Section	ns			nished	tion	de	• Fabric	Work, ng	Obj. N	/larker	
ert #						In Place Culvert Size and Type	End Type	Pipe	End Section	Pipe	End Section	Pipe	End Section	Cattle Pass End Section	24"CMP	24" RCP	48" RCP	54" RCP	24" CMP	24" RCP	18" RCP	36" RCP	48" RCP	54" RCP	Cleanout Pipe Culvert	Cellular Grout	Contractor Furnished Borrow	B&C Protection Gabions	Class B Riprap	Type B Drainage	Incidental Wo Grading	Remove Delineator	New Back to Back	Repair Comments
Culvert	HWY	MRM	Disp	Station	Side			(Ft)	(Each)	(Ft)	(Each)	(Ft)	(Each) (	Each) (	Ft)	(Ft)	(Ft)	(Ft)	(Each)	(Each)	(Each)	(Each)	(Each)	(Each)	(Each)	(CuYd)	(CuYd)	(CuYd)		(SqYd)	(LS)	(Each)	(Each)	
88	US 12	180.08	0.865	a 375+00	Lt Rt	24" RCP	Flared Flared	8	1	8	1														1								1	
89	US 12	181	0.132	a 384+07	Lt	24" CMP	Flared				_														1						Х		1	
					Rt		Flared		1		1																	4.5		15	Х		1	
90	US 12	181.52	0.014	a 403+87	Lt	54" RCP	Flared	24		24			1											1				12.0		34			2	
			-		Rt		Flared	24	1	24	1		-		-																		2	
91	US 12	181.52	0.088	b 27+25	Lt Rt	4'x6' RCP	Flared Flared							1												62.2	42							Plug Cattle Pass
92	US 12	101 52	0.107	b 29+10	I t	54" RCP	None											8						1			15		37.3	48	Х		1	Extend Pipe For Roadway
92	US 12	181.52	0.197	0 29+10	Rt	54 KCP	None	8		8														1							Х		1	Widening
93	US 12	182	0.056	b 50+50	Lt	18" RCP	Flared		1		1																						1	
					Rt		Flared	8	1	8	1																						1	
94	US 12	182	0.096	b 57+50	Lt	18" RCP	Flared	16		16			1								1												1	
					Rt		Flared None	_					1								1												1	
95	US 12	182	0.358	b 71+15	Lt Rt	48" RCP	None	8		8		_		-		+							1										2	
					Lt		Flared					8					8						1								Х		2	
96	US 12	182	0.553	b 82+00	Rt	30" RCP	Flared																									1	1	
					I t		Flared			$\dagger$					+										1						Х			
97	US 12	182	1.000	b 104+43	Rt	54" RCP	Flared		1		1																	12.0		34	X			
00	110 12	102	0.055	h 100 i C F	Lt	24" RCP	None																		1								1	
98	US 12	183	0.055	b 108+65	Rt	Z4 KCP	None				-	8	1			8				1													1	
			PROJE	CT 05TY SU	BTOTALS			96	7	96	7	16	4	2	0	8	8	8	0	1	2	0	2	3	5	62.2	57	28.5	37.3	131	LS	1	22	
			PRO.	ECT 05TY T	OTALS			392	40	392	40	46	19	6	30	8	8	8	2	7	8	2	2	3	16	164.4	121	54	37.3	214	LS	7	97	



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## 06A1

Parish   P									Remove									<del></del>		Flared					=	<b>-</b>		Ι_			
Math							Per Original F		for	Reset	Rem	ove	ı	Pipe		Slo	oped E	ind	Safety Ends	End	Cleanout		n Gabion	ge Fabric	nsity Fi	ert Join	rt Joint	ut Void	Vork, 8	Obj. Marker	
1	ulvert #	LIMOV	MDM	Dien	Station	Sido			End			End					24"	24" RCP	18"	36"	Pipe Culvert	Grout	B&C	Туре							Repair Comments
1		HWY	IVIKIVI	DISP	Station			Flared	(Lacii)	(Eacil)	(FL)		(Ft)			icii) (L	Laciij	(Lacii)	(Lacii)	1	(Eacil)	(curu)	(Curu)	(SqTu)	(Curu)	(FL)	(FL)	(Gai)	(L3)		
180	14	1806	372	0.255	751+96		54" CMP Arch				74									1					17.2						†
1806   1806								<del>                                     </del>							30						1									-	
180	15	1806	371	0.916	770+00		24" CMP	<b>—</b>																						-	1
1																													Х	-	
1806   371   180	16	1806	371	0.805	775+85		24" CMP																							<u> </u>	1
1806   37												1				1															
180   371   390   797-00   Rt   24°CMP   Flared	17	1806	371	0.425	797+00	Rt	24" CMP					1				1															1
180		4000				Lt	2 4 11 22 42					1				1														1	
1806   371   370	18	1806	3/1	0.390	/9/+00	Rt	24" CMP	Flared				1				1														1	1
Part	10	1906	271	0.272	700.70	Lt	60" CMD	Flared																						2	
1806   371   0.35   80-84   Rt   24" CMP   Flared	19	1800	3/1	0.572	799+70	Rt	60 CIVIP	Flared																						2	
The color of the	20	1806	271	0.350	800+84	Lt	24" CMP	Flared													1									1	
1806   371   370	20	1800	3/1	0.550	800184	Rt	Z4 CIVII	Flared																							
Parison   Pari	21	1806	371	0.308	803+00	Lt	5'x7' RCP	Flared																		240	240	12		2	<b>-</b>
1806   371   0.48   804-25   Rt   24" CMP   Flared						Rt		_																						2	foam/fill joints with sealant. See notes.
Table   Tabl	22	1806	371	0.287	804+25		24" CMP	_				1				1															<u> </u>
1806   371   370   370.35																					1										
24         1806         370.35         0.52         826+00         Lt Rt         24"CMP         Flared         IIIIIIIIIIIIIIIIIIIIIIIIIIIIIIIIIIII	23	1806	371	0.140	812+00		24" CMP																								4
1806   370.35   0.52   826+00   Rt   24" CMP   Flared								1								_				-	1									1	
25         1806         370.35         0.384         83+50         Lt Rt         24" CMP         Flared         IIIIIIIIIIIIIIIIIIIIIIIIIIIIIIIIIIII	24	1806	370.35	0.522	826+00		24" CMP	<b>—</b>																							-
1806   370.35   0.384   833+50   Rt   24"CMP   Flared								_																							
26         1806         370.35         0.180         846+33 to 846+74         It 81° CMP         Sloped         Image: Control of the state	25	1806	370.35	0.384	833+50		24" CMP													<u> </u>											-
1806   370.35   0.180   846+74   Rt					946+22 to			_					64						2		1										
26 1806 370.35 0.180 846+51 Lt Rt 24" RCP Sloped 30 1	26	1806	370.35	0.180			18" CMP						04			_														-	
26   1806   370.35   0.180   846+51   Rt   24" RCP   Sloped   30   1   1   1   1   1   1   1   1   1								_	+					30			1							1							
26 1806 370.35 0.180 845+35 Lt Rt 18" RCP Flared 8 2 30.2 30.2 30.2 X 1	26	1806	370.35	0.180	846+51		24" RCP									_	-						1								
26   1806   370.35   0.180   845+35   Rt   18" RCP   Flared   8   2																													Х		
	26	1806	370.35	0.180	845+35		18" RCP				8	2										30.2		1					1	_	1
			P	ROJECT 0	6A1 SUBTOT			<b>-</b>	0	0	82	9	64	60 :	100	5	2	0	2	4	8	30.2	0	0	17.2	240	240	12			



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						Per Original F	Plans	Remove for Reset	Reset	Ren	nove		Pipe		s	loped I	End	Safety Ends	Flared End Section	Cleanout		າ Gabion	ge Fabric	nsity Fill	Culvert Joint	t Joint	ut Void	Vork, B	Obj. M	larker	
Culvert #						In Place Culvert Size and Type	End Type	End Section	End Section	Pipe	End Section	18"CMP		36"	24" CMP	24" RCP	24" RCP Arch	18" CMP	36" RCP	Pipe Culvert	Cellular Grout	B&C Protection	Type B Drainage	Controlled Density Fill	Cleanout	Repair Culvert	Chemical Grout	Incidental Wo Grading		New Back to Back	Repair Comments
3	HWY	MRM	Disp	Station	Side			(Each)	(Each)	(Ft)	(Each)	(Ft)	(Ft)	(Ft) (I	Each)	(Each)	(Each)	(Each)	(Each)	(Each)	(CuYd)	(CuYd)	(SqYd)	(CuYd)	(Ft)	(Ft)	(Gal)	(LS)	(Each)	(Each)	
27	1806	370	0.067		Lt	24" RCP	None																				1			1	Right Install scour protection and re-grade
					Rt		None															4.5	15					Х			around FES.
28	1806	369	0.860		Lt	36" CMP	Flared													1										2	Right install scour protection around
					Rt		None															6.0	19					Х		1	outlet end.
29	1806	369	0.704	a 687+46	Lt	24" RCP	None													1										1	
					Rt		None													1							1			1	
30	1806	369	0.229		Lt	48" RCP	Flared													1							1		1	2	
					Rt		Flared																				ļ		1	2	
31	1806	369	0.044	a 923+00	Lt	24" RCP	Flared													1										1	
					Rt		Flared																							1	
32	1806	368	0.500	a 950+00	Lt	24" RCP Arch	Flared									-				1							1			1	
					Rt		Flared													1										1	
33	1806	368	0.191	a 965+00	Lt	24" RCP Arch	Flared													1										1	
					Rt		Flared				1					1	1			1										1	
34	1806	367.64	0.172	a 988+35	Lt Rt	24" RCP	Flared Flared				1					1											1			1	
					Lt		Flared													1		6.0	10					Х		1	Left end section install scour protection
35	1806	366	0.444	b 1076+23		30" CMP														1		6.0	19				1				and re-grade for proper drainage. FES are
				1070+23	Rt		Flared													1											located past fence line.
36	1806	366.00	0.293	b	Lt	24" CMP	Flared																							1	
				1068+26	Rt		Flared																								
37	1806	365.72	0.266	b	Lt	18" RCP	Flared													1										1	
				1084+80	Rt		Flared																							1	
38	1806	365.72	0.057	b 1095+25	Lt	24" RCP	Flared	1	1											1								Х		1	Regrade left outlet
			DO 1507 33		Rt		Flared													1		46.5						1.0		1	
				SA1 SUBTOT				1	1	0	2	0	0	0	0	1	1	0	0	14	0	16.5	53	0	0	0	0	LS	2	23	
			PROJECT	06A1 TOTAI	.5			1	1	82	11	64	60	100	5	3	1	2	4	22	30.2	16.5	53	17.2	240	240	12	LS	4	53	



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## 06RC

						Per Origina	l Plans	Rem	ove	Pi <sub>l</sub>	ре		Sloped End	ls	lbow	CMP	Cleanout	ction	Drainage Ibric	Nork,	ack rker	
# # t						In Place Culvert Size and Type	End Type	Pipe	End Section	24"CMP	24" RCP	18" CMP	24" CMP	24" RCP	24" CMP Elbow	24" RCP to CI Transition	Pipe Culvert	B&C Protection Gabion	Type B Drain Fabric	Incidental Work, Grading	Back to Back Object Marke	Repair Comments
Culvert	HWY	MRM	Disp	Station	Side			(Ft)	(Each)	(Ft)	(Ft)	(Each)	(Each)	(Each)	(Each)	(Each)	(Each)	(CuYd)	(SqYd)	(LS)	(Each)	
6	1806	359.75	0.057		Lt	24" CMP	Flared										1				1	
	1800	333.73	0.037		Rt	24 CIVII	Flared										1				1	
7	1806	360	0.411		Lt	18" CMP	Flared										1				1	
,	1800	300	0.411		Rt	18 CIVIF	Flared		1			1									1	
8	1806	360	0.600	a 184+20	Lt	24" CMP	Flared	62	1		27			1							1	Replace with 24" RCP
8	1800	300	0.000	a 104+20	Rt	24 CIVIF	Flared	02	1		33			1							1	Replace with 24 RCF
9	1806	361	0.120		Lt	24" CMP	Flared														1	Re-grade around right
9	1800	301	0.120		Rt	24 CIVIP	Flared													Х	1	FES.
10	1806	361	0.574		Lt	24" CMP	Flared														1	
10	1800	301	0.574		Rt	24 CIVIP	Flared														1	
11	1806	361	0.924		Lt	78"x60" CMP	Sloped										1			Х	2	Do grado left outlet
11	1806	201	0.924		Rt	78 XOU CIVIP	Sloped										1				2	Re-grade left outlet
12	1906	262	0.712	b 38+16	Lt	18" CMP	Flared	82	1	26	24		1		2	1		4.5	15		1	Romaya and Ranlaca Dina
12	1806	362	0.712	n 20+10	Rt	19 CIVIP	Flared	02	1		38			1							1	Remove and Replace Pipe
12	1006	262	0.966	h 4C + 24	Lt	10" CNAD	Flared	82	1	24	39		1		2	1		4.5	15		1	Domeyo and Donlags Dire
13	1806	362	0.866	b 46+24	Rt	18" CMP	Flared	82	1		25			1							1	Remove and Replace Pipe
			PROJE	CT 06RC TO	TALS			226	7	50	186	1	2	4	4	2	5	9	30	LS	18	



# TABLE OF PIPE QUANTITUES ING PURPOSES ONLY SOUTH DAKOTA

OF PROJECT
TH NH 0012(230)171...+

SHEET TOTAL SHEETS
B19 B85

## 06E0

						Per Original	Plans		ve for set	Re	eset	Remove	Flared End Section	Classist	Work,	livert	Culvert	rout	Obj. M	arker	
Culvert #						In Place Culvert Size and Type	End Type	Pipe	End Section	Pipe	End Section	End Section	36" RCP	Cleanout Pipe Culvert	Incidental Wo	Cleanout Culvert Joints	Repair Cul	Chemical Grout Void Fill	Remove	New Back to Back	Repair Comments
3	HWY	MRM	Disp	Station	Side			(Ft)	(Each)	(Ft)	(Each)	(Each)	(Each)	(Each)	(LS)	(Ft)	(Ft)		(Each)	(Each)	
39	1806P	370.13	0.076	83+50±	Lt	72" RCP	Flared							1							Cleanout pipe
39	10007	370.13	0.070	63+30 <u>+</u>	Rt	72 RCF	Flared							1							Creanout pipe
40	1806P	370.13	0.133	80+80±	Lt	30" RCP	Flared							1						1	
40	10001	370.13	0.133	80+80±	Rt	30 NCF	Flared													1	
41	1806P	370.13	0.401	66+65	Lt	84" RCP	Flared									198	198	12		2	See notes, Expansion foam/fill joints
41	18001	370.13	0.401	00103	Rt	84 NCI	Flared									130	138	12		2	with sealant
42	1806P	370.13	0.665	54+00	Lt	36" RCP	Flared							1						2	
72	10001	370.13	0.005	34.00	Rt	30 1101	Flared							1						2	
43	1806P	370.13	0.869	42+00	Lt	36" RCP	Flared	8	1	8	1								1	2	
	1000.	070.20		12700	Rt	33	Flared					1	1						1	2	
44	1806P	371.00	0.119	36+00	Lt	36" RCP	Flared	16	1	16	1								1	2	
					Rt		Flared	16	1	16	1								1	2	
45	1806P	371.00	0.171	32+67	Lt	36" RCP	Flared													2	
			-		Rt		Flared												1	2	
46	1806P	371.00	0.448	18+80	Lt	2-30" RCP Arch	Flared							1	Х					2	Regrade to achieve drainage
		0.1.00			Rt		Flared							1	Х					2	
47	1806P	371.00	0.751	2+68	Lt	2-36" RCP Arch	Flared							1	Х					2	Cleanout and re-grade around both
.,		3, 2.00			Rt	_ 33	Flared							1	Х					2	ends.
			PROJECT	06E0 TOTAL	LS			40	3	40	3	1	1	9	LS	198	198	12	5	30	



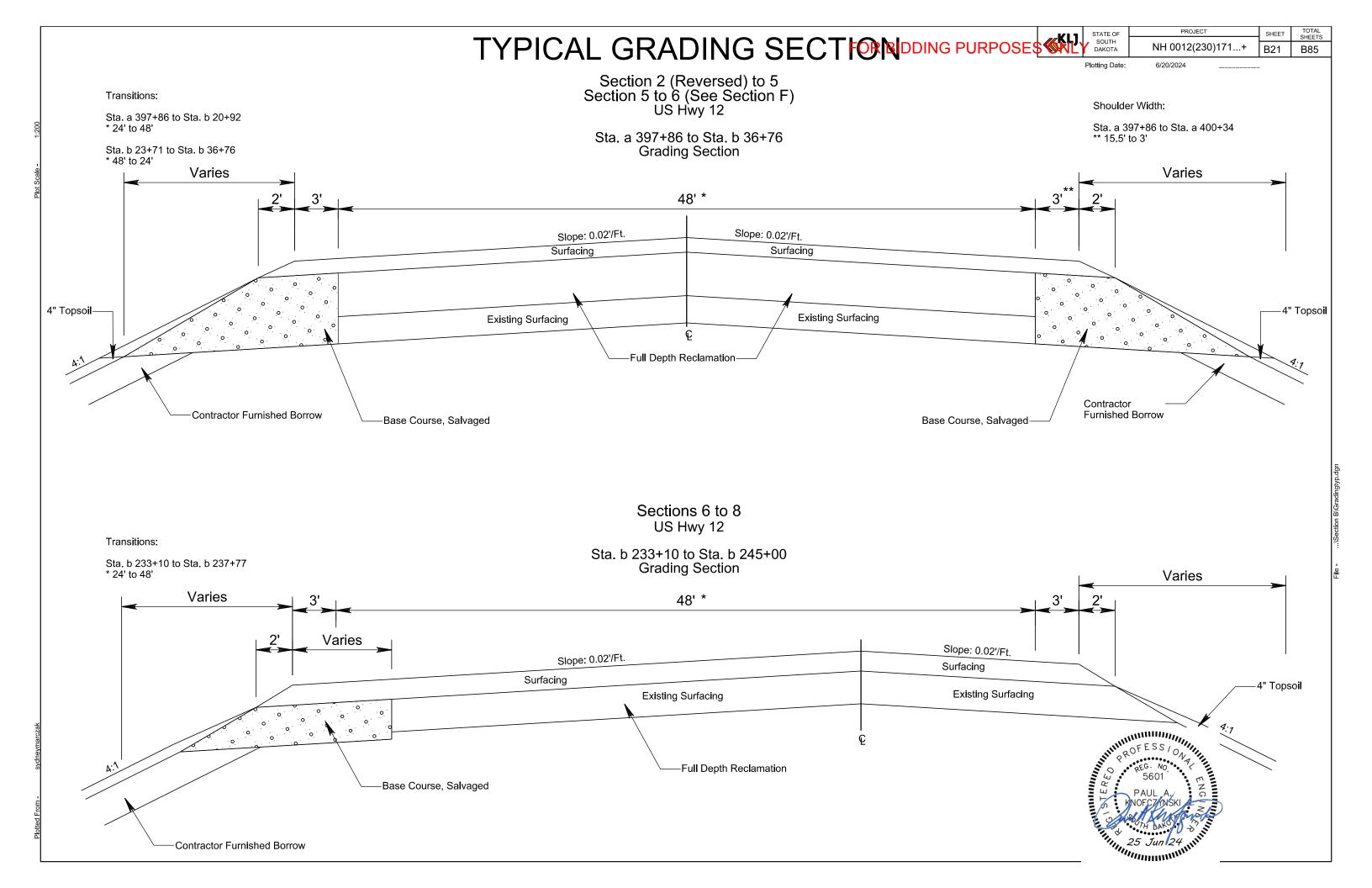
# TABLE OF PIPE QUANTITUES ING PURPOSES ONL STATE OF DAKOTA

TE OF PROJECT SHEET UTH NH 0012(230)171...+ B20

## 07CD

						Per Original Pl	ans	Cleanout	Work,	Obj. Marker	
Culvert #						In Place Culvert Size and Type	End Type	Pipe Culvert	Incidental Work, Grading	New Back to Back	Repair Comments
Cul	HWY	MRM	Disp	Station	Side			(Each)	(LS)	(Each)	
1	063	252	0.606	424+00	Lt	24" RCP	Flared	1		1	
1	003	232	0.000	424+00	Rt	24 KCP	Flared	1		1	
2	063	252.78	0.139	412+60	Lt	24" RCP	Flared	1		1	
2	003	232.76	0.139	412+00	Rt	Z4 KCF	Flared	1		1	
3	063	253	0.104	397+70	Lt	24" CMP	Flared			1	RT re-grade side of pipe flow
3	003	233	0.104	397+70	Rt	24 CIVIP	Flared		1	1	line for adequate drainage.
4	063	253	0.47	378+10	Lt	24" RCP	Flared	1		1	
4	003	255	0.47	376+10	Rt	24 KCP	Flared	1		1	
_	063	252.6	0.151	363,00	Lt	36" BCD	Flared	1		2	
5	003	253.6	0.151	363+00	Rt	36" RCP	Flared	1		2	
			PROJ	ECT 07CD TO	TALS			8	LS	12	

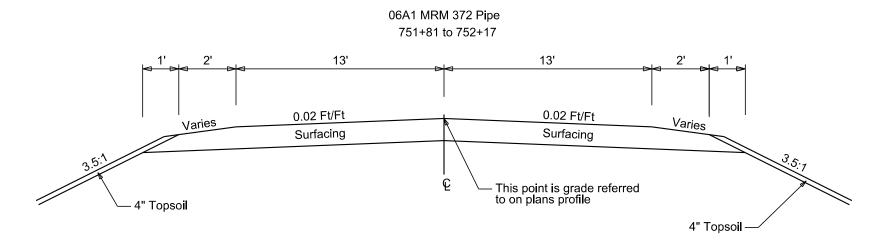


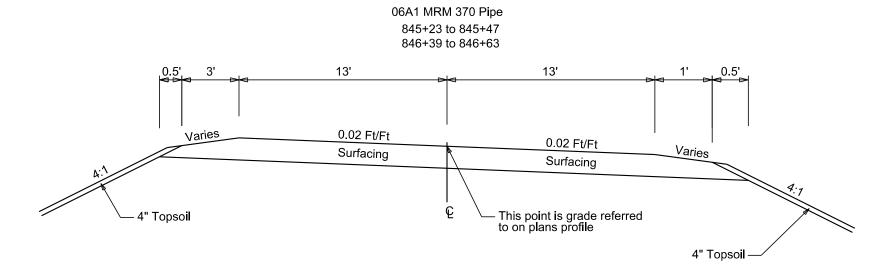


PROJECT NH 0012(230)171...+

TOTAL SHEETS SHEET B22 B85

Plotting Date:







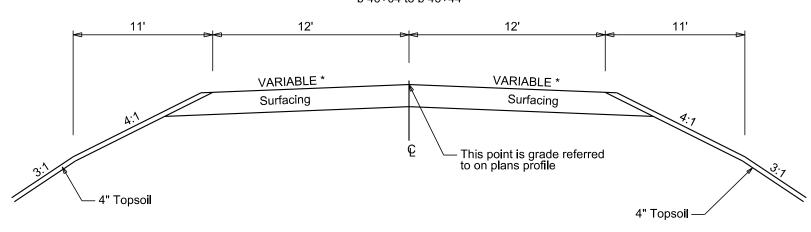
PROJECT NH 0012(230)171...+

TOTAL SHEETS SHEET B23 B85

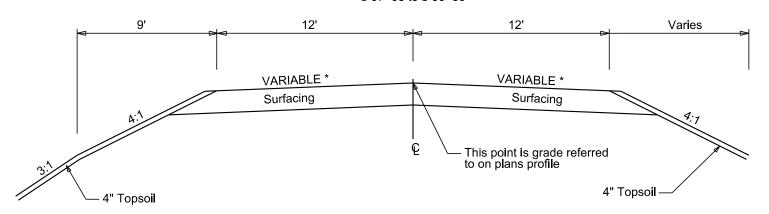
Plotting Date: 6/20/2024

\* Match Existing

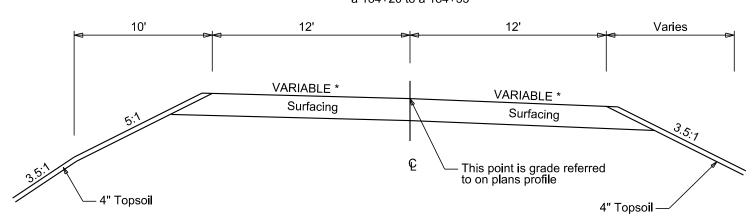
06RC MRM 362 Pipe (N) b 46+04 to b 46+44



#### 06RC MRM 362 Pipe (S) b 37+95 to b 38+35



#### 06RC MRM 360 Pipe a 184+20 to a 184+35





# HORIZONTAL ALIGNMENT DATAIDDING PURPOSES ONLY

STATE OF	PROJECT	SHEET	TOTAL
SOUTH DAKOTA	NH 0012(230)171+	B24	B85

## 05TY\_Mainline

<u>Type</u>	STATION			<u>Northing</u>	<u>Easting</u>
POB	a 0+00.00	TI - 0074 07	C 5°20 20   5	664275.869	1805251.712
DI	- 00+74-07	TL= 2671.97	S 5°30'39" E	664646 054	1005500 247
PI	a 26+71.97	TI - 2040 40	S 5°31'41" E	661616.251	1805508.317
DI	o FF   20 16	TL= 2848.19	553141 E	650701 202	1005702 607
PI	a 55+20.16	TL= 2803.50	S 5°31'10" E	658781.303	1805782.687
РС	a 83+23.66	TL- 2003.50	333110 E	655990.799	1806052.333
PI	a 92+28.86	R = 7610.81	Delta = 13°33'55" L	655089.845	1806139.851
PT	a 101+25.59	10 - 7010.01	Della - 10 00 00 L	654234.553	1806436.249
	a 101.25.55	TL= 8194.09	S 19°04'07" E	004204.000	1000430.243
PC	a 183+19.68	12 0101.00	0 10 0107 2	646490.088	1809113.270
PI	a 191+18.32	R = 8681.71	Delta = 10°30'43" L	645735.562	1809375.027
PT	a 199+12.48		20.00 .0 2	645041.454	1809770.046
		TL= 6319.15	S 29°44'36" E		
PI	a 262+31.62			639554.819	1812905.079
		TL= 331.80	S 30°29'16" E		
PC	a 265+63.42			639268.899	1813073.417
PI	a 270+32.44	R = 2830.65	Delta = 18°48'58" L	638873.885	1813326.293
PT	a 274+93.02			638581.542	1813693.061
		TL= 293.77	S 53°15'32" E		
PI	a 277+86.79			638405.808	1813928.473
		TL= 4915.42	S 54°21'03" E		
PI	a 327+02.21			635540.998	1817922.746
		TL= 4798.87	S 54°20'10" E		
PC	a 375+01.09			632743.116	1821821.599
PI 	a 384+96.90	R = 7643.89	Delta = 14°50'42" L	632162.258	1822630.463
PT	a 394+81.57	<del></del>	0.00040440#	631808.023	1823561.147
DI	404.00.04	TL= 948.34	S 69°18'40" E	004.470.070	1001110 005
PI	a 404+29.91	TI - 405 40	0.0094012011 F	631472.978	1824448.335
		TL= 125.12	S 69°19'39" E <b>4+80.80 Bk. = b 20+40.00</b>	۸h	
PI	b 21+46.23	Equation. a 40	4+00.00 BK D 20+40.00	631428.808	1824565.396
ГІ	0 21+40.23	TL= 4867.23	S 69°14'47" E	031420.000	1024303.390
PI	b 70+13.46	1L= 4007.23	3 09 1447 L	629704.114	1829116.806
	D 70 · 10.40	TL= 883.34	S 69°17'14" E	023704.114	1023110.000
PI	b 78+99.80	12 000.04	0 00 11 14 2	631158.471	1825280.338
	2 . 0 . 00.00	TL=4102.88	S 69°14'20" E	331130.111	.020200.000
PC	b 120+02.68	12 1132.00	2 00 20 2	629704.114	1829116.806
PI	b 127+05.80	R= 11165.91	Delta = 7°12'23" L	629453.433	1829773.717
			-		-

PT	b 134+07.06			629287.136	1830456.885
		TL= 3183.70	S 76°21'41" E		
POE	165+90.75			628536.432	1833550.808
		US12 &	SD 1806 Intersection		
POB	b 230+00.00			634507.313	1842825.558
		TL= 327.13	N 59°56'05" E		
PI	b 233+27.13			634671.201	1843108.674
		TL= 134.85	N 61°48'42" E		
PI	b 234+61.98			634734.901	1843227.532
		TL= 142.67	N 62°02'44" E		
PI	b 236+04.65			634801.781	1843353.557
		TL=150.58	N 65°12'02" E		
PI	b 237+55.23			634864.941	1843490.251
		TL= 147.68	N 69°59'13" E		
PC	b 239+02.91			634915.482	1843629.012
PI	b 243+12.86	R= 1157.33	Delta= 39°00'37" R	635031.663	1844022.153
PT	b 246+90.89			634874.474	1844400.769
		TL= 152.55	S 64°31'26" E		
PI	b 248+43.44			634808.855	1844538.489
		TL= 196.16	S 59°01'41" E		
POE	b 250+39.58			634707.921	1844706.657

### PCN 06RC - P 1806(22)359

### 06RC\_MRM 360

<u>Type</u>	<u>Station</u>			<u>Northing</u>	<u>Easting</u>
POB	a 178+54.10			617592.107	1839955.693
		TL= 167.13	S 32°08'24" W		
PC	a 180+21.23			617450.588	1839866.780
PI	a 188+18.04	R = 820.00	Delta = 88°21'22" L	616775.892	1839442.889
PT	a 192+85.76			616332.821	1840105.147
		TL= 168.34	S 56°12'58" E		
POE	a 194+54.10			616239.213	1840245.062



# HORIZONTAL ALIGNMENT DE AIDDING PURPOSES ONLY STATE OF SOUTH DAKOTA

**Easting** 

1841138.048

1841295.431

1841362.765

1841203.768

1840802.780

1840680.604

1840700.683

1840748.417

1840771.211

1840621.296

1840205.257

Northing

624693.996

625488.564

625828.511

626136.437

626913.016

627149.630

627415.167

628046.405

628347.830

628610.322

629338.778

PC	974+28.29			650369.986	1838935.187
PI	979+65.91	R = 1913.74	Delta = 31°23'00" R	650152.570	1839426.891
PT	984+76.52			649710.901	1839733.439
		TL= 596.74	S 34°45'48" E		
PC	990+73.26			649220.670	1840073.692
PI	993+20.46	R = 1148.09	Delta = 24°18'07" R	649017.594	1840214.641
PT	995+60.22			648774.506	1840259.525
EBK	995+62.08			648772.679	1840259.862
EAH	a 996+13.12			648772.679	1840259.862
		TL= 517.86	S 10°27'41" E		
PC	a 1001+29.12			648265.252	1840353.554
PI	a 1003+10.75	R = 1001.95	Delta = 20°32'56" L	648086.648	1840386.532
PT	a 1004+88.47			647930.983	1840480.103
		TL= 1495.30	S 31°00'37" E		
PC	a 1019+83.77			646649.396	1841250.468
PI	a 1023+46.70	R = 1910.05	Delta = 21°31'01" L	646338.338	1841437.446
EBK	a 1026+99.76			646118.340	1841724.436
EAH	b 1027+58.02			646118.340	1841724.436
PT	b 1027+59.34			646117.538	1841725.482
		TL= 1304.54	S 52°31'38" E		
PC	b 1040+63.88			645323.875	1842760.823
PI	b 1050+51.46	R = 952.26	Delta = 92°05'10" R	644723.048	1843544.608
PT	b 1055+94.36			643961.654	1842915.648
		TL= 1696.11	S 39°33'32" W		
PC	b 1072+90.47			642654.002	1841835.444
PI	b 1082+47.91	R = 1447.22	Delta = 66°58'30" L	641915.845	1841225.680
PT	b 1089+82.17			641065.940	1841666.531
EBK	b 1098+83.01			640266.279	1842081.319
EAH	c 58+48.00			640266.279	1842081.319
		TL= 901.12	S 27°24'58" E		
POE	c 58+48.29			640266.026	1842081.451

06A1_Mainline							
<u>Type</u>	<u>Station</u>			<u>Northing</u>	<u>Easting</u>		
POB	740+00.00			667593.128	1839548.802		
		TL= 1504.55	S 0°20'58" W				
PC	755+04.55			666088.605	1839539.624		
PI	765+79.31	R = 3803.80	Delta = 31°33'19" R	665013.862	1839533.067		
PT	775+99.47			664101.466	1838965.045		
		TL= 6170.08	S 31°54'17" W				
PC	837+69.55			658863.512	1835704.097		
PI	844+78.76	R = 1270.18	Delta = 58°21'13" R	658261.449	1835329.276		
PT	850+63.18			658264.647	1834620.079		
		TL= 309.72	N 89°44'30" W				
PI	853+72.90			658266.044	1834310.361		
		TL= 511.48	N 89°25'31" W				
PC	858+84.38			658271.173	1833798.905		
PI	877+50.95	R = 1146.53	Delta = 116°52'47" L	658289.892	1831932.433		
PT	882+23.23			656616.615	1832759.604		
		TL= 3363.48	S 26°18'18" E				
PC	915+86.71			653601.433	1834250.136		
PI	932+42.49	R = 4568.87	Delta = 39°50'29" L	652117.118	1834983.895		
PT	947+63.74			651447.524	1836498.239		
		TL= 2664.55	S 66°08'48" E				

06RC\_MRM 362

N 11°12'13" E

N 27°18'35" W

N 4°19'28" E

Delta = 38°30'48" L

Delta = 31°38'03" R

Delta = 34°03'22" L

PCN 06A1 - P 1806(19)364

N 29°43'54" W

**Station** 

TL= 810.00

R = 992.00

TL= 873.99

R = 940.00

TL= 633.04

R = 987.00

TL= 838.89

b 5+71.60

b 13+81.60

b 17+28.16

b 20+48.41

b 29+22.41

b 31+88.70

b 34+41.40

b 40+74.44

b 43+76.72

b 46+61.10

b 55+00.00

Type POB

PC

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PΤ

PC

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PΤ

PC

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PΤ

POE



PROJECT

NH 0012(230)171...+

TOTAL SHEETS

B85

SHEET

B25

The coordinates shown on this sheet are based on the South Dakota State Plane Coordinate System. North Zone NAD 83; Geoid 18 (Conus); SF: 0.998992938

# HORIZONTAL ALIGNMENT DATE ADDING PURPOSES ONLY STATE OF SOUTH DAKOTA

PROJECT NH 0012(230)171...+

SHEET B26

TOTAL SHEETS B85

### PCN 06E0 - P 1806(20)370

### PCN 07CD - P 0063(59)251

## 06E0\_Mainline

## 07CD\_Mainline

<u>Type</u>	<b>Station</b>			<u>Northing</u>	<u>Easting</u>	<u>Type</u>	<u>Station</u>			<u>Northing</u>	<u>Easting</u>
POB	0+00.00			665998.021	1831263.516	POB	272+30.70			723758.823	1761787.625
		TL= 724.67	S 7°45'13" E					TL= 1018.46	N 0°50'36" E		
PC	7+24.67			665279.980	1831361.281	PC	282+46.16			724777.173	1761802.613
PI	9+36.81	R = 1126.00	Delta = 21°20'21" R	665069.779	1831389.902	PI	287+94.68	R = 27133.53	Delta = 2°18'13" R	725322.638	1761810.641
PT	11+44.03			664863.575	1831340.070	PT	293+40.06			725867.340	1761840.587
		TL= 2397.16	S 13°35'08" W					TL= 363.43	N 3°08'48" E		
PC	35+41.19			662533.488	1830776.984	PC	297+03.49			726230.223	1761860.537
PI	40+72.20	R = 963.00	Delta = 57°44'46" L	662017.332	1830652.250	PI	304+37.33	R = 22553.68	Delta = 3°43'38" L	726962.950	1761900.820
PT	45+11.76			661636.388	1831022.191	PT	311+70.64			727696.744	1761893.387
		TL= 1969.78	S 44°09'38" E					TL= 799.61	N 0°34'49" W		
PC	64+81.54			660223.285	1832394.477	PI	319+70.25			728496.311	1761885.287
PI	70+50.75	R = 1906.50	Delta = 33°14'51" R	659814.936	1832791.031			TL= 571.63	N 0°27'27" W		
PT	75+87.84			659256.018	1832898.793	PC	325+41.88			729067.921	1761880.722
		TL= 1204.34	S 10°54'47" E			PI	333+24.80	R = 101833.36	Delta = 0°52'52" R	729850.820	1761874.470
POE	87+92.17			658073.459	1833126.795	PT	341+07.69			730633.723	1761880.257
								TL= 1245.08	N 0°25'24" E		
						PI	353+52.77			731878.769	1761889.459
								TL= 1466.40	N 0°22'21" E		
						PI	368+19.18			733345.140	1761898.992
								TL= 2230.82	N 0°22'41" E		
						POE	390+50.00			735575.914	1761913.714



# CONTROL DATA

FOR BIDDING PURPOSES ONLY

	STATE OF	PROJECT	SHEET	TOTAL SHEET:
L'	SOUTH DAKOTA	NH 0012(230)171+	B27	B85

POINT	DESCRIPTION	NORTHING	EASTING	ELEVATION
CP 1		621600.4800	1869421.9500	1776.09
CP 2		628391.1350	1851848.7270	1676.51
CP 3		621600.4800	1869421.9500	1776.09
CP 4		633617.4960	1846036.2430	1631.07
CP 5		633712.1560	1846061.8680	1631.91
CP 6		633986.0470	1846450.1420	1598.68
CP 7		636058.5870	1844166.4560	1627.80
CP 8		636289.8490	1844231.3210	1629.22
CP 9		636493.1000	1843982.1290	1626.91
CP 10		636615.5560	1843918.2360	1625.17
CP 11		636693.4200	1843890.6280	1624.80
CP 12	Corps Brass Cap	640311.7270	1841983.8700	1636.70
CP 14	·	640422.7490	1841978.8810	1632.97
CP 15		640671.5200	1841800.2550	1635.06
CP 16		641421.0450	1841476.4080	1674.55
CP 17		648670.6200	1840249.2550	1706.78
CP 18		648728.1550	1840238.9120	1712.68
CP 19	T bar cap 6a	658743.7310	1835535.1050	1690.24
CP 20	•	633549.4370	1841400.5630	1627.91
CP 21		632637.0460	1840645.8230	1659.97
CP 22		631749.7620	1839935.8190	1697.82
CP 23		631077.8890	1839619.3210	1720.74
CP 24		630558.0180	1838019.0020	1738.57
CP 25		629650.2870	1836563.2630	1634.73
CP 26		629350.4530	1836002.4370	1615.26
CP 27		628473.8210	1833394.2750	1718.96
CP 28		631529.4670	1824579.3980	1763.04
CP 29		651100.1450	1807415.9250	1671.89
CP 30		655222.0620	1806067.7890	1792.70
CP 31		661680.8900	1805343.1050	1766.69
CP 32		666435.5800	1805090.5220	1640.36
CP 33		730823.0470	1761809.5590	2002.48
CP 34		731404.5460	1744420.5030	2028.91
CP 35		738021.4380	1761766.2200	2030.80
CP 36		740621.9630	1761781.7450	2039.52
CP 37		741463.3200	1762038.4600	2040.31
CP 38		742673.5510	1762046.6300	2032.68
CP 39		743705.2040	1762091.8900	2036.08
CP 40		744667.9820	1761813.0500	2036.61
CP 41		745956.5010	1761817.5400	2055.97

The coordinates shown on this sheet are based on the South Dakota State Plane Coordinate System. North Zone NAD 83; Geoid 18 (Conus); SF = 0.998992938

The elevations shown on this sheet are based on NAVD 88.



**LEGEND** 

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PS

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FOR BIDDING PURPOSES ONLY

STATE OF SOUTH DAKOTA

PROJECT NH 0012(230)171...+

4/16/2024

SHEET TOTAL SHEETS

B28 B85

Plotting Date:

Anchor		_
Antenna		太
Approach		
Assumed Corner		<b>?</b>
Azimuth Marker		<b>A</b>
BBQ Grill/ Fireplace		▲
Bearing Tree		<b>®</b>
Bench Mark		<u>A</u>
Box Culvert		
Bridge		
Brush/Hedge		
Buildings		
Bulk Tank		
Cattle Guard		
Cemetery		t
Centerline		
Cistern		©
Clothes Line		
Concrete Symbol		紐
Control Point		₾
Creek Edge		
Curb/Gutter		
Curb		
Dam Grade/Dike/Levee		
Deck Edge		
Ditch Block		<b>737</b> 70
Doorway Threshold		
Drainage Profile		
Drop Inlet		
Edge Of Asphalt		
Edge Of Concrete		
Edge Of Gravel		
Edge Of Other		
Edge Of Shoulder	Б.	
Electric Transformer/Power Junction	Rox	<b>(P</b> )
Fence Barbwire		
Fence Chainlink Fence Electric		
	,	—7 <del>——</del> 7—
Fence Miscellaneous Fence Rock		
Fence Snow		
Fence Wood		
Fence Woven		
Fire Hydrant		<b>රූ</b>
Flag Pole		<u> </u>
Flower Bed		7777
Gas Valve Or Meter		<i>•</i>
Gas Pump Island		
Grain Bin		(GB)
Guardrail		<u> </u>
Gutter		
Guy Pole		9
Haystack		
Highway ROW Marker		
Interstate Close Gate		₹-}}
Iron Pin		<b>⊙</b>
Irrigation Ditch		
Lake Edge		

Lake Edge

Lawn Sprinkler

Mailbox
Manhole Electric
Manhole Gas
Manhole Miscellaneous
Manhole Sanitary Sewer
Manhole Storm Sewer
Manhole Telephone
Manhole Water
Merry-Go-Round
Microwave Radio Tower Miscellaneous Line
Miscellaneous Property Corner
Miscellaneous Property Comer
Overhang Or Encroachment
Overhead Utility Line
Parking Meter
Pedestrian Push Button Pole
Pipe With End Section
Pipe With Headwall
Pipe Without End Section
Playground Slide
Playground Swing
Power And Light Pole
Power And Telephone Pole
Power Meter
Power Pole
Power Pole And Transformer
Power Tower Structure
Propane Tank
Property Pipe
Property Pipe With Cap
Property Stone
Public Telephone
Railroad Crossing Signal
Railroad Milepost Marker
Railroad POW Marker
Railroad ROW Marker
Railroad Signs Railroad Switch
Railroad Track
Railroad Trestle
Rebar
Rebar With Cap
Reference Mark
Retaining Wall
Riprap
River Edge
Rock And Wire Baskets
Rockpiles
Satellite Dish
Septic Tank
Shrub Tree
Sidewalk
Sign Face
Sign Post
Slough Or March

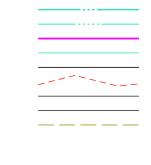
Slough Or Marsh

Stream Gauge

Street Marker

Spring

State and National Line
County Line
Section Line
Quarter Line
Sixteenth Line
Property Line
Construction Line
ROW Line
New ROW Line
Cut and Fill Limits
Control of Access
New Control of Access
Proposed ROW
(After Property Disposal)



Remove Concrete Pavement



Remove Concrete Driveway Pavement

Remove Asphalt Concrete Pavement



Remove Concrete Sidewalk



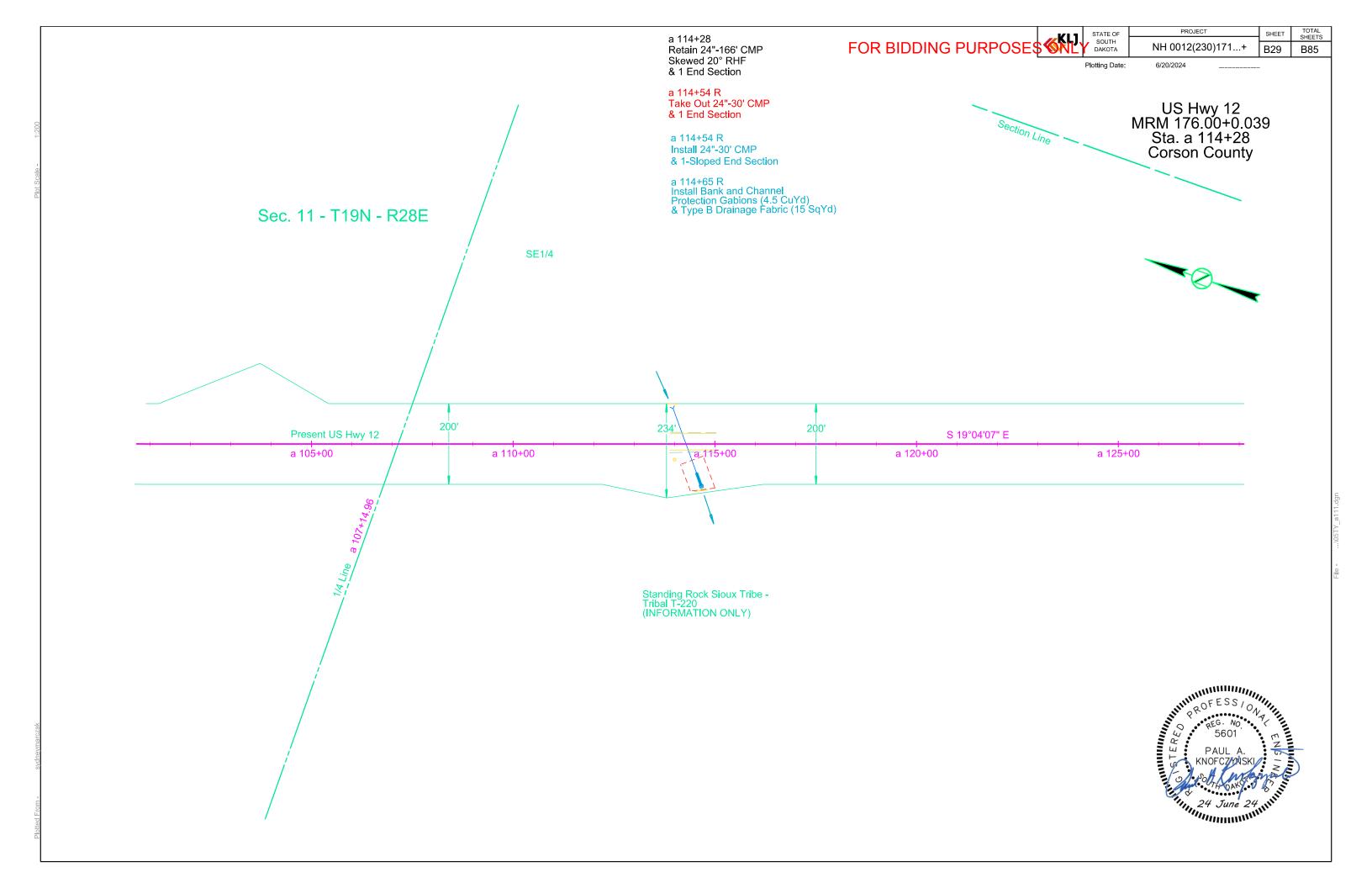
Remove Concrete Median Pavement

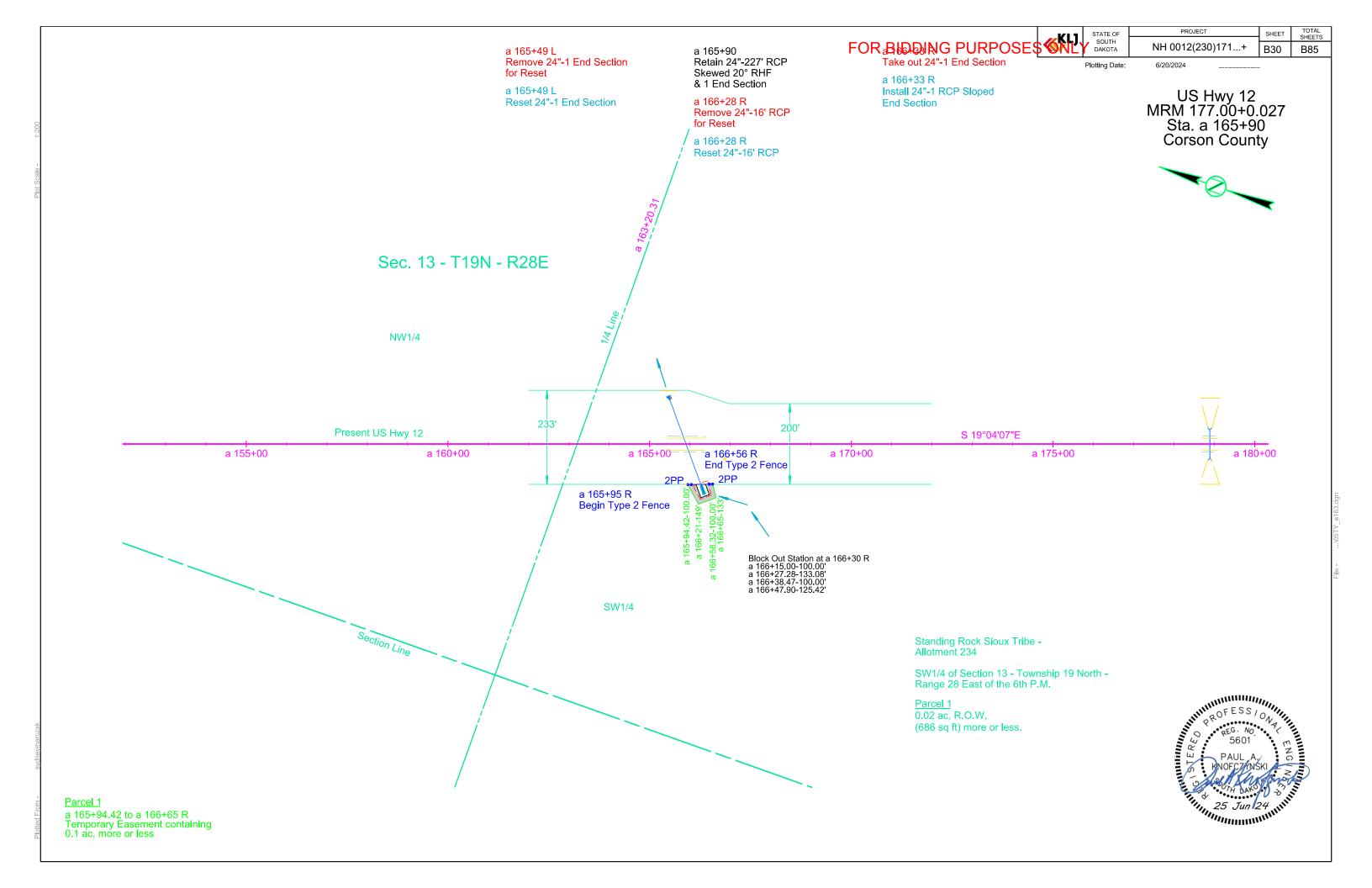
Remove Concrete Curb and/or Gutter



Detectable Warning Pedestrian Push Button Pole and 30" x 48" Clear Space with 1.5% slope

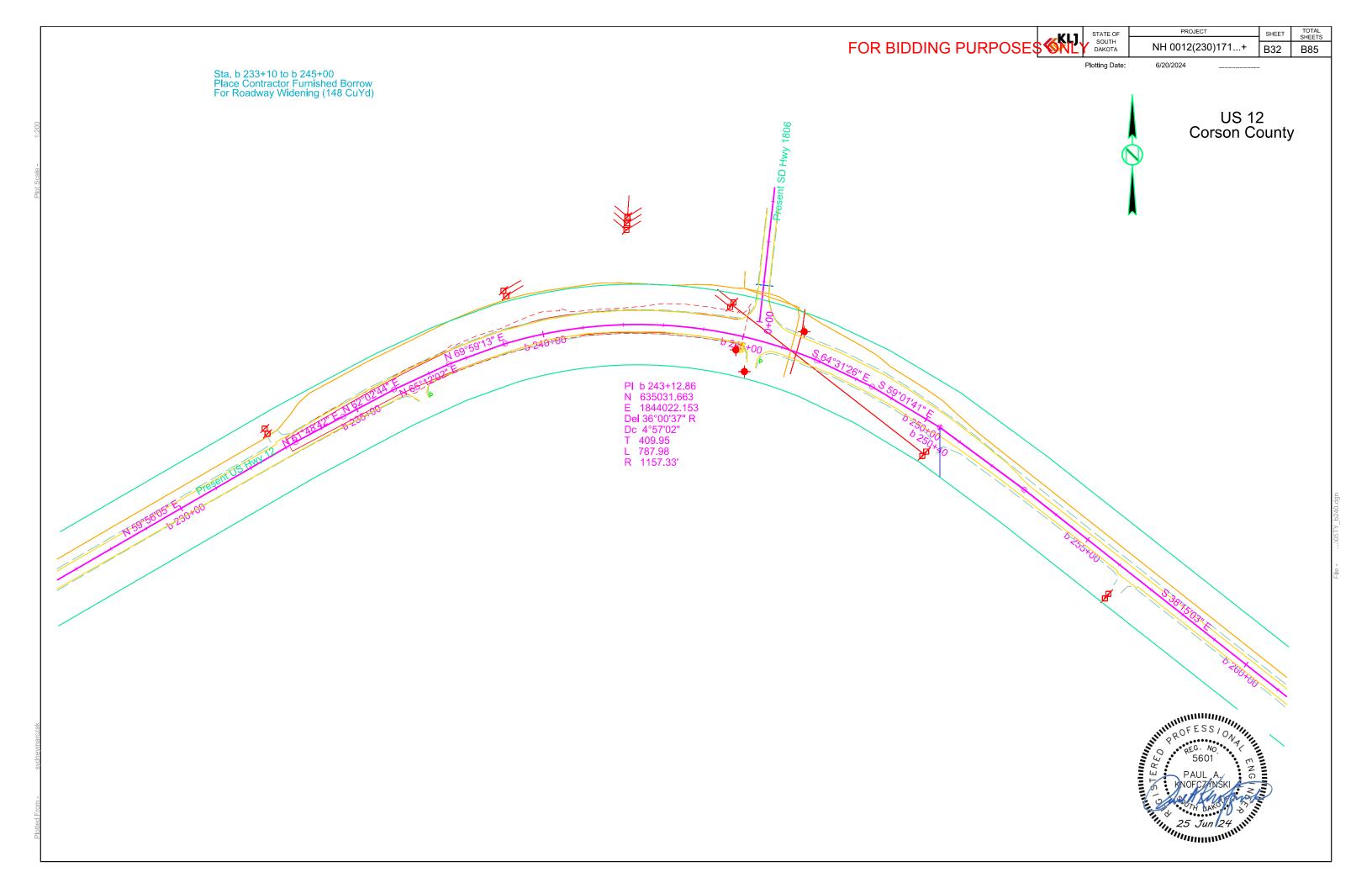


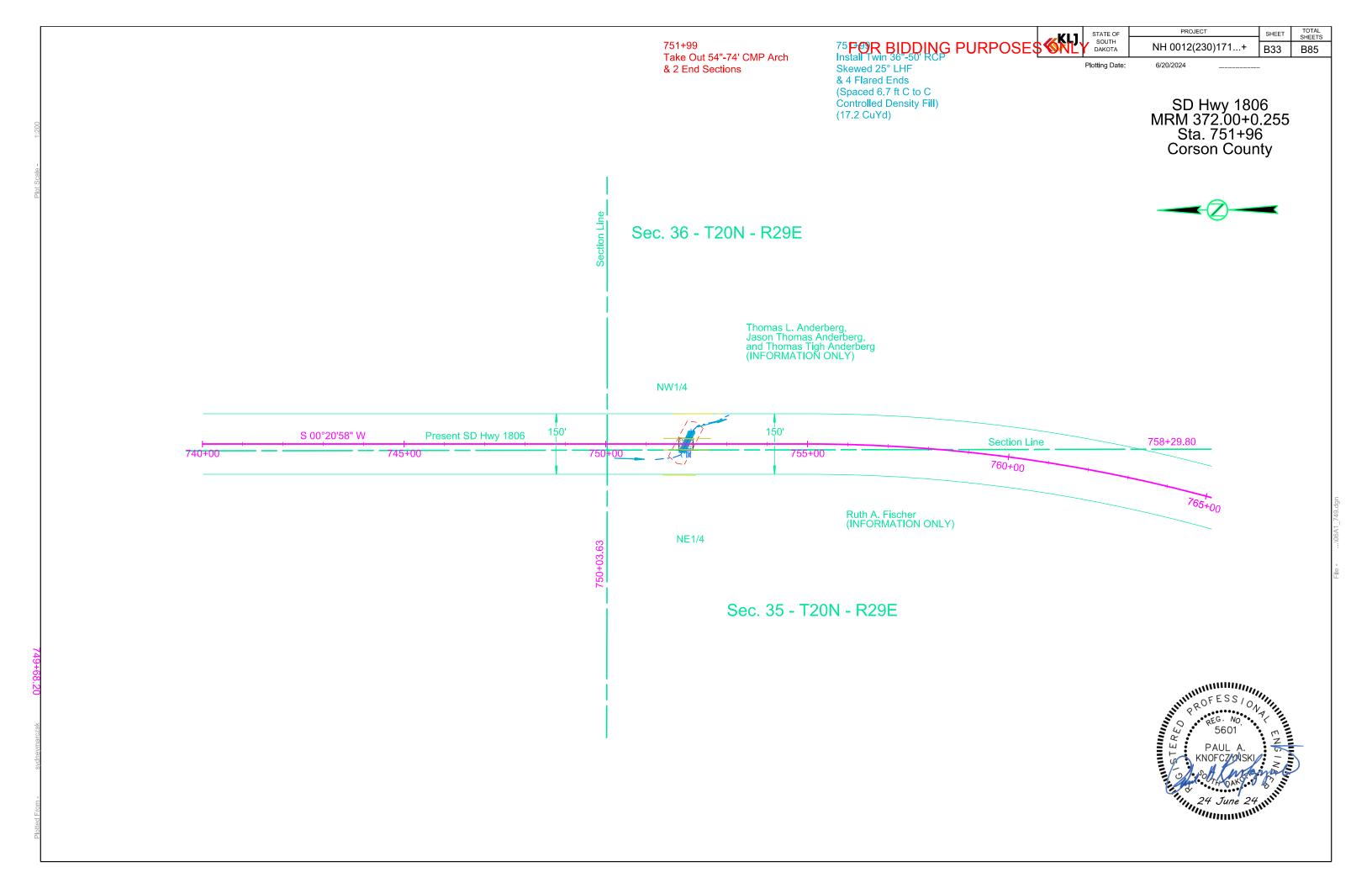


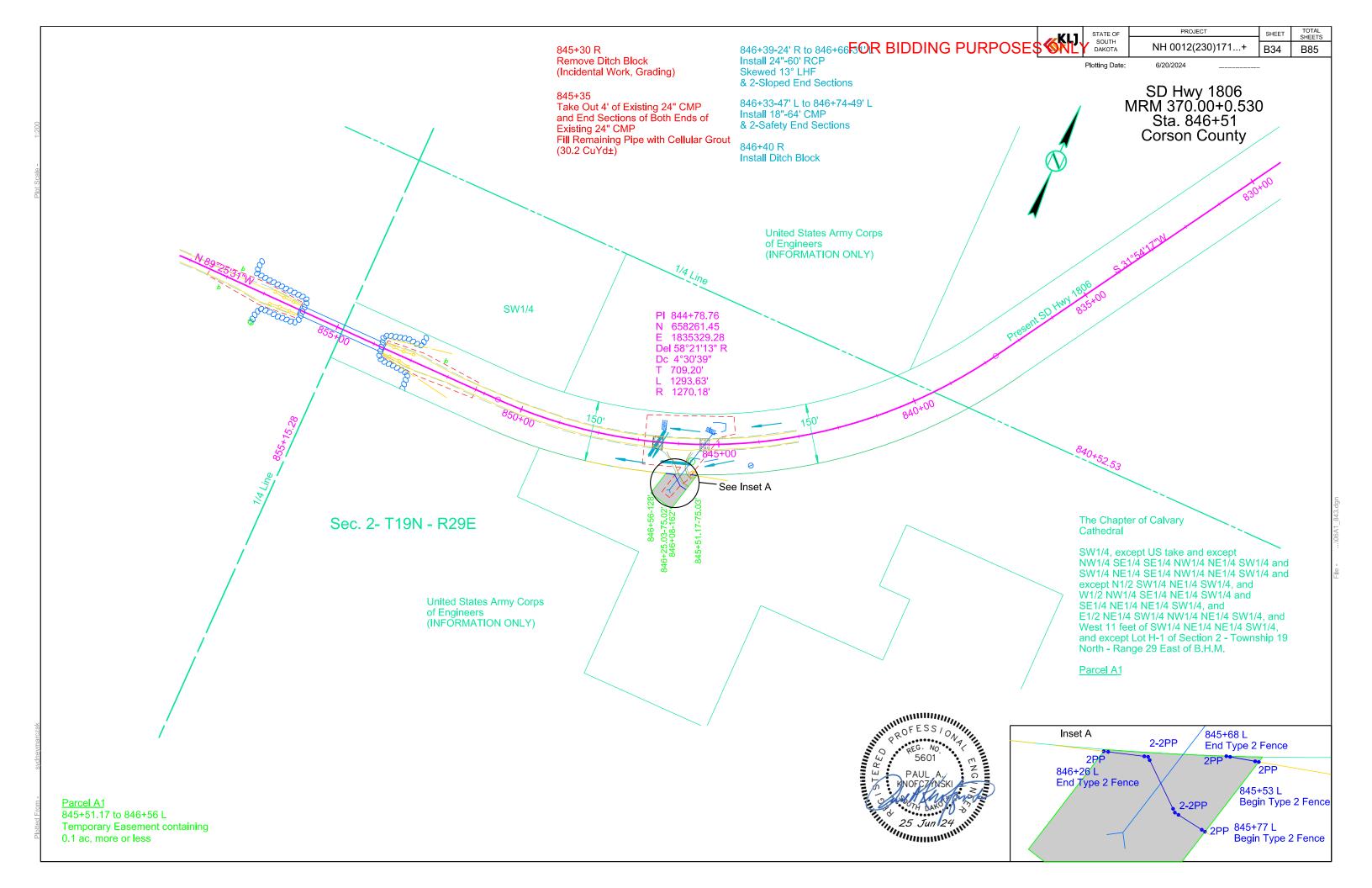


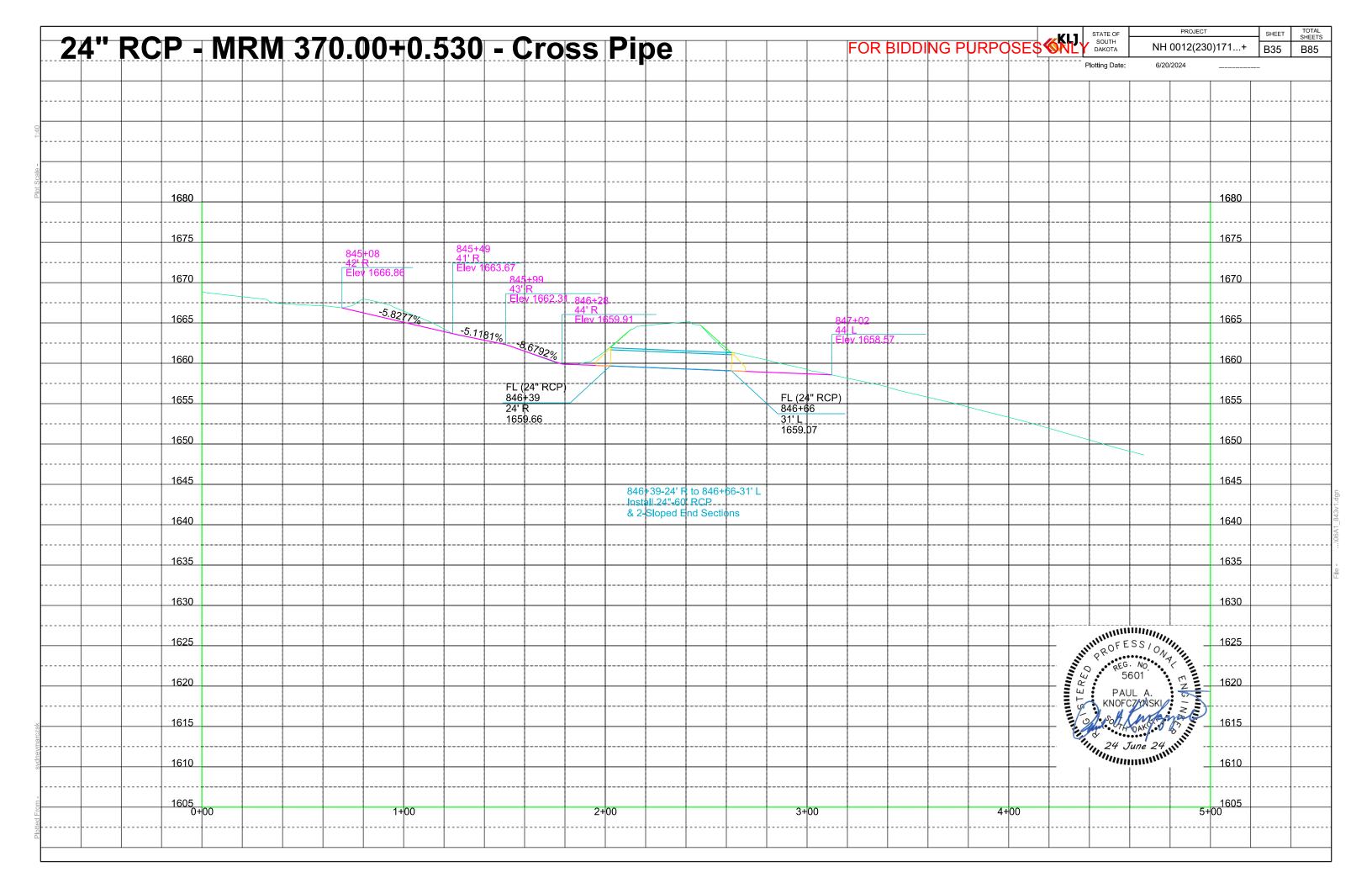
FOR BIDDING PURPOSES SOUTH DAKOTA TOTAL SHEETS PROJECT SHEET NH 0012(230)171...+ B31 B85 b 28+96 Retain 54"-165' RCP & 1 Flared End Sta 27+10 Remove 2 Cattle Pass End Sections b 28+40 L Install 54"-8' RCP & 1 Flared End Plotting Date: 6/20/2024 b 28+13 L Install Class B Riprap (Riprap Dimensions 20' X 12' X 3' - 37.3 Tons) and Type B Drainage Fabric (48 SqYd) Sta 27+10 Plug Existing Cattle Pass with Cellular Grout (62.2 CuYd) b 29+46 R Remove 54"-8' RCP US Hwy 12 Corson County for Reset b 27+10 Place Contractor Furnished Borrow Excavation Material to Shape Inslope (42 CuYd) b 28+33 L Place Contractor Furnished Borrow Excavation Material to Fill Hole Under Pipe Inlet (15 CuYd) b 29+46 R Reset 54"-8' RCP b 29+46 R Install 1-54" RCP Flared End Sec 33. - T19 - R29 SW1/4 S 69°18'40" E 200' Present US Hwy 12 EQNBKa 404+48.80 EQNAHDb 20+40.00 ent SD H PAUL A.

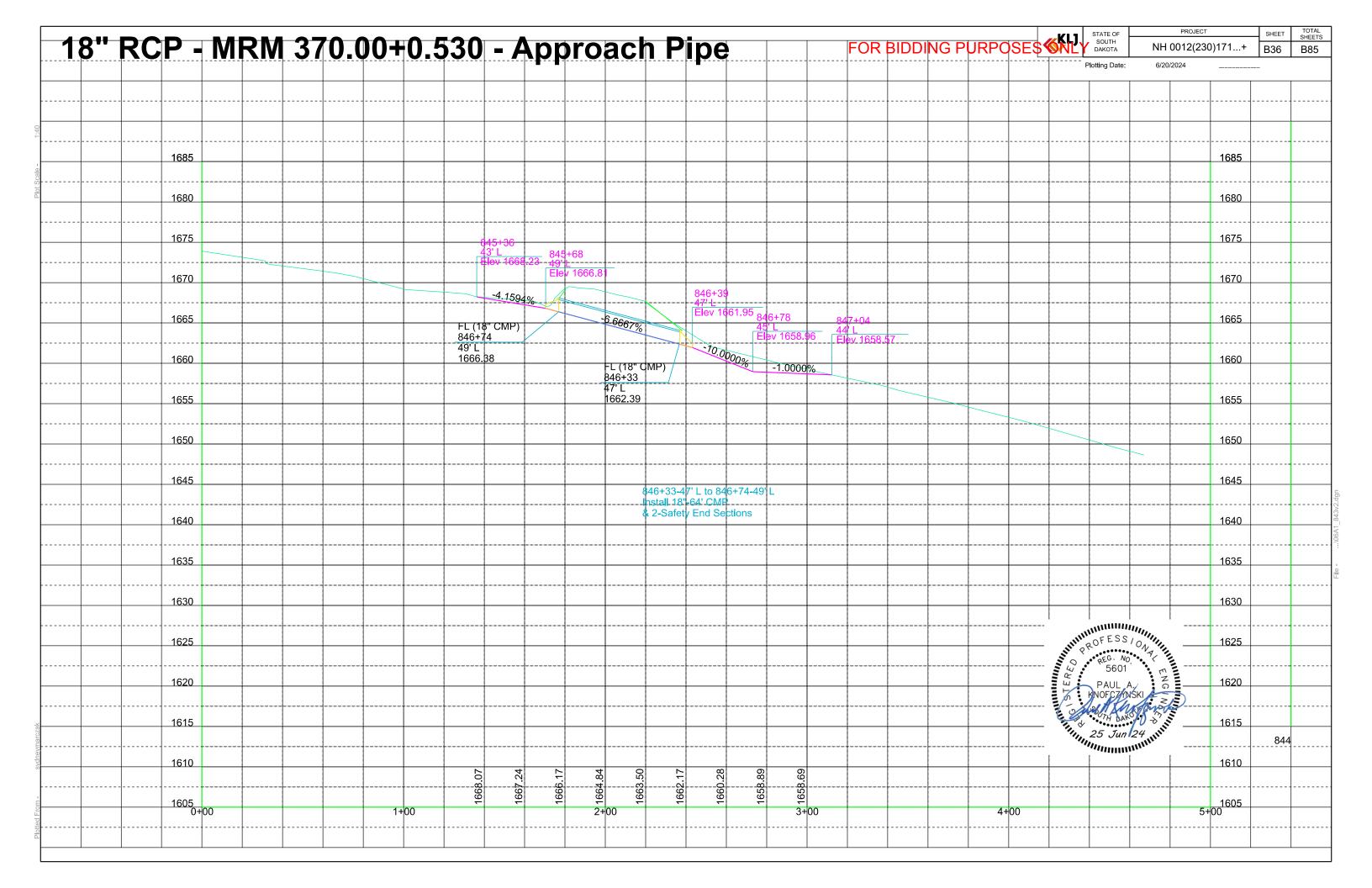
KNOFCZYNISKI 24 June 24 Junin

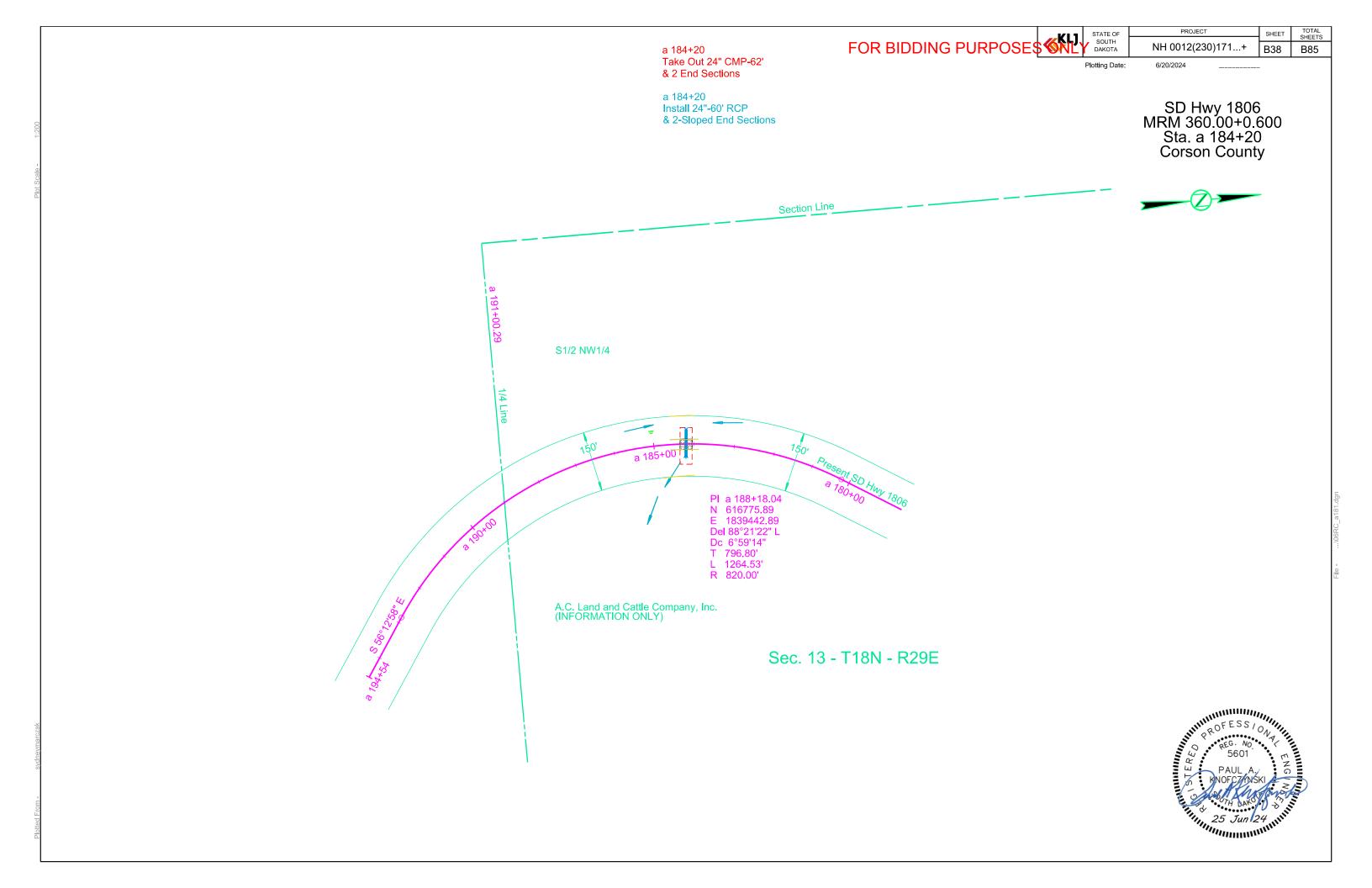


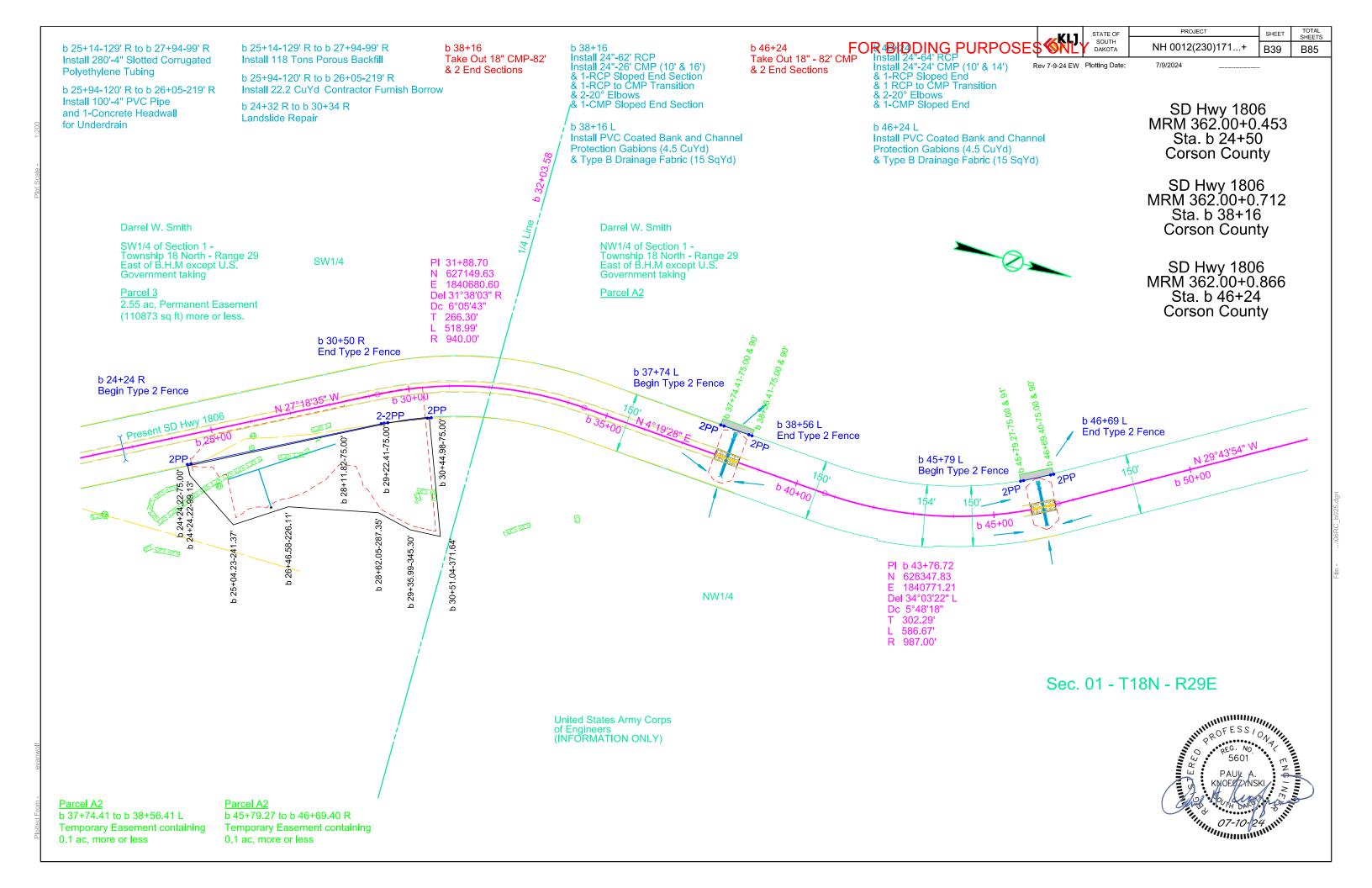


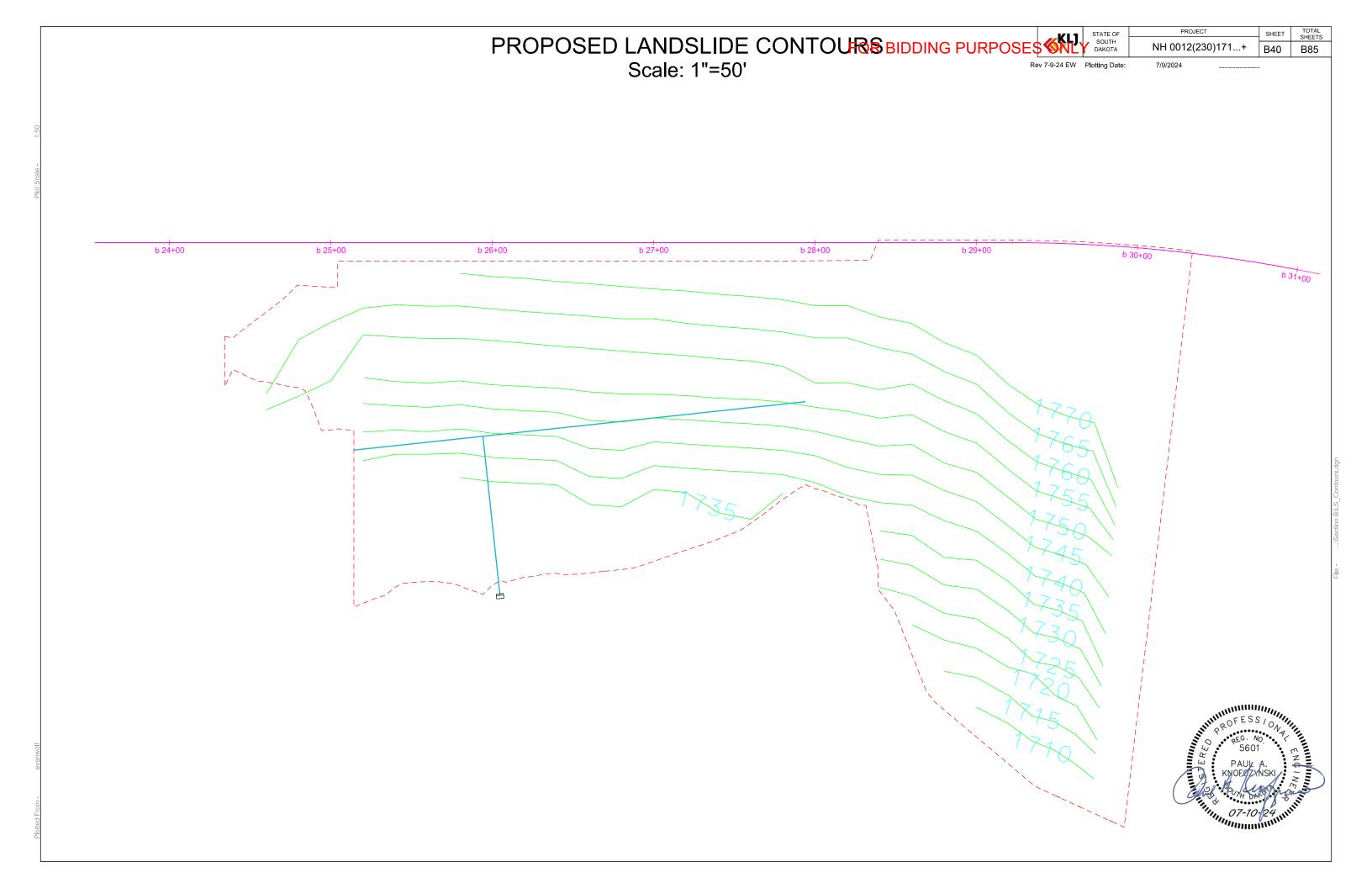


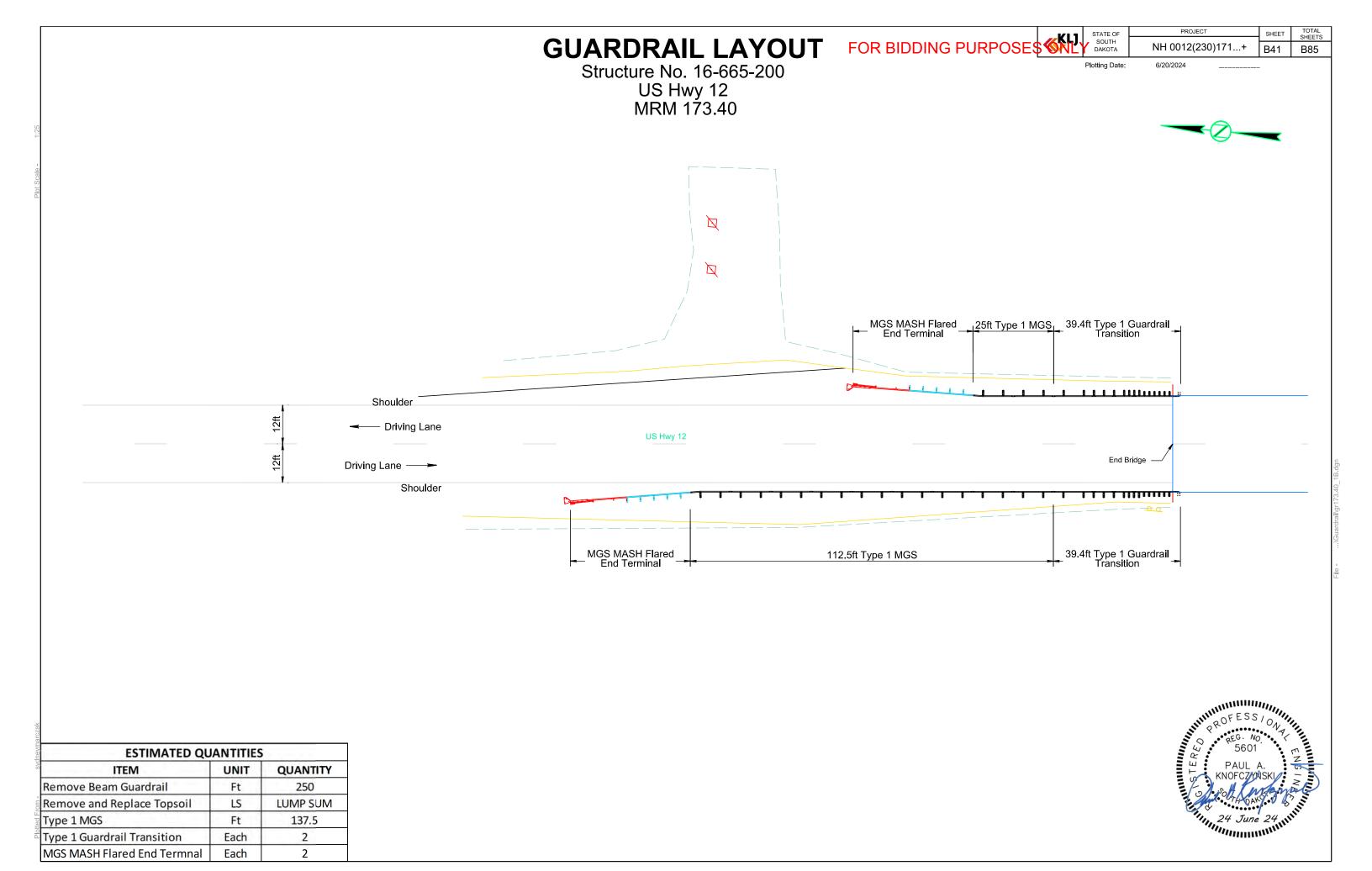


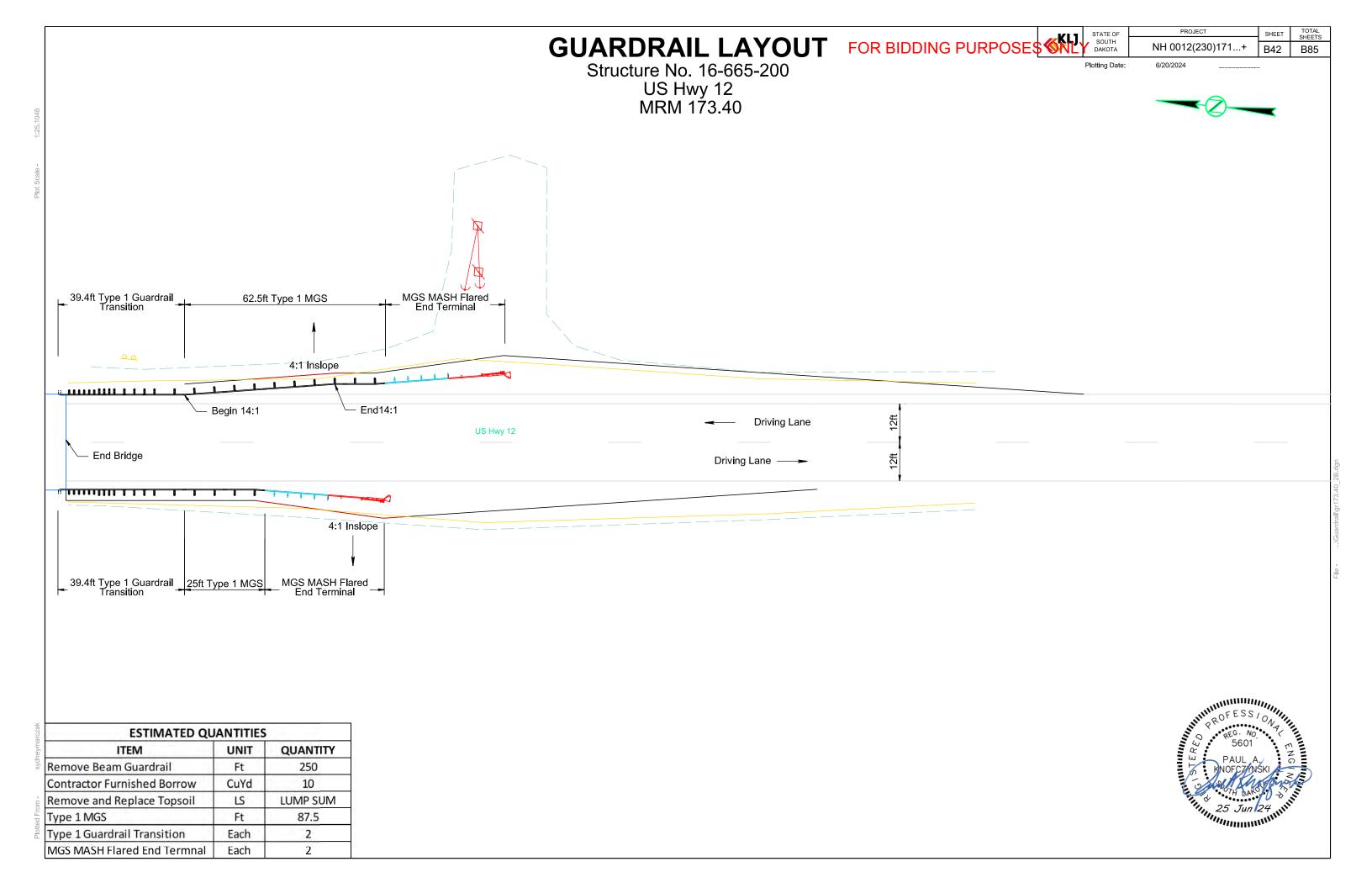












GUARDRAIL LAYOUT FOR BIDDING PURPOSES SOUTH BAKOTA

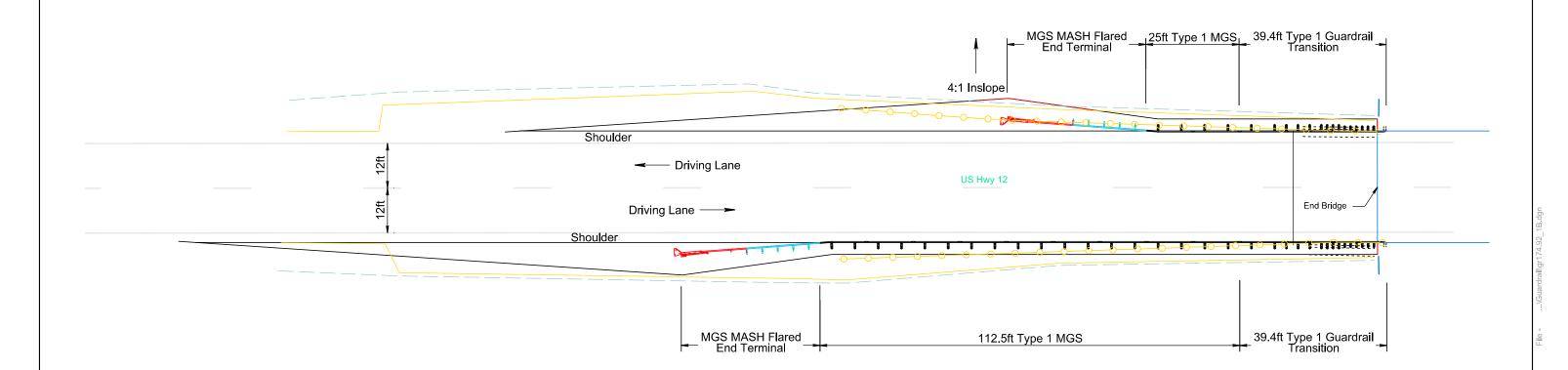
PROJECT NH 0012(230)171...+

TOTAL SHEETS SHEET B43 B85

Plotting Date:

6/20/2024

Structure No. 16-666-216 US Hwy 12 MRM 174.92



ESTIMATED QU	ANTITIES	
ITEM	UNIT	QUANTITY
Remove Beam Guardrail	Ft	300
Contractor Furnished Borrow	CuYd	5
Remove and Replace Topsoil	LS	LUMP SUM
Type 1 MGS	Ft	137.5
Type 1 Guardrail Transition	Each	2
MGS MASH Flared End Termnal	Each	2



Structure No. 16-666-216

US Hwy 12 MRM 174.92

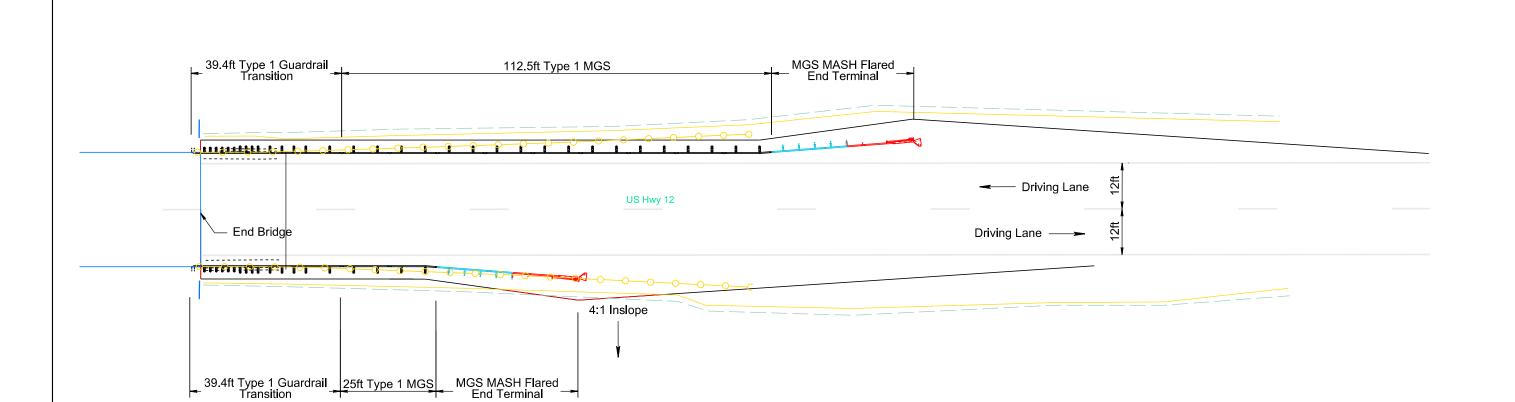
TOTAL SHEETS SHEET B85

NH 0012(230)171...+ B44

Plotting Date:

6/20/2024





ESTIMATED QU	ANTITIES	
ITEM	UNIT	QUANTITY
Remove Beam Guardrail	Ft	300
Contractor Furnished Borrow	CuYd	8
Remove and Replace Topsoil	LS	LUMP SUM
Type 1 MGS	Ft	137.5
Type 1 Guardrail Transition	Each	2
MGS MASH Flared End Termnal	Each	2



US Hwy 12 MRM 186.185

PROJECT NH 0012(230)171...+

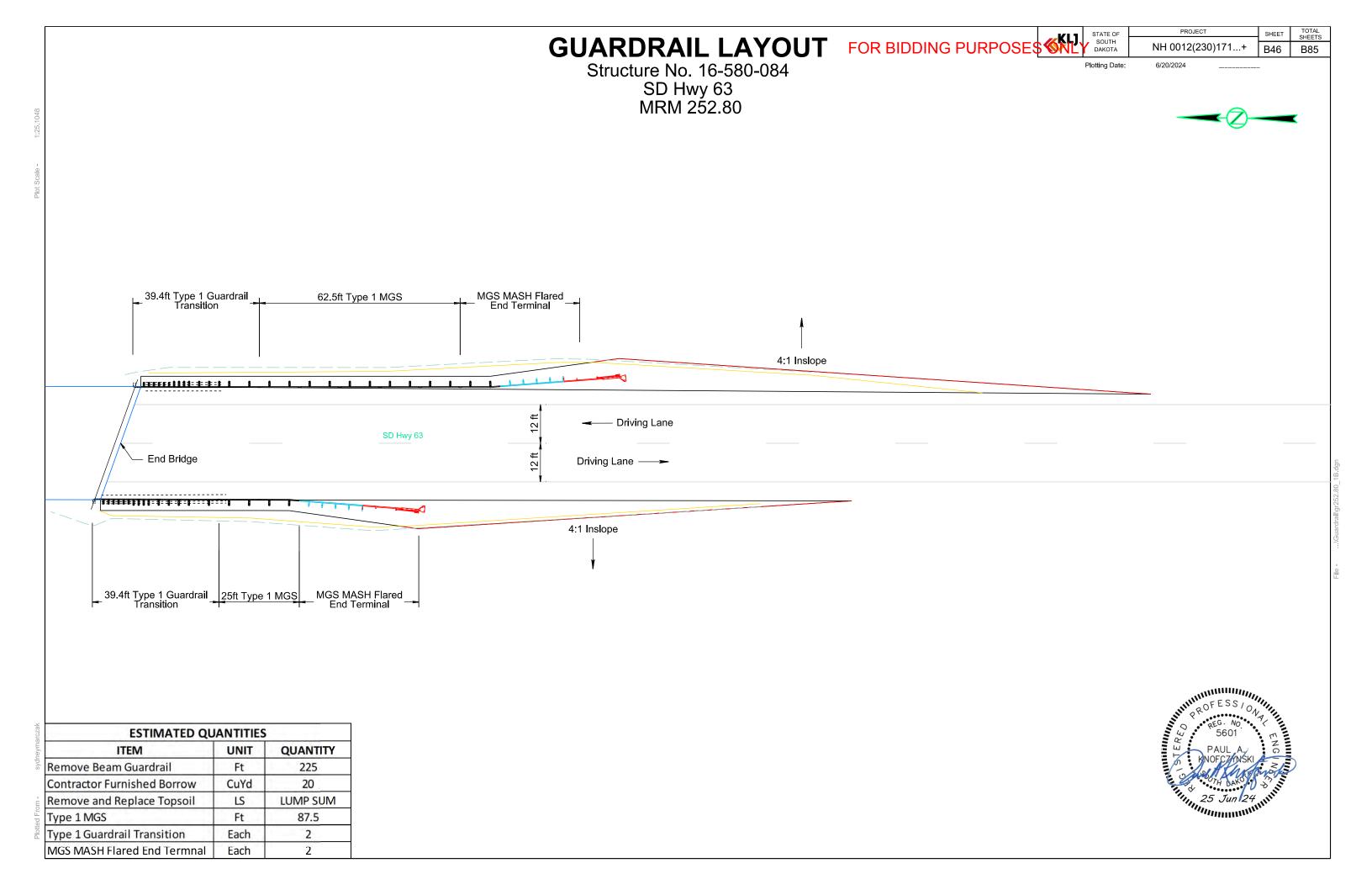
TOTAL SHEETS SHEET B45 B85

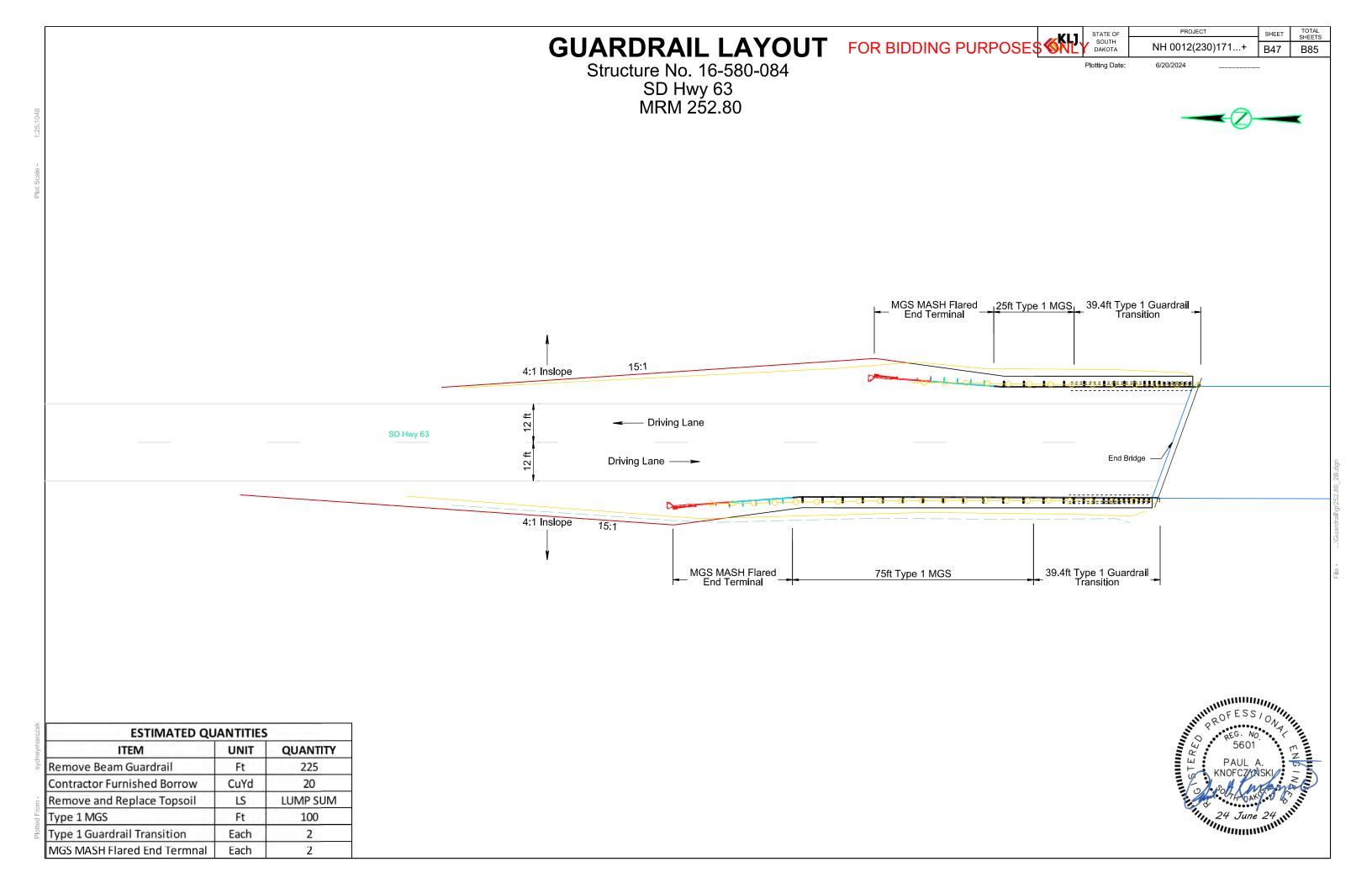
Plotting Date: 6/20/2024

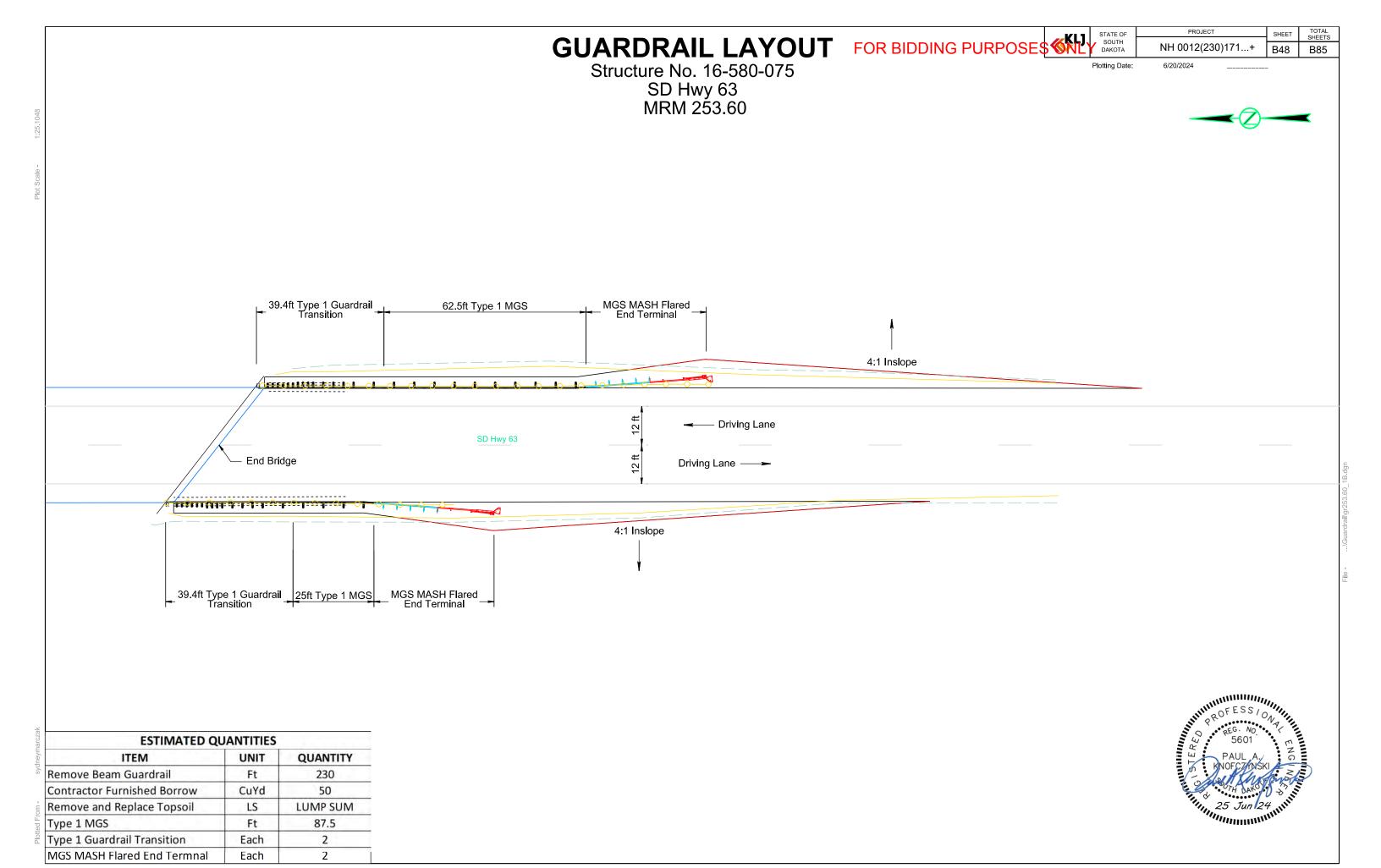
MGS MASH Flared 25ft Type 1 MGS 39.4ft Type 1 Guardrail Transition → Driving Lane US Hyw 12 Driving Lane ----End Bridge MGS MASH Flared End Terminal 39.4ft Type 1 Guardrail Transition 125ft Type 1 MGS

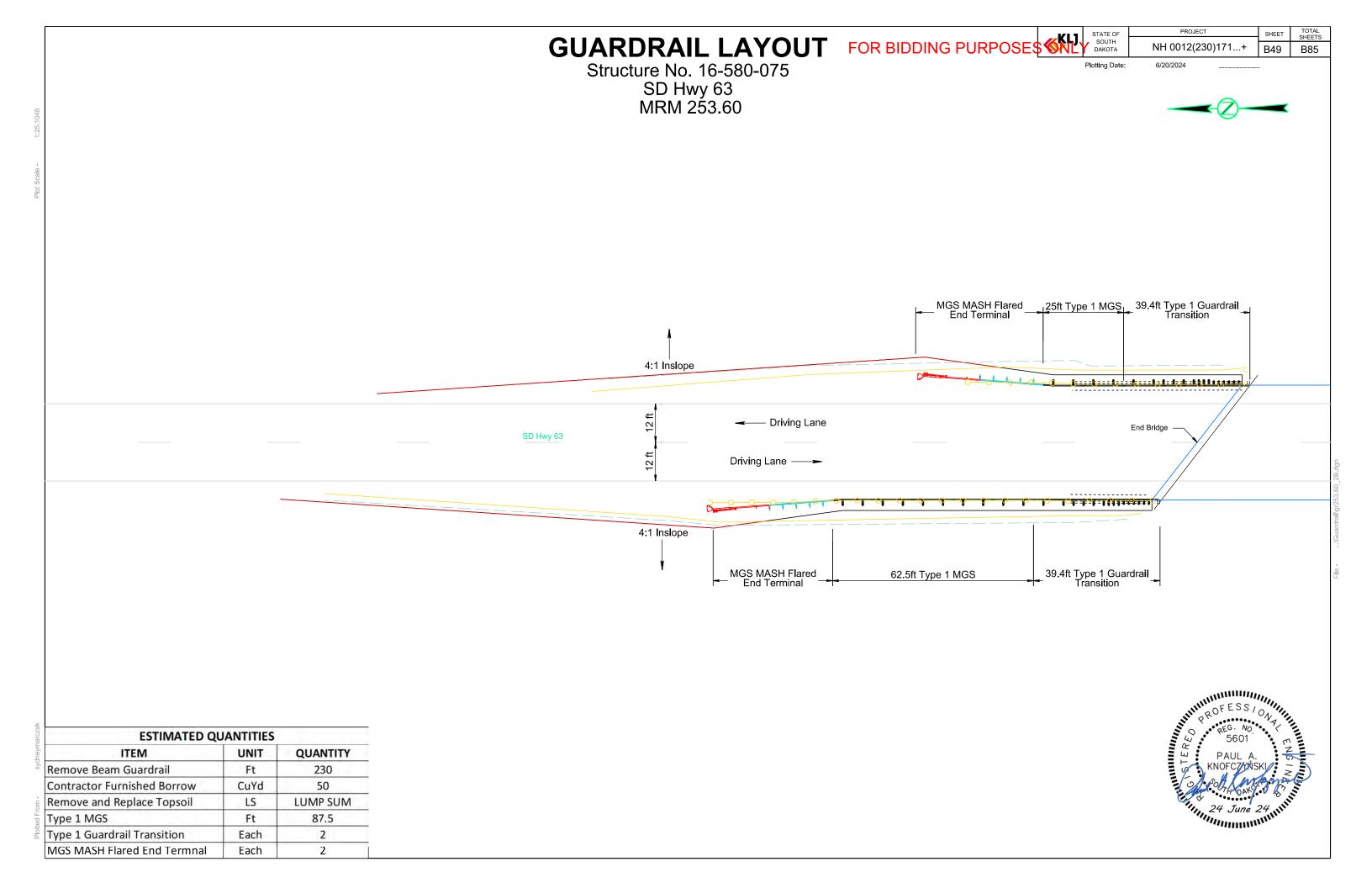
ESTIMATED QU	ANTITIES	
ITEM	UNIT	QUANTITY
Remove Beam Guardrail	Ft	300
Remove and Replace Topsoil	LS	LUMP SUM
Type 1 MGS	Ft	150
Type 1 Guardrail Transition	Each	2
MGS MASH Flared End Termnal	Each	2

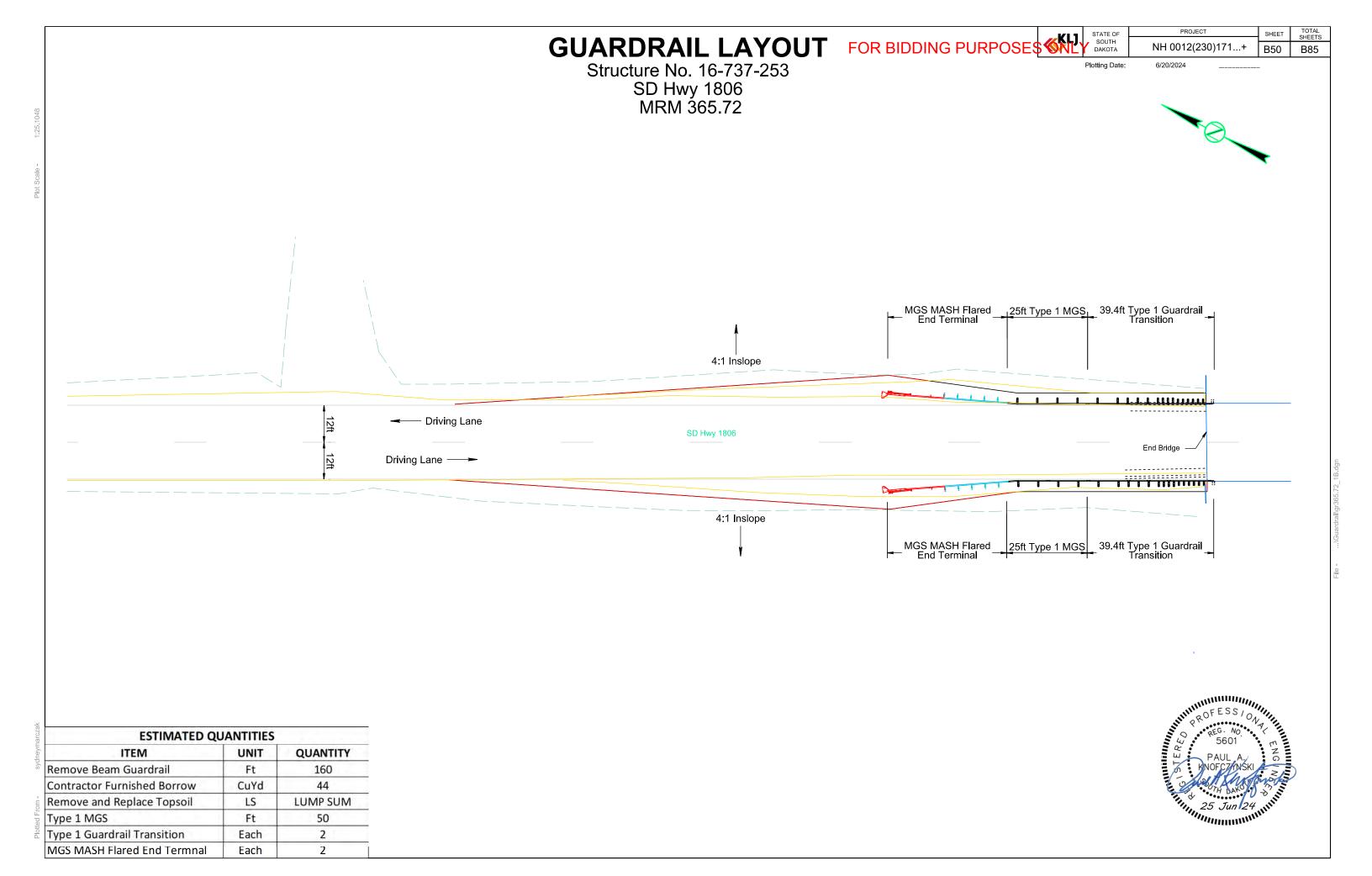












Structure No. 16-737-253

SD Hwy 1806 MRM 365.72

FOR BIDDING PURPOSES SOUTH DAKOTA

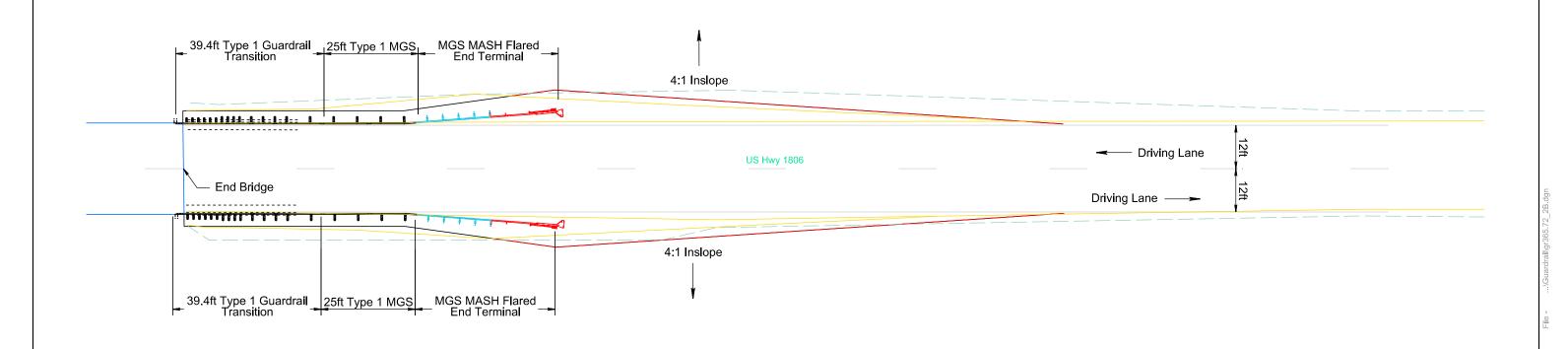
PROJECT NH 0012(230)171...+

TOTAL SHEETS SHEET B51 B85

Plotting Date:

6/20/2024





ESTIMATED QUANTITIES					
ITEM	UNIT	QUANTITY			
Remove Beam Guardrail	il Ft 160	160			
Contractor Furnished Borrow	CuYd	44			
Remove and Replace Topsoil	LS	LUMP SUM			
Type 1 MGS	Ft	50			
Type 1 Guardrail Transition	Each	2			
MGS MASH Flared End Termnal	Each	2			





PROJECT NH 0012(230)171...+

TOTAL SHEETS SHEET B52 B85

Plotting Date:



4:1	Inslope	MGS MASH Flared 25ft Type 1 MGS End Terminal	39.4ft Type 1 Guardrail Transition
		· <u> </u>	-1
SD Hwy 1806	→ Driving Lane		
12ft	Driving Lane —— <del>➤</del>		End Bridge
		1 1 1 1 22-1-1-1-1-1-1-1-1-1-1-1-1-1-1-1	17711
	4:1 Inslope  MGS MASH Flared End Terminal	25ft Type 1 MGS 39.4ft Type 1 Guar Transition	rdrail

ESTIMATED QU	ANTITIES	
ITEM	UNIT	QUANTITY
Remove Beam Guardrail	Ft	200
Contractor Furnished Borrow	CuYd	50
Remove and Replace Topsoil	LS	LUMP SUM
Type 1 MGS	Ft	50
Type 1 Guardrail Transition	Each	2
MGS MASH Flared End Termnal	Each	2



# **GUARDRAIL LAYOUT**

Structure No. 16-732-234

SD Hwy 1806 MRM 367.64

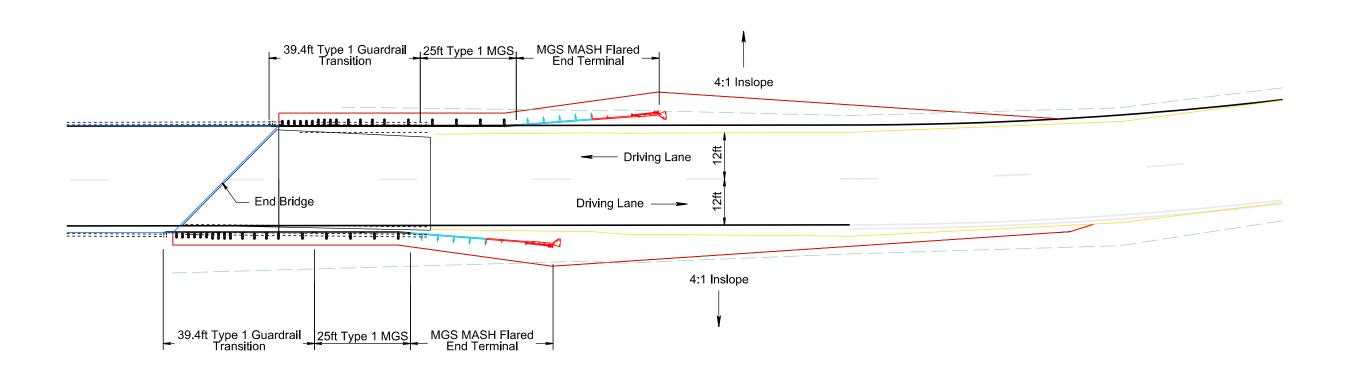
FOR BIDDING PURPOSES SOUTH DAKOTA

PROJECT NH 0012(230)171...+

TOTAL SHEETS SHEET B53 B85

Plotting Date:





ESTIMATED QUANTITIES						
ITEM	UNIT	QUANTITY				
Remove Beam Guardrail	Ft	200				
Contractor Furnished Borrow	CuYd	22				
Remove and Replace Topsoil	LS	LUMP SUM				
Type 1 MGS	Ft	50				
Type 1 Guardrail Transition	Each	2				
MGS MASH Flared End Termnal	Each	2				



Structure No. 16-720-217

SD Hwy 1806 MRM 370.35

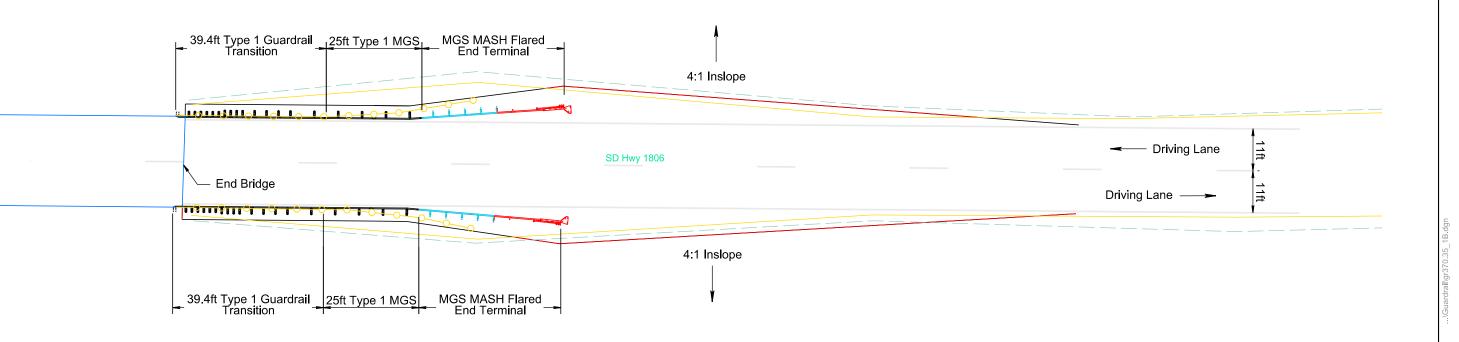
PROJECT NH 0012(230)171...+

TOTAL SHEETS SHEET B54 B85

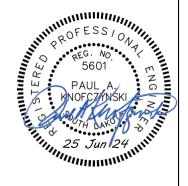
Plotting Date:

6/20/2024





ESTIMATED QUANTITIES					
ITEM	UNIT	QUANTITY			
Remove Beam Guardrail	Ft	160			
Contractor Furnished Borrow	CuYd	30			
Remove and Replace Topsoil	LS	LUMP SUM			
Type 1 MGS	Ft	50			
Type 1 Guardrail Transition	Each	2			
MGS MASH Flared End Termnal	Each	2			



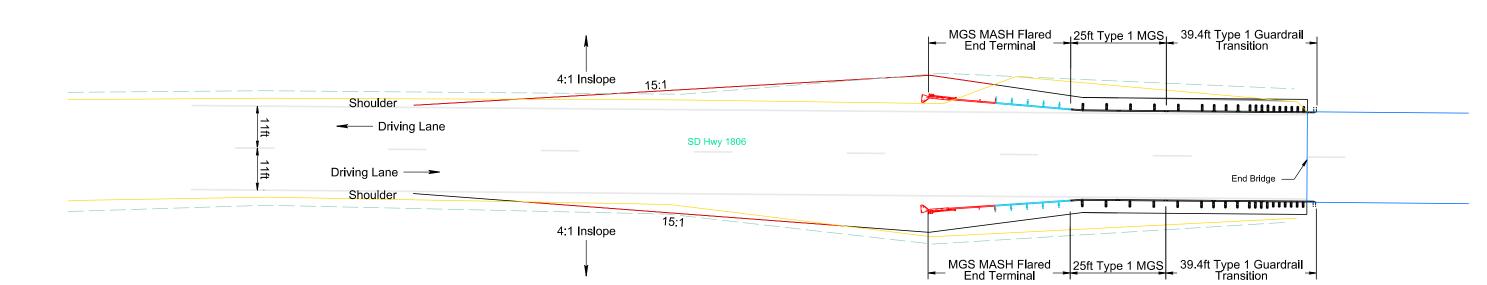
SD 1806 North of US 12 MRM 370.35

PROJECT NH 0012(230)171...+

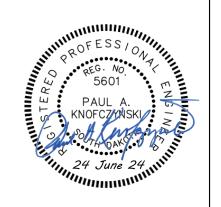
TOTAL SHEETS SHEET B55 B85

Plotting Date:

6/20/2024



ESTIMATED QUANTITIES						
ITEM	UNIT	QUANTITY				
Remove Beam Guardrail	Ft	160				
Contractor Furnished Borrow	CuYd	50				
Remove and Replace Topsoil	LS	LUMP SUM				
Type 1 MGS	Ft	50				
Type 1 Guardrail Transition	Each	2				
MGS MASH Flared End Termnal	Each	2				



PROJECT NH 0012(230)171...+

SHEET TOTAL SHEETS B56 B85

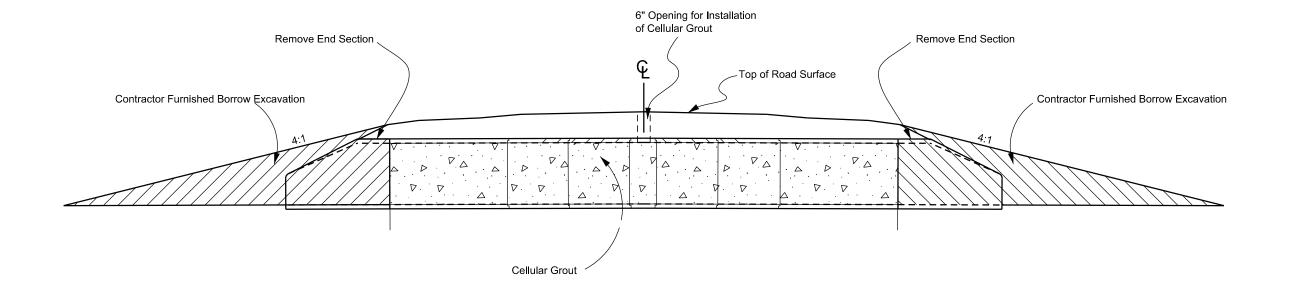
Plotting Date:

Revised By: EJW 9/29/2021 6/20/2024

# EXISTING RC CATTLE PASS

See Table of Pipe Quantities for Locations to be Plugged

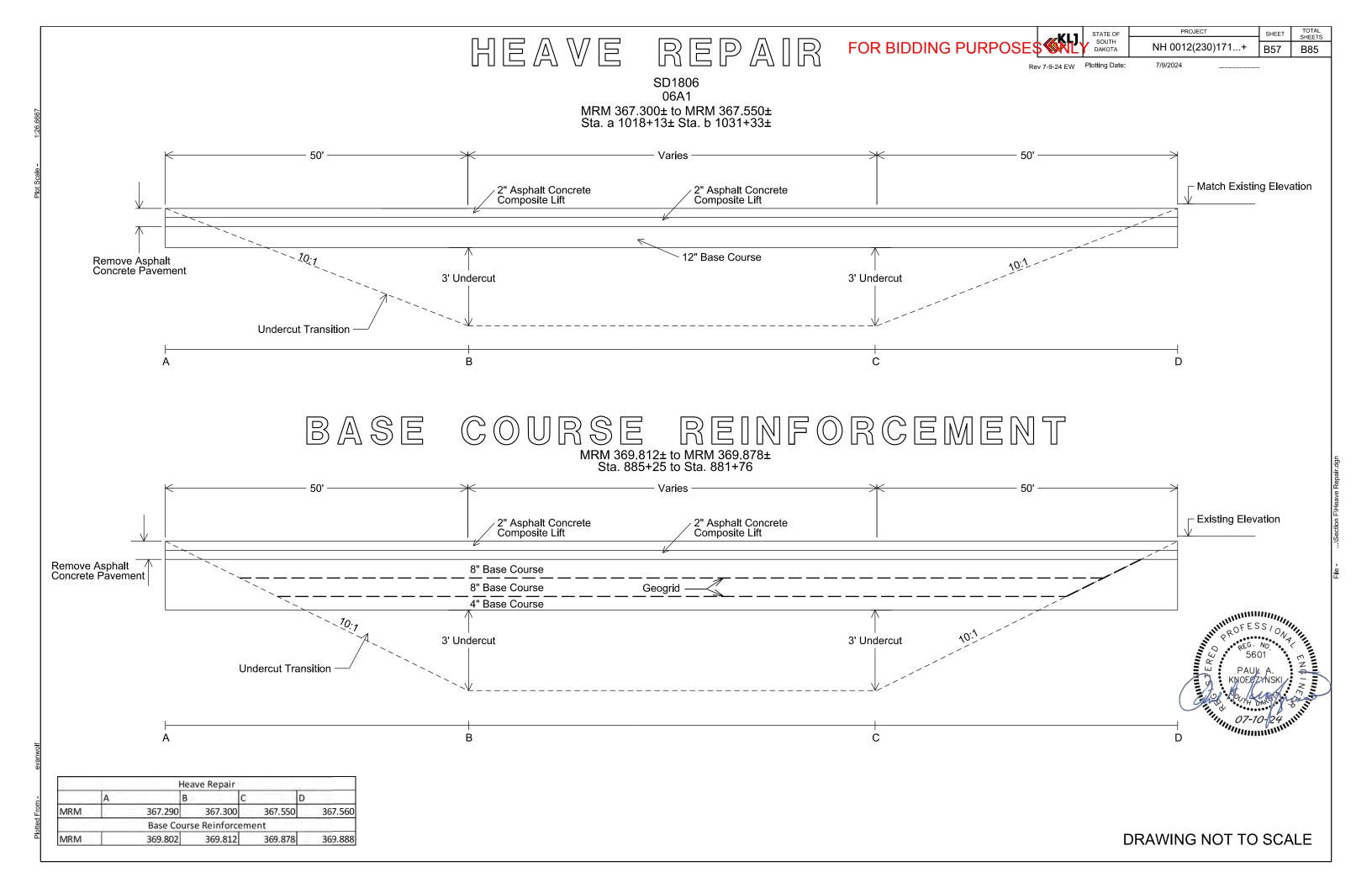
CELLULAR GROUT





- Contractor will match the existing roadway inslope to the satisfaction of the Engineer.
- Refer to plan notes for plugging the remaining void throughout the cattle pass.
- Refer to the plans for quantities.





STATE OF SOUTH DAKOTA

NH 0012(230)171...+

TOTAL SHEETS SHEET B58 B85

Plotting Date:

6/20/2024

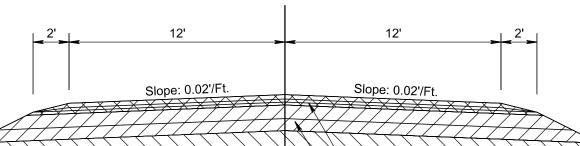
SD1806 06A1

Remove Asphalt Concrete Pavement

MRM 369.812± to MRM 369.878±

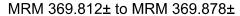
Unclassified Excavation (Salvage top 5" for use as Temporary Surfacing)

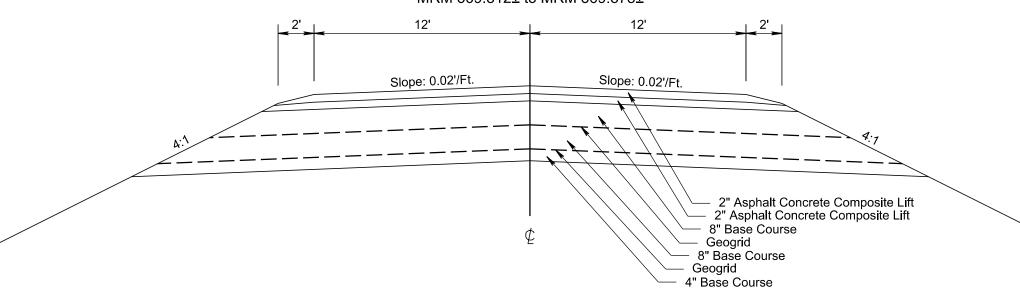
Undercut 3' Existing Subgrade



1.5" Asphalt Concrete, In Place

9.5" Base Course/Gravel Surfacing, In Place





\* This Detail does not show the ultimate resurfacing section which will include Cold Milling Asphalt Concrete and the 2" Class Q3R Asphalt Concrete overlay that will be accomplished after the heave repair has been completed.



STATE OF SOUTH DAKOTA

PROJECT

SHEET B59

NH 0012(230)171...+

TOTAL SHEETS B85

Plotting Date: Rev 7-9-24 EW

2" Class Q3R Asphalt Concrete Lift 2-1.5" Asphalt Concrete Composite Lifts In Place Surfacing (Asphalt or Granular Base) In Place Surfacing (Asphalt or Granular Base) Soil Backfill Varies RC Pipe Culvert Varies 3" Bedding 15% of Pipe Dia Flow Line of Culvert 30% of Pipe Dia at Centerline of Roadway 1 Ft. min. Select Fill Material (See Mainline Cross Pipe Replacement Notes)

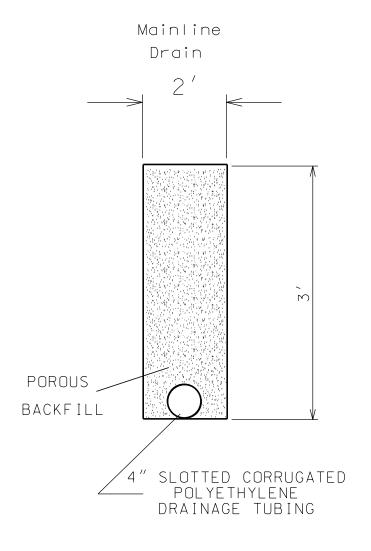


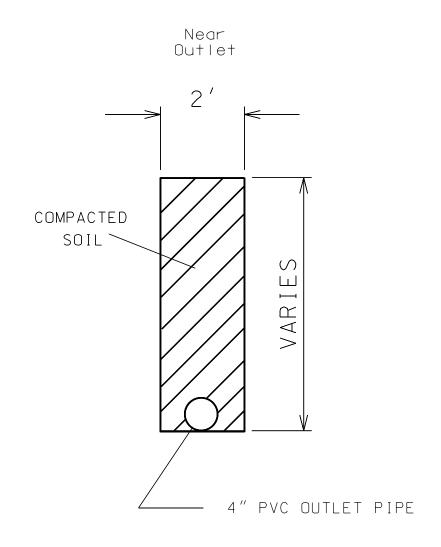
\* This Detail does not show the ultimate resurfacing section which will include Cold Milling Asphalt Concrete and the Class Q3R Asphalt Concrete Surfacing that will be accomplished after the culvert replacement has been completed.

Plotting Date:

# TYPICAL UNDERDRAIN INSTALLATION

SDI806 Underdrain





UNDERDRAINS WILL BE CONSTRUCTED IN ACCORDANCE WITH SECTION 680 OF THE SPECIFICATIONS

The elevation view above is typical for either a ditch cut or fill section. Entrances that vary from above should be specified in the plans.

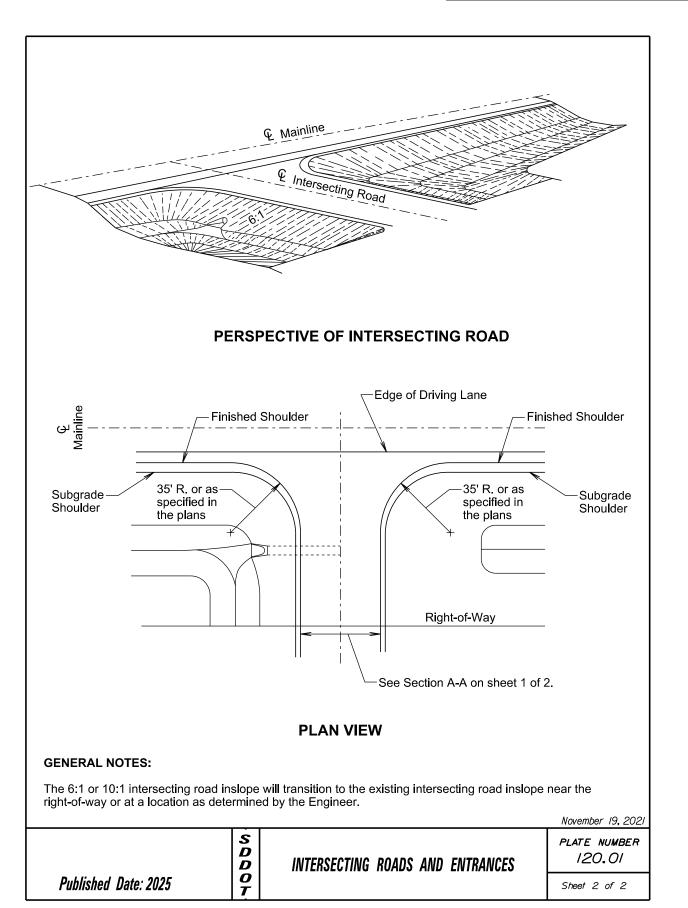
Pipe length will be adjusted if necessary during construction to obtain the 6:1 slope. For grading projects, the pipe length is estimated typically using a 4" thickness of surfacing directly over the subgrade above the pipe.

The transition area between the mainline inslope and the entrance or intersecting road inslope will be rounded to eliminate an abrupt transition.

The turning radii will be 35' for intersecting roads and entrances unless stated otherwise in the plans.

			November 19, 2021
	S D D	INTERSECTING ROADS AND ENTRANCES	PLATE NUMBER 120.01
Published Date: 2025	$\left  \begin{array}{c} oldsymbol{o} \\ oldsymbol{\tau} \end{array} \right $		Sheet I of 2

FOR BIDDING PURPOSES ONLY STATE OF SOUTH DAKOTA NH 0012(230)171...+ B61 B85

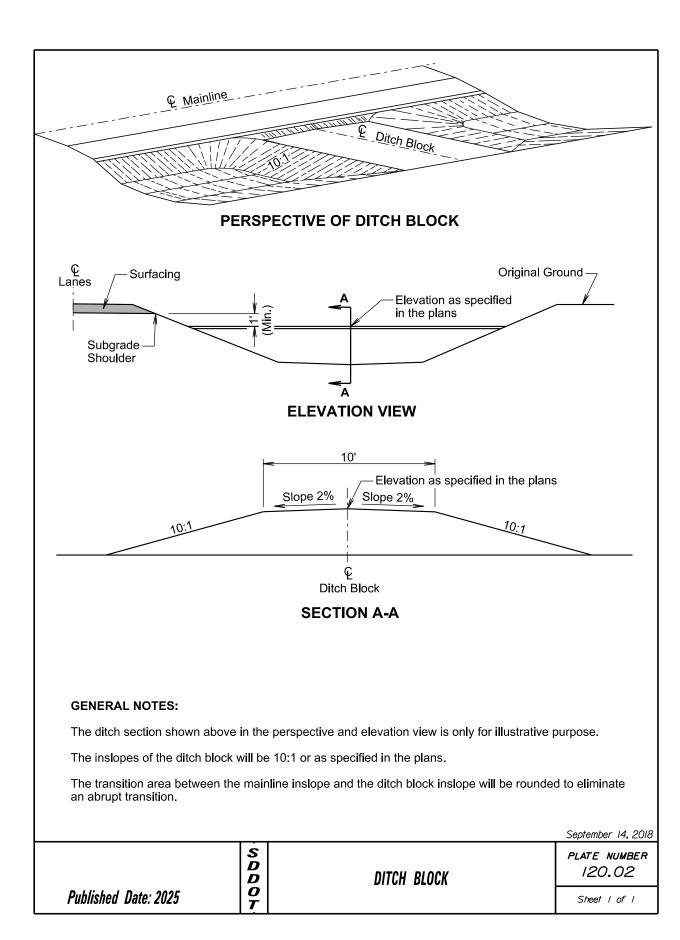


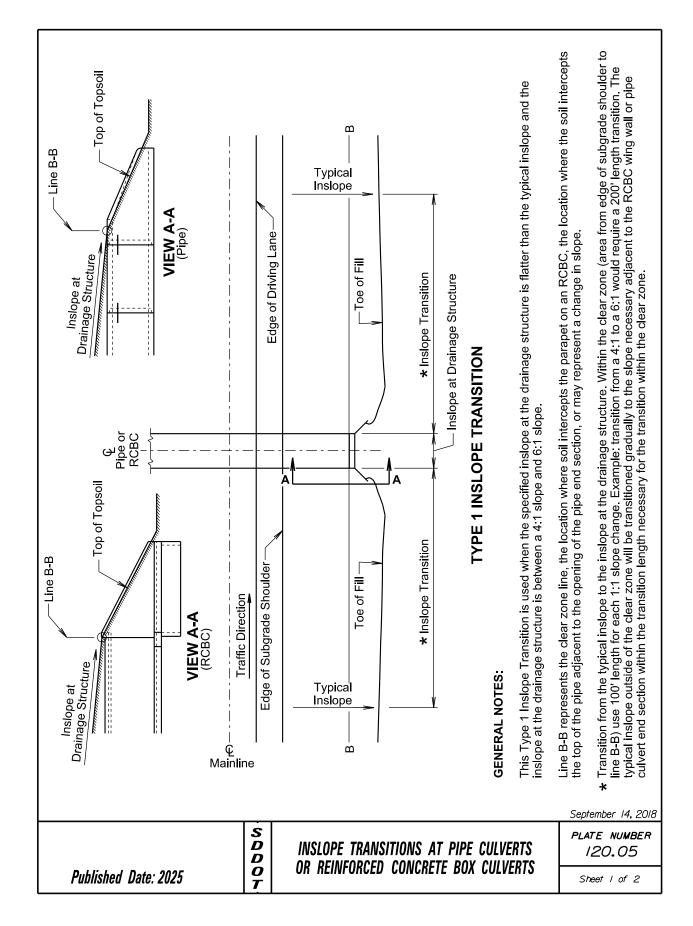
STATE OF

PROJECT NH 0012(230)171...+

TOTAL SHEETS SHEET B62 B85







STATE OF

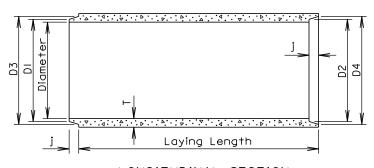
TOTAL SHEETS

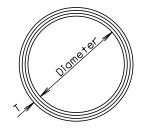
PROJECT SHEET NH 0012(230)171...+ B63 B85

# TOLERANCES IN DIMENSIONS

Diameter:  $\pm 1.5\%$  for 24" Dia. or less and  $\pm 1\%$  whichever is more for 27" Dia. or greater. Diameters at joints:  $\pm$   $\frac{1}{6}$ " for 30" Dia. or less and  $\pm$   $\frac{1}{4}$ " for 36" or greater. Length of joint (j):  $\pm$   $\frac{1}{4}$ ".

Wall thickness (T): not less than design T by more than 5% or  $\frac{3}{16}$ ", whichever is greater. Laying length: shall not underrun by more than  $\frac{1}{2}$ ".





# LONGITUDINAL SECTION

# END VIEW

#### GENERAL NOTES:

Construction of R.C.P. shall conform to the requirements of Section 990 of the Specifications.

Not more than 2 four-foot sections shall be permitted near the ends of any culvert. Four-foot lengths shall be used only to secure the required length of culvert.

Diam. (in.)	Approx. Wt./Ft. (Ib.)	T (in.)	J (in.)	DI (in.)	D2 (in.)	D3 (in.)	D4 (in.)
12	92	2	13/4	131/4	135/8	13%	141/4
15	127	21/4	2	161/2	16%	171/4	175/8
18	168	21/2	21/4	195/8	20	20¾	203/4
21	214	23/4	21/2	22 1/8	231/4	23¾	241/8
24	265	3	23/4	26	26¾	27	273/8
27	322	31/4	3	291/4	295/8	30 <sup>1</sup> / <sub>4</sub>	30%
30	384	31/2	31/4	323/8	323/4	331/2	33%
36	524	4	33/4	38¾	391/4	40	401/2
42	685	41/2	4	451/8	455/8	461/2	47
48	867	5	41/2	511/2	52	53	531/2
54	1070	51/2	41/2	577/8	58¾	59 <b>%</b>	59%
60	1296	6	5	641/4	64¾	66	661/2
66	1542	61/2	51/2	70%	711/8	$72^{1}/_{2}$	73
72	1810	7	6	77	771/2	79	791/2
78	2098	71/2	61/2	83%	83%	85 <b>%</b>	861/8
84	2410	8	7	89¾	901/4	921/8	925/8
90	2740	81/2	7	95¾	961/4	981/8	985/8
96	2950	9	7	1021/8	1025/8	1041/2	105
102	3075	91/2	71/2	109	1091/2	1111/2	112
108	3870	10	71/2	1151/2	116	118	1181/2

June 26, 2015 PLATE NUMBER

450.01

Sheet | of |

S D D O T REINFORCED CONCRETE PIPE Published Date: 2025

Transition from typical inslope to the inslopes adjacent to the drainage structure. Within the clear zone (area from edge of subgrade shoulder to line B-B) use 100' length for each 1:1 slope change. Example: transition from a 4:1 to a 6:1 would require a 200' length transition. The typical inslope outside of the clear zone will be transitioned to a 3:1 inslope within the transition length necessary for the transition within the clear zone. Line B-B represents the clear zone line, the location where soil intercepts the parapet on an RCBC, the location where the soil intercepts the top of the pipe adjacent to the opening of the pipe end section, or may represent a change in slope. This Type 2 Inslope Transition is used when the specified inslope at the pipe or RCBC is flatter than a 6:1 slope. VIEW A-, (Pipe) \*\* Inslope Transition Inslope Flatter than a 6:1 Slope Toe of Fill of Driving Lane Inslope at drainage structure 2 INSLOPE TRANSITION 6 1 Inslope Transition from Inslope at drainage structure to a 6:1 inslope and 3:1 inslope. Inslope E Pipe or RCBC Top of Topsoil Inslope TYPE 3:1 Inslope Line B-B Inslope Transition Edge of Subgrade Shoulder of Fill-Traffic Direction <u>0</u> VIEW A-A (RCBC) Inslope Flatter than a 6:1 Slope GENERAL NOTES: Typical Inslope Ω Mainline \* September 14, 2018 S D D O PLATE NUMBER

INSLOPE TRANSITIONS AT PIPE CULVERTS

OR REINFORCED CONCRETE BOX CULVERTS

120.05

Sheet 2 of 2

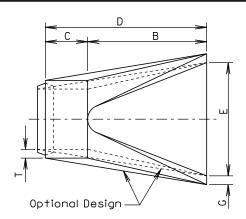
Typical

Inslope

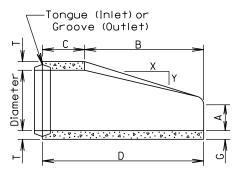
of Topsoil

Published Date: 2025

Line B-B

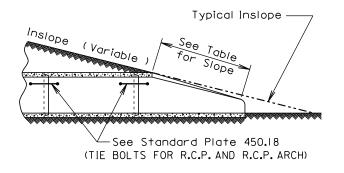


TOP VIEW



LONGITUDINAL SECTION

Published Date: 2025

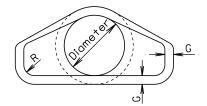


#### SLOPE DETAIL

#### GENERAL NOTES:

Lengths of concrete pipe shown on plan sheets are between flared ends only.

Construction of R.C.P. Flared End shall conform to the requirements of Section 990 of the Specifications.



END VIEW

Dia. (in.)	Approx. Wt. of Section (lbs.)	Approx. Slope (X to Y)	T (in.)	A (in.)	B (in.)	C (in.)	D (in.)	E (in.)	G (in.)	R (in.)
12	530	2.4: 1	2	4	24	48 1/8	721/8	24	2	11/2
15	740	2.4: 1	21/4	6	27	46	73	30	21/4	11/2
18	990	2.3:	21/2	9	27	46	73	36	21/2	11/2
21	1280	2.4: 1	23/4	9	36	371/2	731/2	42	23/4	11/2
24	1520	2.5: 1	3	91/2	431/2	30	731/2	48	3	11/2
27	1930	2.5:	31/4	101/2	491/2	24	731/2	54	31/4	11/2
30	2190	2.5: 1	$3\frac{1}{2}$	12	54	193/4	73¾	60	31/2	11/2
36	4100	2.5: 1	4	15	63	343/4	973/4	72	4	11/2
42	5380	2.5: 1	41/2	21	63	35	98	78	41/2	11/2
48	6550	2.5: 1	5	24	72	26	98	84	5	11/2
54	8240	2: 1	51/2	27	65	331/4	981/4	90	51/2	11/2
60	8730	1.9:1	6	35	60	39	99	96	5	11/2
66	10710	1.7:1	61/2	30	72	27	99	102	51/2	11/2
72	12520	1.8:1	7	36	78	21	99	108	6	11/2
78	14770	1.8:1	71/2	36	90	21	111	114	61/2	11/2
84	18160	1.6:1	8	36	901/2	21	1111/2	120	61/2	11/2
90	20900	1.5:1	81/2	41	871/2	24	1111/2	132	61/2	6

June 26, 2015

S D D 0

R. C. P. FLARED ENDS

PLATE NUMBER 450.10

> Published Date: 2025 Sheet I of I

S D D 0 7

R. C. P. SLOPED ENDS

\* 24

\* 30 31/2

9

60

PLATE NUMBER 450.13

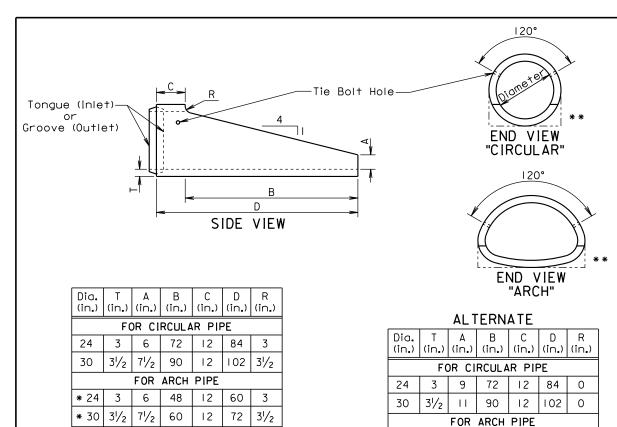
September 22, 2006

48 | 12 | 60 | 0

12

72

Sheet I of I



\* Equivalent Diameter of Circular R.C.P.

66

\* 42 | 4<sup>1</sup>/<sub>2</sub> | 10 | 77<sup>1</sup>/<sub>4</sub> | 18<sup>3</sup>/<sub>4</sub> |

30

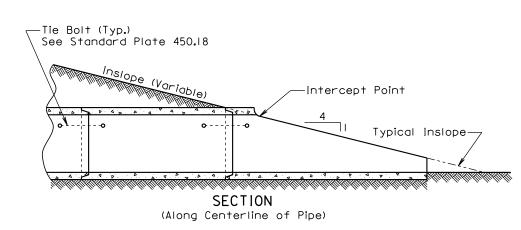
96

96

0

\* 36 | 4½ | 85/<sub>8</sub>

\*\* Acceptable Flat Bottom Alternate.



#### GENERAL NOTE:

The length of concrete pipe shown in the construction plans is between sloped ends.

STATE OF DAKOTA

PROJECT NH 0012(230)171...+

TOTAL SHEETS SHEET B65 B85



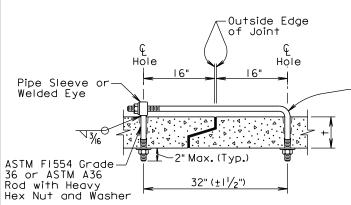
Rod Dia. Pipe Sleeve Dia. Wall "t" (in.) ≤ 31/4  $3\frac{1}{2}-6\frac{1}{2}$ 11/4

Pipe Dia.

END VIEW

"CIRCULAR"

Published Date: 2025



#### GENERAL NOTES:

Tie bolts shall conform to ASTM F1554 Grade 36 or ASTM A36. Nuts shall be heavy hex conforming to ASTM A563. Washers shall conform to ASTM F436.

Pipe Sleeve shall conform to ASTM A500 or A53, Grade B.

Galvanize adjustible eye bolt tie assembly in accordance with ASTM A153.

-ASTM FI554 Grade 36 or ASTM A36 Tie Bolt with 2 Heavy Hex Nuts and 2 Washers

#### ADJUSTABLE EYE BOLT TIE

# Bolt Dia. (in.) (in.) < 48 > 48 6 ASTM A307 Bolt $\angle 6$ " × 4" × $\frac{3}{4}$ " × L $\rightarrow$ with Heavy Hex Nut and 2 Washers -Bolts may be reversed ANGLE AND BOLT TIE

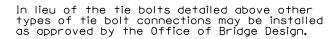
#### GENERAL NOTES:

Angles shall conform to ASTM A36.

Bolts shall conform to ASTM A307. Nuts shall be heavy hex conforming to ASTM A563. Washers shall conform to ASTM F436.

Galvanize angles, bolts, nuts, and washers in accordance with ASTM A153.

#### GENERAL NOTES:



All pipe sections of R.C.P. and R.C.P. Arch shall be tied with tie bolts except for pipe located between drop inlets, manholes, and junction boxes. All pipe sections of pipes that only enter or exit drop inlets, manhole, and junction boxes shall be tied with tie bolts.

There will be no separate measurement or payment for the tie bolts. The cost for furnishing and installing the tie bolts shall be incidental to the contract unit price per foot for the corresponding bid item for R.C.P. or R.C.P. Arch.

February 28, 2013

D D O

END VIEW

"ARCH"

TIE BOLTS FOR R.C.P. AND R.C.P. ARCH

PLATE NUMBER 450.18

> Published Date: 2025 Sheet I of I

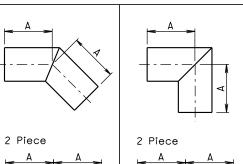
# D D 0

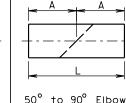
C.M.P. FABRICATED LENGTHS FOR ELBOWS

PLATE NUMBER *450.32* 

June 26, 2001

Sheet | of |





3 Piece 90° Elbow

5 10 45 EIDOW			30 10	30 EI	DOW	30 E100W					
Diameter	Δ	L	Diameter	Α	L	Diameter	Α	В	С	L	
Inches	Feet	Feet	Inches Feet Fe		Feet	Inches	Inches			Feet	
12	1	2	12	2	4	12	251/2	11	181/2	4	
15		2	15	2	4	15	261/2	12	18	4	
18		2	18	2	4	18	27	14	۱7	4	
21	2	4	21	2	4	21	27	15	161/2	4	
24	2	4	24	2	4	24	271/2	16	16	4	
27	2	4	27	2	4	27	271/2	17	151/2	4	
30	2	4	30	3	6	30	40	19	261/2	6	
33	2	4	33	3	6	33	40	20	26	6	
36	2	4	36	3	6	36	401/2	21	251/2	6	
42	2	4	42	3	6	42	41	23	241/2	6	
48	2	4	48	4	8	48	531/2	26	35	8	
54	3	6	54	4	8	54	54	28	34	8	
60	3	6	60	4	8	60	541/2	31	321/2	8	
66	3	6	66	4	8	66	54	33	311/2	8	
72	3	6	72	5	10	72	671/2	36	42	10	
78	3	6	78	5	10	78	68	39	401/2	10	
84	3	6	84	5	10	84	68 <sup>1</sup> / <sub>2</sub>	41	391/2	10	
90	3	6	90	6	12	90	70	46	37	10	
96	3	6	96	6	12	96	82	46	49	12	

FABRICATED ELBOW LENGTHS FOR ALL CORRUGATIONS

# GENERAL NOTES:

All dimensions shown are nominal.

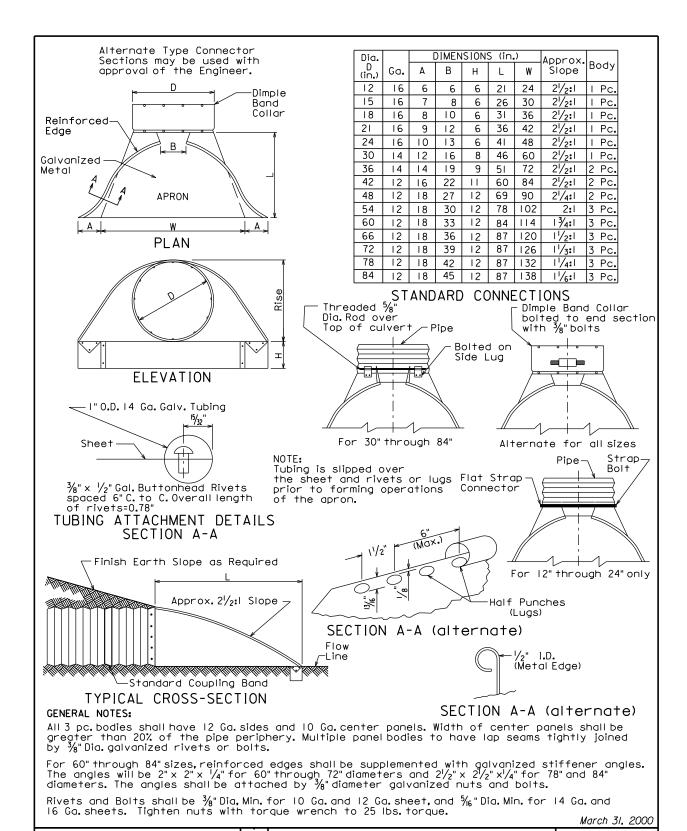
5° to 45° Fibow

L = Linear Feet of C.M.P. required to fabricate fitting.

STATE OF DAKOTA

PROJECT NH 0012(230)171...+

TOTAL SHEETS SHEET B66 B85



C.M.P. FLARED ENDS

S

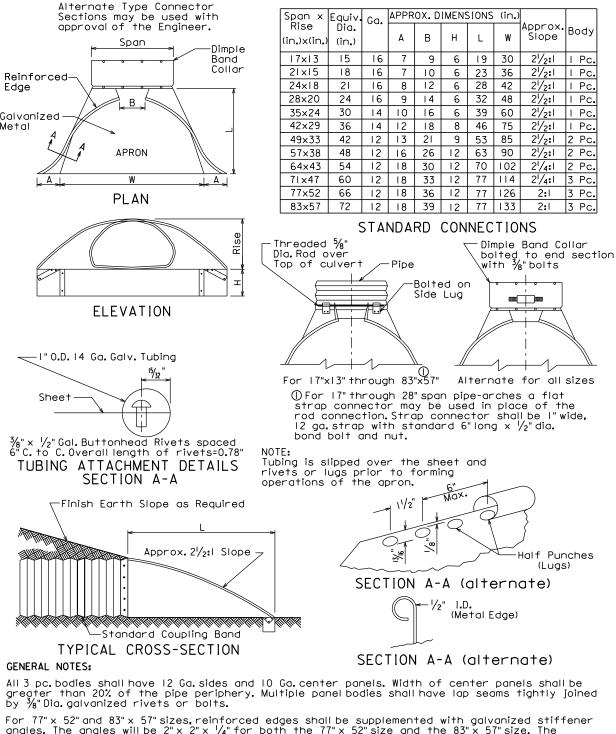
D

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Published Date: 2025



All 3 pc. bodies shall have 12 Ga. sides and 10 Ga. center panels. Width of center panels shall be

For 77" x 52" and 83" x 57" sizes, reinforced edges shall be supplemented with galvanized stiffener angles. The angles will be 2" x 2" x  $\frac{1}{4}$ " for both the 77" x 52" size and the 83" x 57" size. The angles shall be attached by  $\frac{3}{6}$ " Dia galvanized nuts and bolts.

Rivets and Bolts shall be  $\frac{3}{8}$ " Dia, Min. for 10 Ga. and 12 Ga. sheet, and  $\frac{5}{16}$ " Dia. Min. for 14 Ga. and 16 Ga. sheets. Tighten nuts with torque wrench to 25 lbs. torque.

March 31, 2000 PLATE NUMBER

D D 0 Published Date: 2025

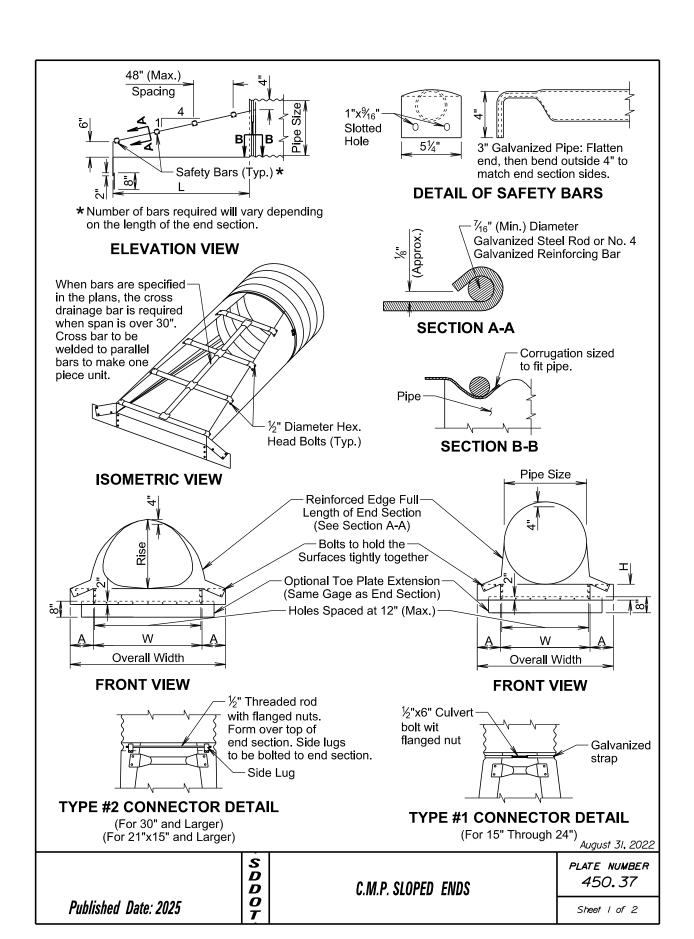
PLATE NUMBER

450.35

Sheet I of I

C.M.P. ARCH FLARED ENDS

*450.36* Sheet I of I



# FOR BIDDING PURPOSES ONLY

Π	STATE OF SOUTH DAKOTA	PROJECT	SHEET	TOTAL SHEETS	
Y		NH 0012(230)171+	B67	B85	

	ARCH C.M.P. SLOPED ENDS									
Equiv.	(Incl	nes)	(Min.)	Thick.	Dim	ensi	ons (	Inches)	L Dime	ensions
Dia. (Inch)	Span	Rise	Inch	Gage	Α	Н	W	Overall Width	Slope	Length (Inch)
18	21	15	.064	16	8	6	27	43	4:1	20
21	24	18	.064	16	8	6	30	46	4:1	32
24	28	20	.064	16	8	6	34	50	4:1	40
30	35	24	.079	14	12	9	41	65	4:1	56
36	42	29	.109	12	12	9	48	72	4:1	76
42	49	33	.109	12	16	12	55	87	4:1	92
48	57	38	.109	12	16	12	63	95	4:1	112
54	64	43	.109	12	16	12	70	102	4:1	132
60	71	47	.109	12	16	12	77	109	4:1	148
72	83	57	.109	12	16	12	89	121	4:1	188

CIRCULAR C.M.P. SLOPED ENDS										
Pipe	(Min.) Thick. Dimensions (Inches) L Dimensions									
Dia. (Inch)	Inch	Gage	Α	Ι	W	Overa <b>ll</b> Width	Slope	Length (Inch)		
15	.064	16	8	6	21	37	4:1	20		
18	.064	16	8	6	24	40	4:1	32		
21	.064	16	8	6	27	43	4:1	44		
24	.064	16	8	6	30	46	4:1	56		
30	.109	12	12	9	36	60	4:1	80		
36	.109	12	12	9	42	66	4:1	104		
42	.109	12	16	12	48	80	4:1	128		
48	.109	12	16	12	54	86	4:1	152		
54	.109	12	16	12	60	92	4:1	176		
60	.109	12	16	12	66	98	4:1	200		

#### **GENERAL NOTES:**

Safety bars will be provided when specified in the plans.

Sloped ends will be fabricated from galvanized steel and will conform to the requirements of the Specifications.

Safety bars will be fabricated from steel schedule 40 pipe in conformance with ASTM A53, grade B or HSS 3.5x.216 in conformance with ASTM A500, grade B.

Slotted holes for safety bar attachment will be provided for all end sections.

Attachment to circular pipes 15" through 24" diameter will be made with Type #1 straps. All other sizes will be attached with Type #2 rods and lugs.

When stated in the plans, optional toe plate extension will be punched and bolted to end section apron lip with %" diameter galvanized bolts. Steel for toe plate extension will be same gauge as end section. Dimensions will be overall width less 6" by 8" high.

Installation will be performed in accordance with the Specifications.

Cost of all work and materials required for fabrication and installation of sloped ends will be incidental to the bid items for the various sizes of sloped ends.

August 31, 2022

	SDD	C.M.P. SLOPED ENDS	PLATE NUMBER 450.37
Published Date: 2025			Sheet 2 of 2

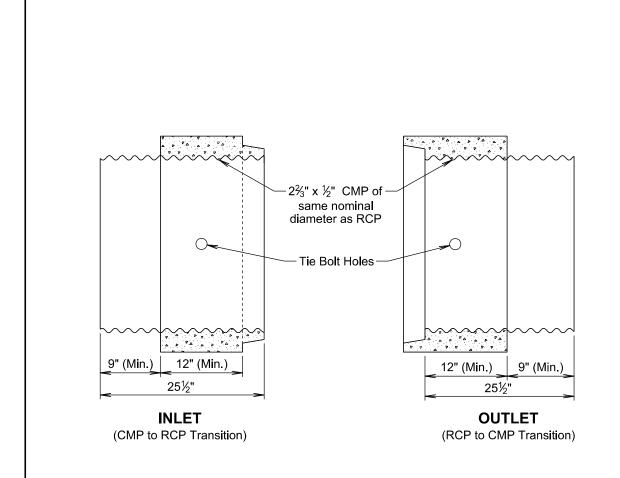
14'-0" or 16'-6"

14'-0" or 16'-6"

STATE OF

PROJECT TOTAL SHEETS SHEET NH 0012(230)171...+ B68 B85 DAKOTA

-3½" Dia. x 6'-6"



# GENERAL NOTE:

Arch pipe transitions will be fabricated similar to the round transition shown above.

S D

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O T

All pipe transitions will be precast as shown. Alternate designs other than shown will need to be approved by the Engineer.

November 19, 2022

Published Date: 2025

C.M.P. TO R.C.P. TRANSITION R.C.P. TO C.M.P. TRANSITION PLATE NUMBER *450.50* 

Published Date: 2025 Sheet I of I

2

TYPE OF FENCE

TYPE DESCRIPTION

3 Barbed Wires

4 Barbed Wires

5 Barbed Wires

26" Woven Wire

with 2 Barbed Wires

26" Woven Wire

with 4 Barbed Wires

32" Woven Wire

with 3 Barbed Wires

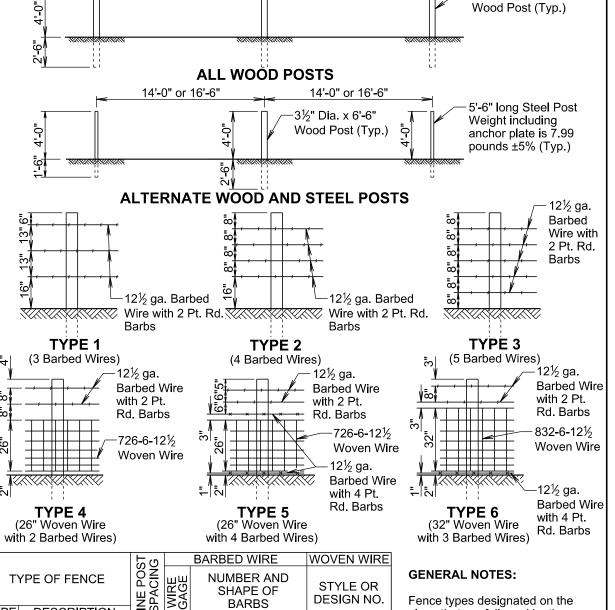
LINE

|16'-6"|12%|

|16'-6"|12%|

16'-6" 12%

14'-0" 12½



STYLE OR

DESIGN NO.

726-6-12½

726-6-12½

832-6-121/2

SHAPE OF

BARBS

2 Point Round

2 Point Round

2 Point Round

2 Point Round

14'-0" 12½ 2 wires with 2 Pt. Rd. 2 wires with 4 Pt. Rd.

14'-0" 12½ 2 wires with 2 Pt. Rd. 1 wire with 4 Pt. Rd.

S

D D

0

#### **GENERAL NOTES:**

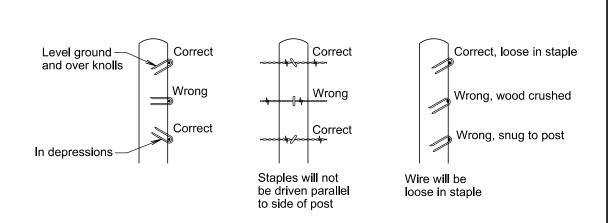
Fence types designated on the plans that are followed by the letter S will have smooth (barbless) wires.

When type 5S or 6S is designated the bottom wire may be barbed, smooth, or left off.

All degrees of curvature stated for fence are at centerline of roadway. June 26, 2019

PLATE NUMBER 620.01 RIGHT-OF-WAY FENCE Sheet I of I





# STAPLE INSTALLATION

## **GENERAL NOTES:**

The Right-of-Way fence will consist of barbed wire or a combination of woven wire and barbed wire. The barbed wire and/or woven wire will be fastened to all wood posts or fastened to alternating wood and steel posts. Only wood posts will be used for brace panels. Gates will be of the type designated in the plans or as otherwise directed by the Engineer. Fence will be constructed conforming to the details on the standard plates and in the plans unless otherwise directed by the Engineer.

Right-of-Way fence on Interstate Projects will be constructed one foot within the Interstate Right-of-Way lines except at bridge openings, cattle passes, and as otherwise directed by the Engineer.

Right-of-Way fence other than on Interstate Projects will be constructed within one foot of the Right-of-Way on the Landowner's side except at bridge openings, cattle passes, and as otherwise directed by the Engineer.

Barbs will be fabricated from zinc coated 14 ga. wire. Two point barbs will be wrapped twice around one main strand at four-inch spacings and the four point barbs will be interlocked and wrapped around both main strands at five-inch spacings.

The gages of wire and wood post lengths and sizes are the minimum acceptable unless otherwise specified in the plans. The tolerances for steel posts will be as stated in AASHTO M281. Woven wire will conform to design and specifications of ASTM A116 and barbed wire will conform to ASTM A121.

June 26, 2019

S D D STAPLE INSTALLATION AND GENERAL RIGHT-OF-WAY FENCE NOTES Published Date: 2025

PLATE NUMBER 620.02

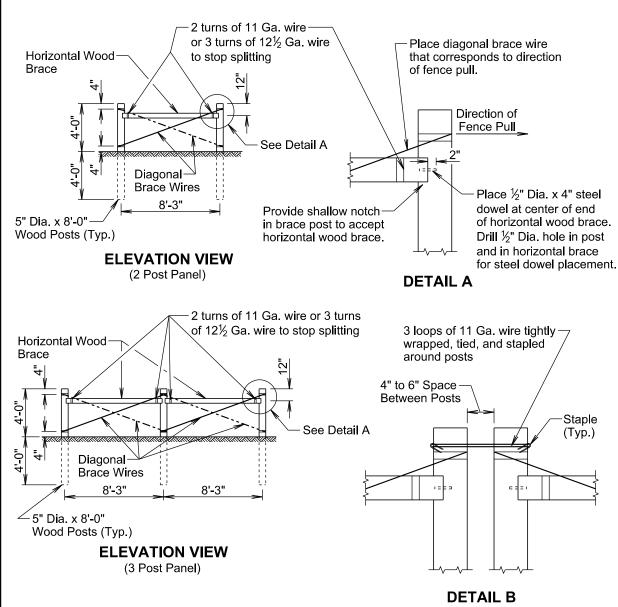
Published Date: 2025 Sheet I of I

D D O

BRACE PANELS AND APPLICATIONS OF BRACE PANELS PLATE NUMBER 620.03

March 31, 2024

Sheet I of 3



#### **GENERAL NOTES:**

Two Post Panels will be installed at least every 1320' between corners.

Two Post Panels will be installed at any sharp vertical angle crest points and as directed by the Engineer.

Horizontal wood braces will consist of 4" dia. x 8' wood posts or rough 4" x 4" x 8' timbers.

Diagonal brace wires will be fabricated with 4 strands of 9 Ga. galvanized wire twisted tight. The diagonal brace wires will be installed in accordance with the direction of the fence pull. Two diagonal brace wires are required if fence pull is in both directions.

STATE OF

PROJECT NH 0012(230)171...+ B70

TOTAL SHEETS B85

SHEET

SPACING OF 2 POST PANELS WITHIN CURVES RADIUS OF CURVE | SPACING OF 2 POST PANEL Greater than 1800 Ft. **\*\*** 1320' \*\*At P.C., P.T., and at every Less than 1800 Ft. 1320' between P.C. and P.T.

\*\* Fence lengths greater than 1320' and less than 2640' place 2 Post Panel approximately at midpoint.

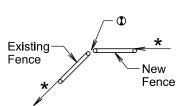
#### **GENERAL NOTE:**

All radius of curvature stated for fence are at centerline of roadway.

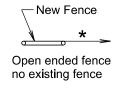
If fence length is less than 600' to next corner use

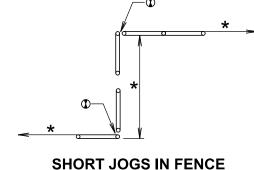
\* a 2 post panel.

\* If fence length is greater than 600' to next corner use a 3 post panel.

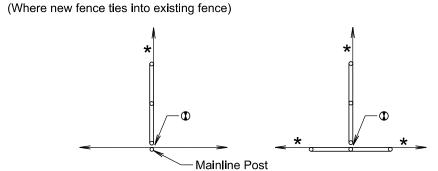


① See Detail B on Sheet 1 of 3.

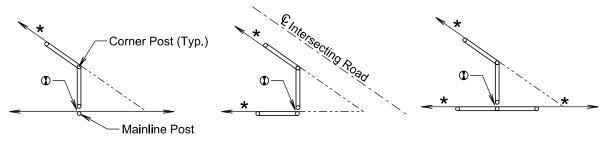




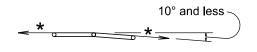
## **BEGIN OR END FENCE**



# **CROSS FENCE**



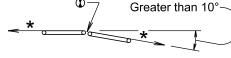
# **SHARP ANGLES IN CROSS FENCE**



D

D

0



Additional fence panel is NOT required when an angle in the mainline fence is 10° and less.

Additional fence panel is required when an angle in the mainline fence is greater than 10°.

#### **ANGLES IN MAINLINE FENCE**

March 31, 2024

Published Date: 2025

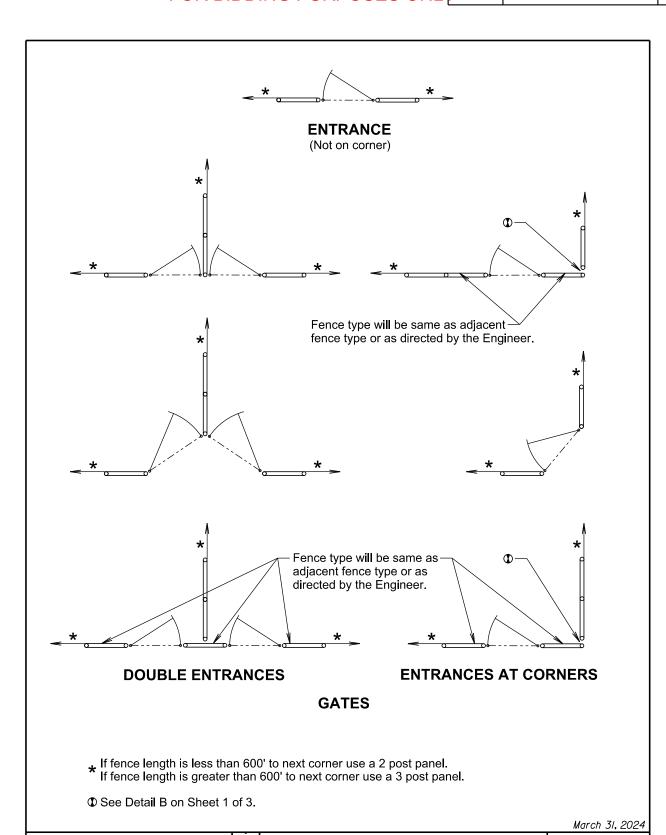
BRACE PANELS AND APPLICATIONS OF BRACE PANELS PLATE NUMBER 620.03

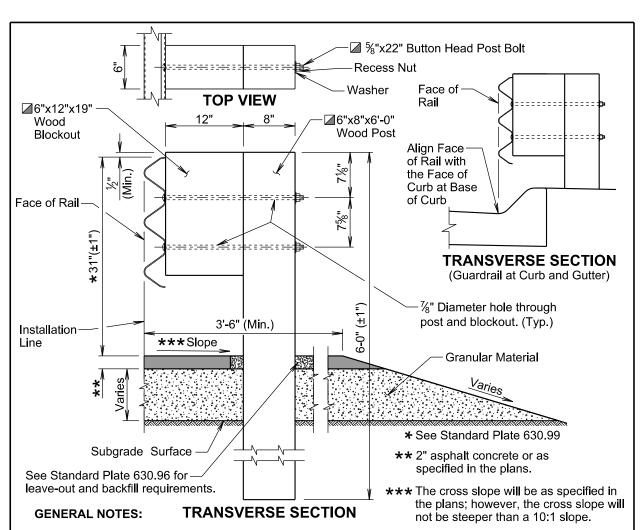
Published Date: 2025 Sheet 2 of 3

S **D D O** 

BRACE PANELS AND APPLICATIONS OF BRACE PANELS PLATE NUMBER 620.03

Sheet 3 of 3





Asphalt concrete will be the same type used elsewhere on the project or will be as specified in the plans. If asphalt concrete is not specified in the plans, the asphalt concrete will conform to the Specifications for "Asphalt Concrete Composite."

Granular material will be the same type used elsewhere on the project or will be as specified in the plans. If granular material type is not specified in the plans, the material will conform to the Specifications for "Base Course". The granular material will be placed the same thickness as the mainline surfacing or as specified in the plans.

Topsoil is not shown in the transverse section drawing.

✓ The post and blockout illustrated above is typical for single thrie beam guardrail. When other variations of posts and blockouts are specified on other standard plates (e.g. transitions) then the posts and blockouts will be as specified on the other standard plates or as specified in the plans.

Slots in the rails will be provided as specified in the plans and by the manufacturer. A drilled hole through the rail is not allowed as a replacement for a slot. If the Contractor must create a slot, a cutting torch or plasma cutter is not allowed. The slot edges will be smooth and free of burrs or notches.

The top of post and top of block will have a true square cut. The top of block will be a maximum of  $\pm \frac{1}{2}$  inch from the top of the post.

September 14, 2019

	S D D	THRIE BEAM GUARDRAIL	PLATE NUMBER 630.01	
Published Date: 2025			Sheet I of 5	

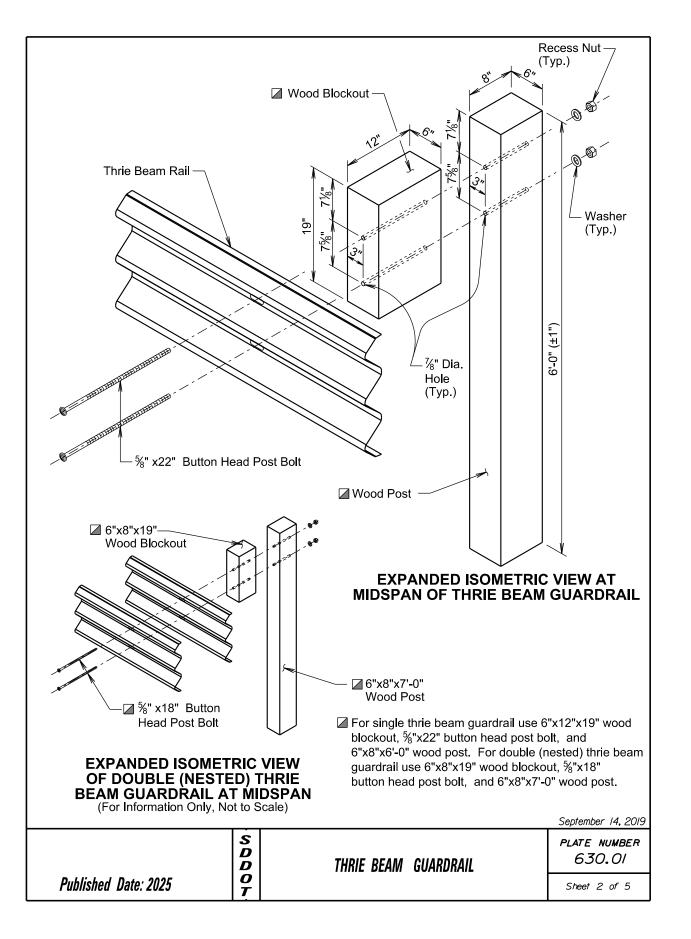
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STATE OF SOUTH

PROJECT
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SHEET TOTAL SHEETS

B71 B85

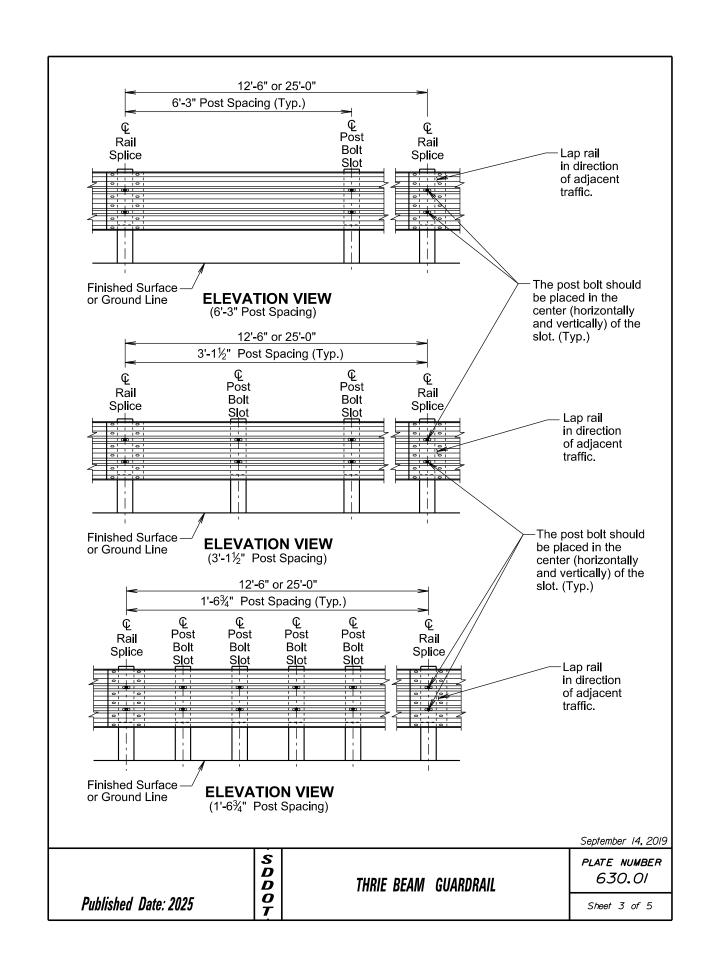


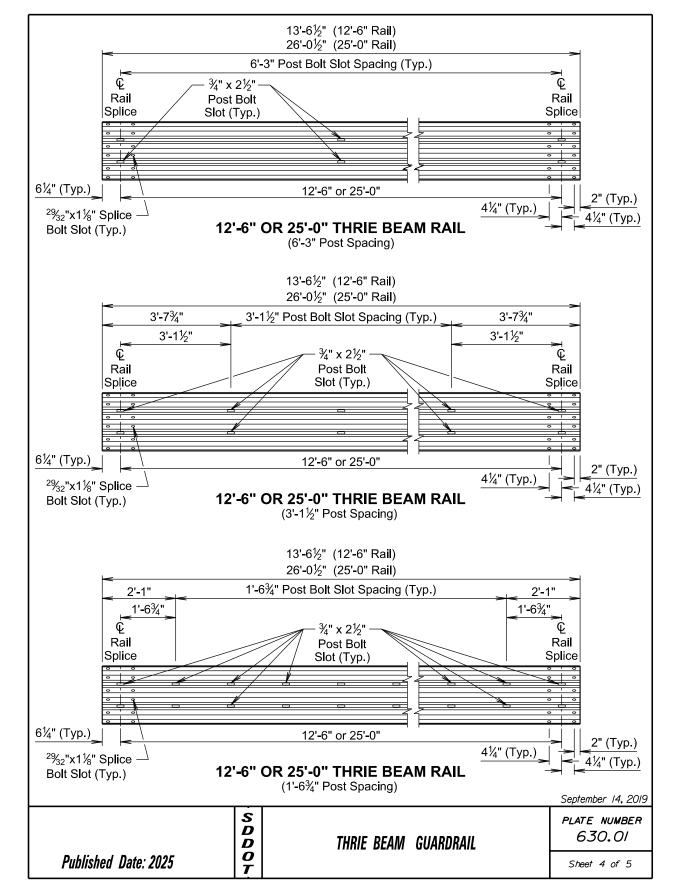
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TOTAL SHEETS

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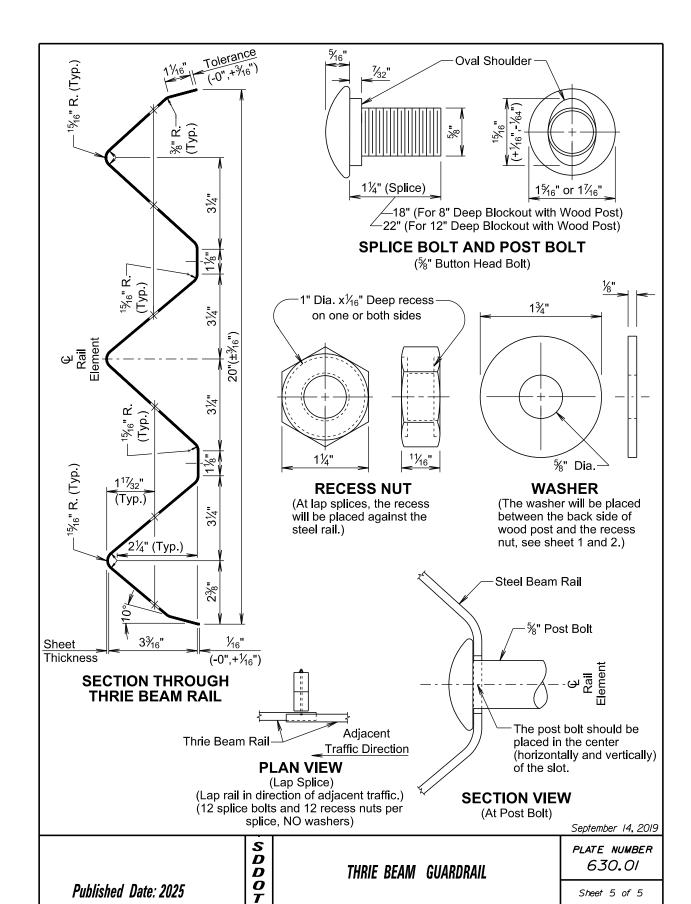






PROJECT NH 0012(230)171...+

TOTAL SHEETS SHEET B73 B85



Published Date: 2025

TYPE AND DETAILS OF MGS								
Type of MGS	W Beam Rail Single or Double (Nested)	Blockout Size	Blockout Material		Post Material	Post Spacing		
1	Single	6"x12"x14"	Wood	6"x8"x6'-0"	Wood	6'-3"		
1C	Single	6"x12"x14"	Wood	6"x8"x7'-6"	Wood	6'-3"		
2	Single	6"x12"x14"	Wood	6"x8"x6'-0"	Wood	3'-1½"		
3	Single	6"x12"x14"	Wood	6"x8"x6'-0"	Wood	1'-6¾"		
4	Double	6"x12"x14"	Wood	6"x8"x6'-0"	Wood	6'-3"		

STANDARD PLATE REFERENCE						
Type of MGS	See Standard Plate(s)					
1	630.20, 630.22					
1C	630.20, 630.25					
2	630.20					
3	630.20					
4	630,20					

# **GENERAL NOTES:**

Sheet 5 of 5

Asphalt concrete will be the same type used elsewhere on the project or will be as specified in the plans. If asphalt concrete is not specified in the plans, the asphalt concrete will conform to the Specifications for "Asphalt Concrete Composite"

Granular material will be the same type used elsewhere on the project or will be as specified in the plans. If granular material type is not specified in the plans, the material will conform to the Specifications for "Base Course". The granular material will be placed the same thickness as the mainline surfacing or as specified in the plans.

Topsoil is not shown in the transverse section drawing on sheet 2 of 6.

All W beam rail will be Type 1 and Class A (12 Ga.) unless specified otherwise in the plans.

W beam rail section lengths may be 12'-6" and/or 25'-0". The combination of section lengths used will be compatible with the total length of rail per site as shown in the plans.

Slots in the rails will be provided as specified in the plans and by the manufacturer. A drilled hole through the rail is not allowed as a replacement for a slot. If the Contractor must create a slot, a cutting torch or plasma cutter is not allowed. The slot edges will be smooth and free of burrs or notches.

All costs for constructing the MGS including labor, equipment, and materials including all posts, blockouts, steel beam rail, and hardware will be incidental to the contract unit price per foot for the respective MGS contract item.

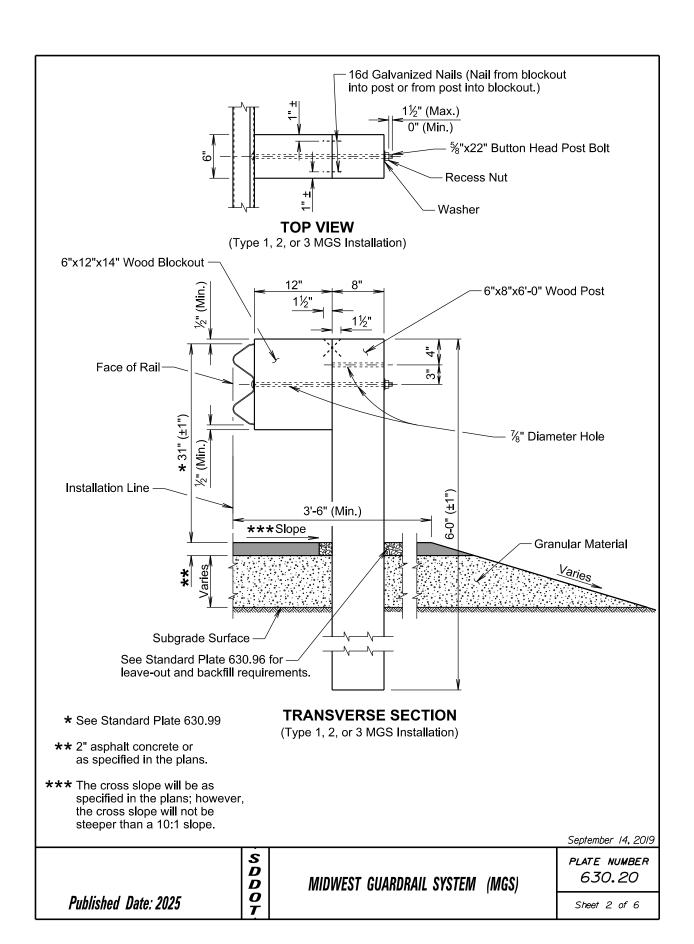
September 14, 2019

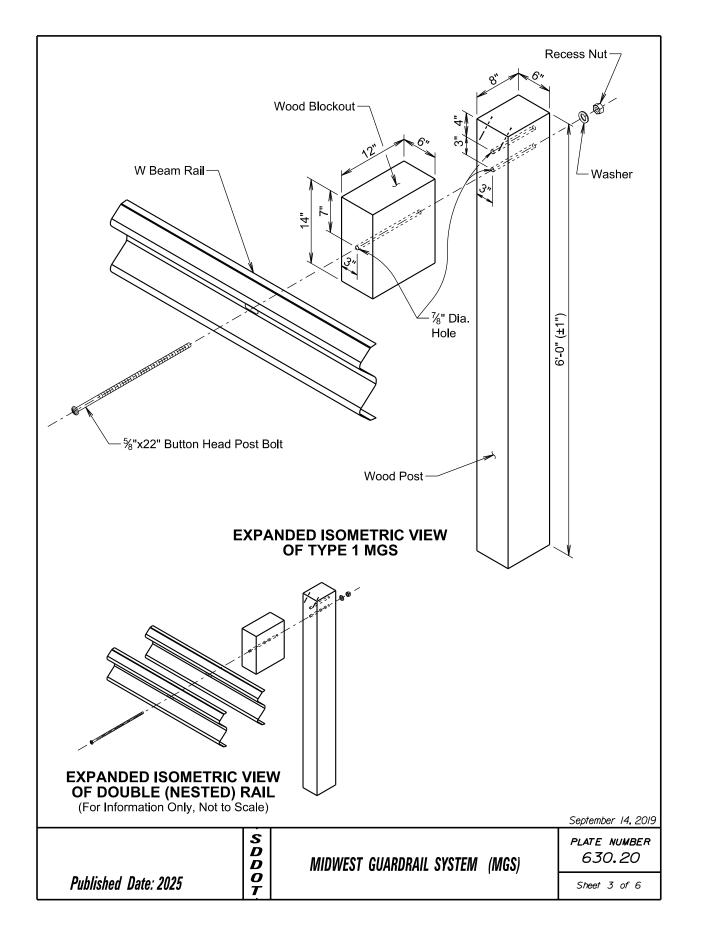
S D PLATE NUMBER *630.20* D O T MIDWEST GUARDRAIL SYSTEM (MGS) Published Date: 2025 Sheet I of 6

STATE OF SOUTH DAKOTA

PROJECT NH 0012(230)171...+ SHEET TOTAL SHEETS B85

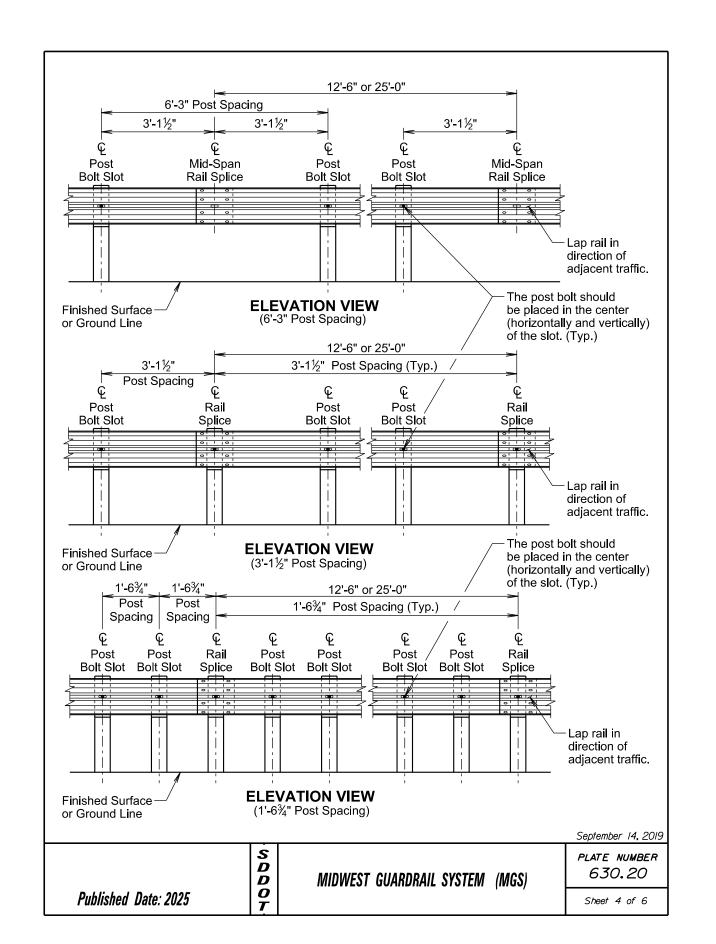
B74

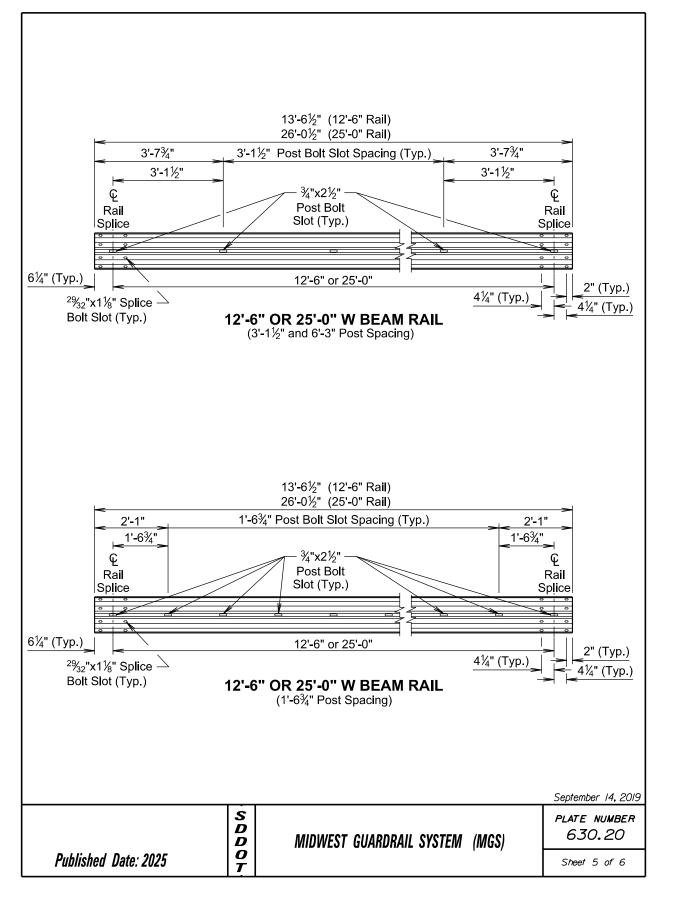




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SHEET TOTAL SHEETS B85

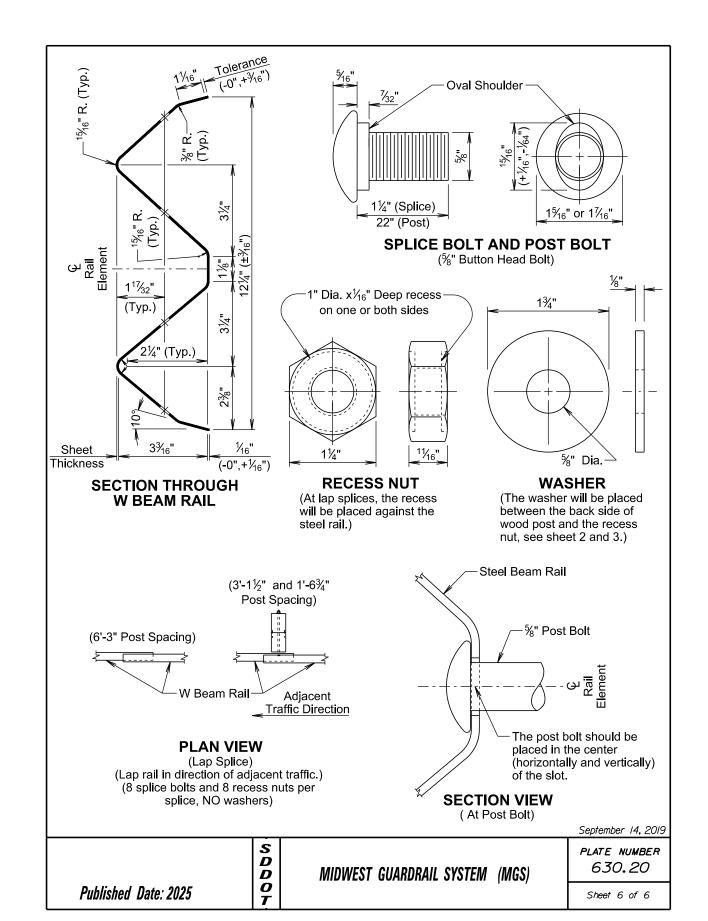


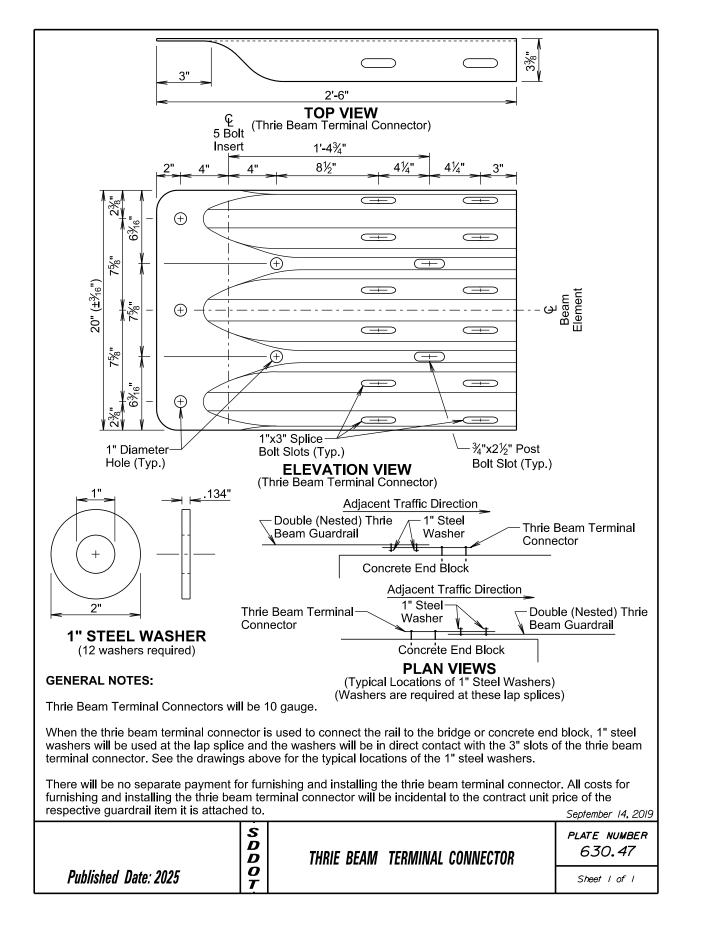


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TOTAL SHEETS SHEET B76 B85

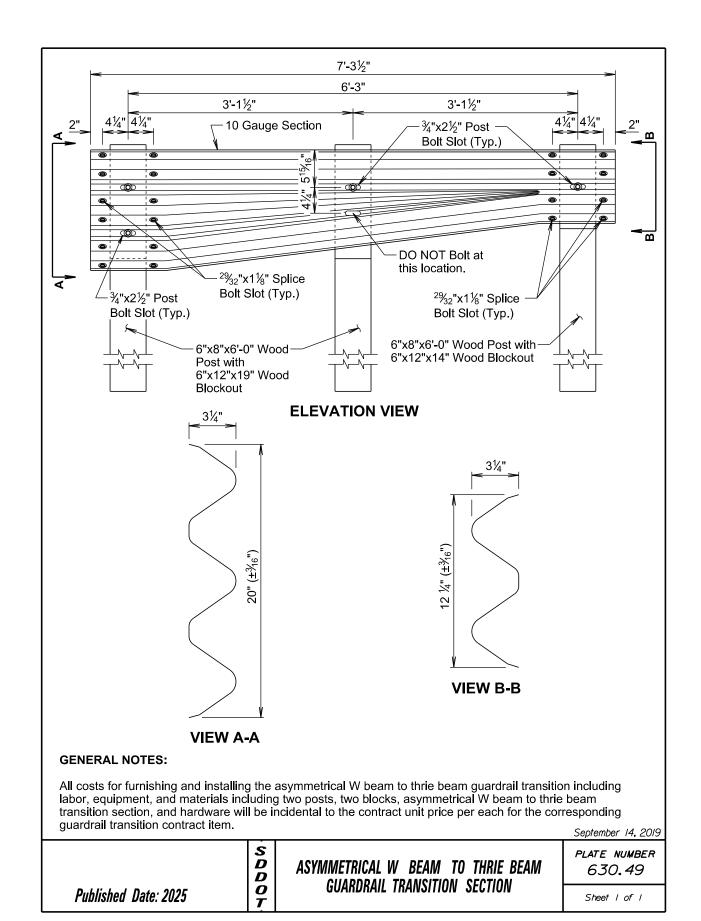
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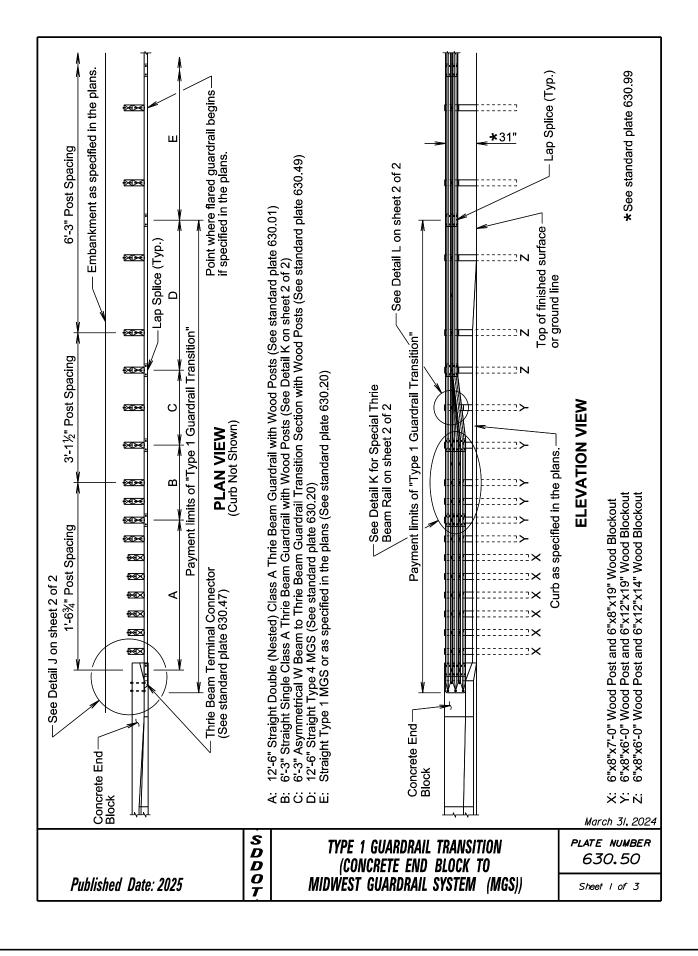




PROJECT

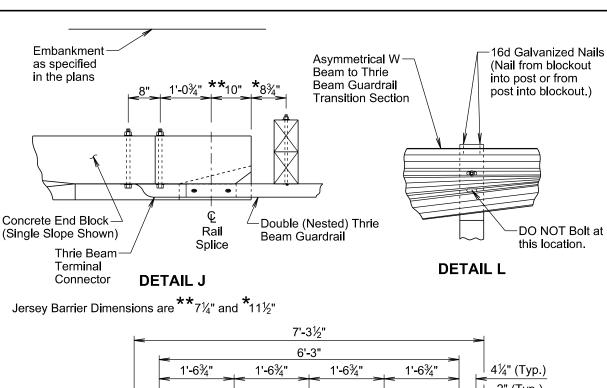
TOTAL SHEETS SHEET NH 0012(230)171...+ B77 B85

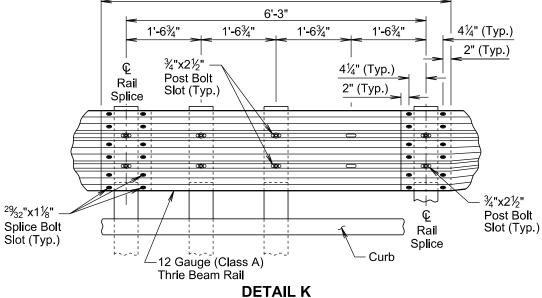




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SHEET TOTAL SHEETS B78 B85





(Special Thrie Beam Rail)

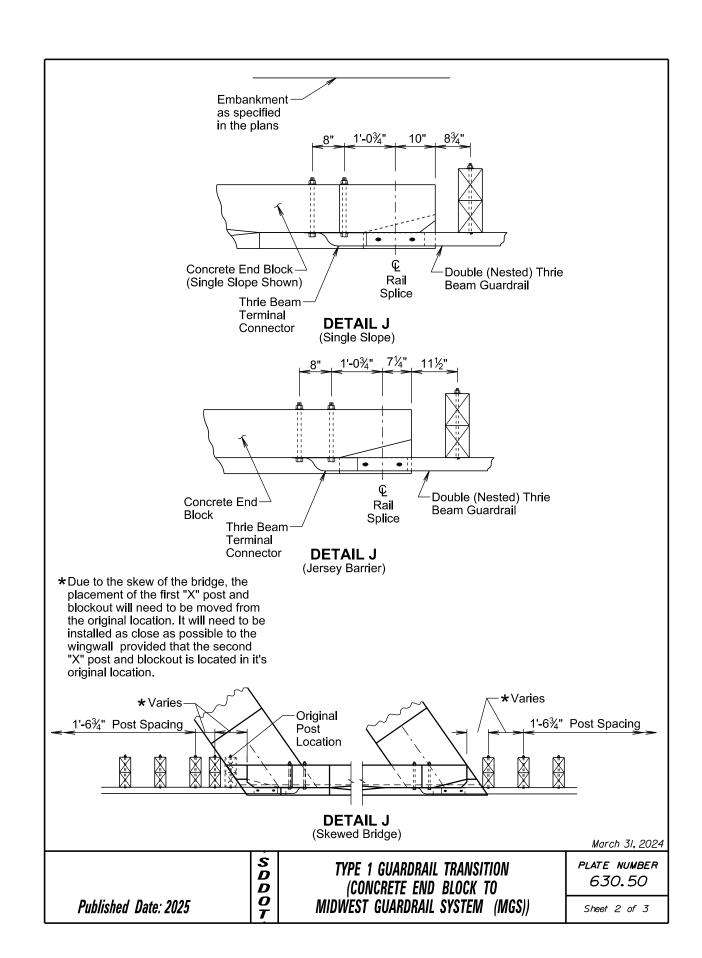
# **GENERAL NOTES:**

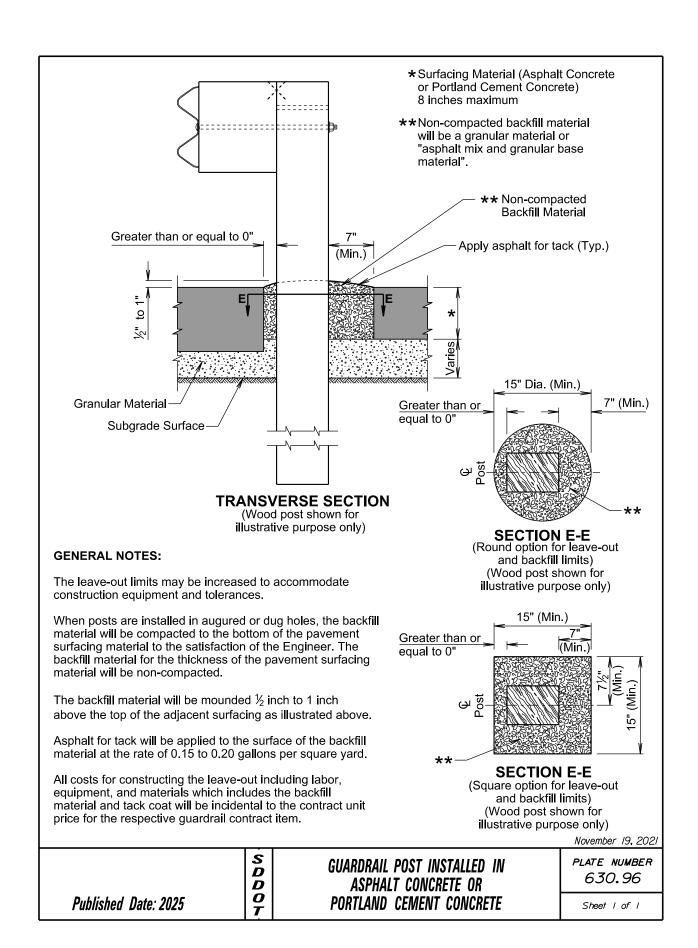
Throughout the type 1 guardrail transition, slots in the rails will be provided as specified in the plans and by the manufacturer. A drilled hole through the rail is not allowed as a replacement for a slot. If the Contractor must create a slot, a cutting torch or plasma cutter is not allowed. The slot edges will be smooth and free of burrs or notches.

All costs for furnishing and installing the type 1 guardrail transition including labor, equipment, and materials which includes all rail sections, posts and blockouts, hardware, and incidentals will be included in the contract unit price per each for "Type 1 Guardrail Transition".

March 31, 2024

S D PLATE NUMBER TYPE 1 GUARDRAIL TRANSITION 630.50 (CONCRETE END BLOCK TO D 0 MIDWEST GUARDRAIL SYSTEM (MGS)) Published Date: 2025 Sheet 3 of 3





### SOUTH FOR BIDDING PURPOSES ONL

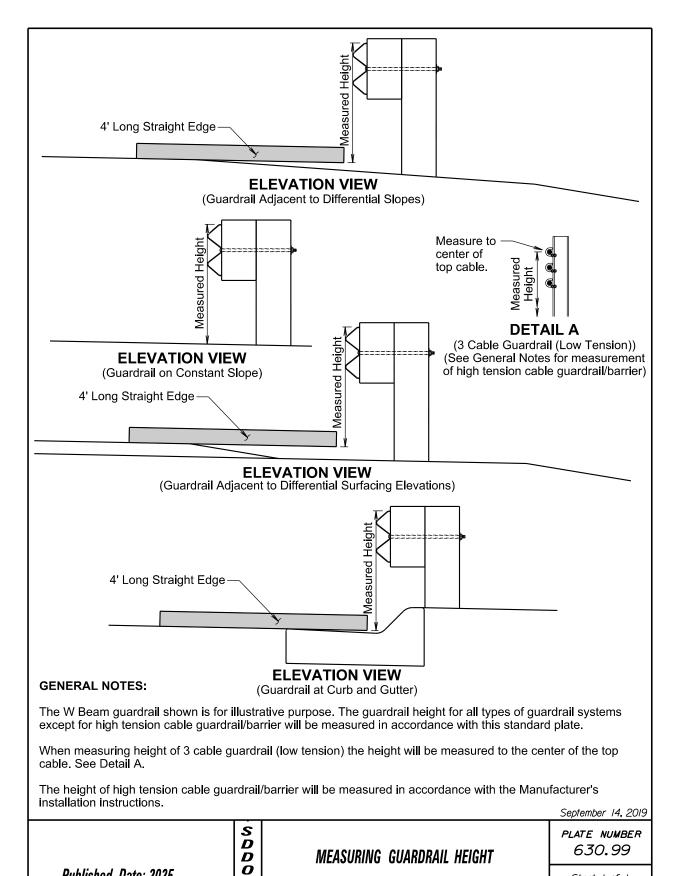
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SHEET NH 0012(230)171...+ B79

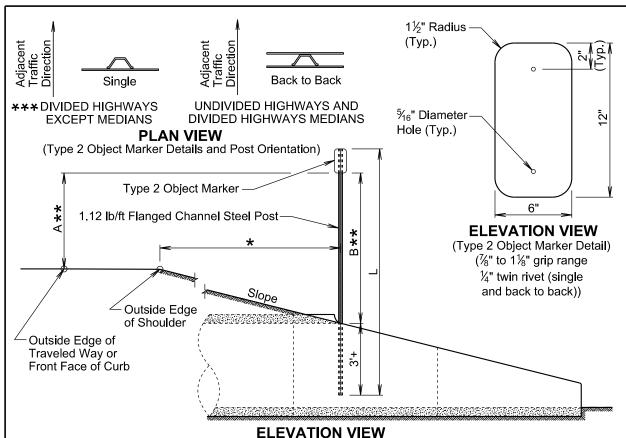
Sheet I of I

TOTAL SHEETS

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Published Date: 2025



(Pipe culvert shown for illustrative purpose.)

	TYPE 2 OBJECT MARKER POST LENGTHS									
OFFSET (*)		1'	2'	3'	4'	5'	6'	7'	8'	Greater Than 8'
		POST LENGTH (L)								
	3.1	8'-6"	8'-9"	9'-3"	9'-6"	9'-9"	10'-3"	10'-6"	10'-9"	8'-0"
SLOPE	4:1	8'-6"	8'-9"	9'-0"	9'-3"	9'-9"	9'-9"	10'-0"	10'-3"	8'-0"
SLC	5.1	8'-3"	8'-6"	8'-9"	9'-0"	9'-3"	9'-3"	9'-6"	9'-9"	8'-0"
	6:1	8'-3"	8'-6"	8'-9"	8'-9"	9'-0"	9'-3"	9'-3"	9'-6"	8'-0"

## **GENERAL NOTES:**

\*\*\* The type 2 object marker may be installed back to back when specified in the plans.

Post Length L was calculated based on a shoulder width of 6 feet at a crosslope of 4 percent and L was rounded up to the nearest 3 inches.

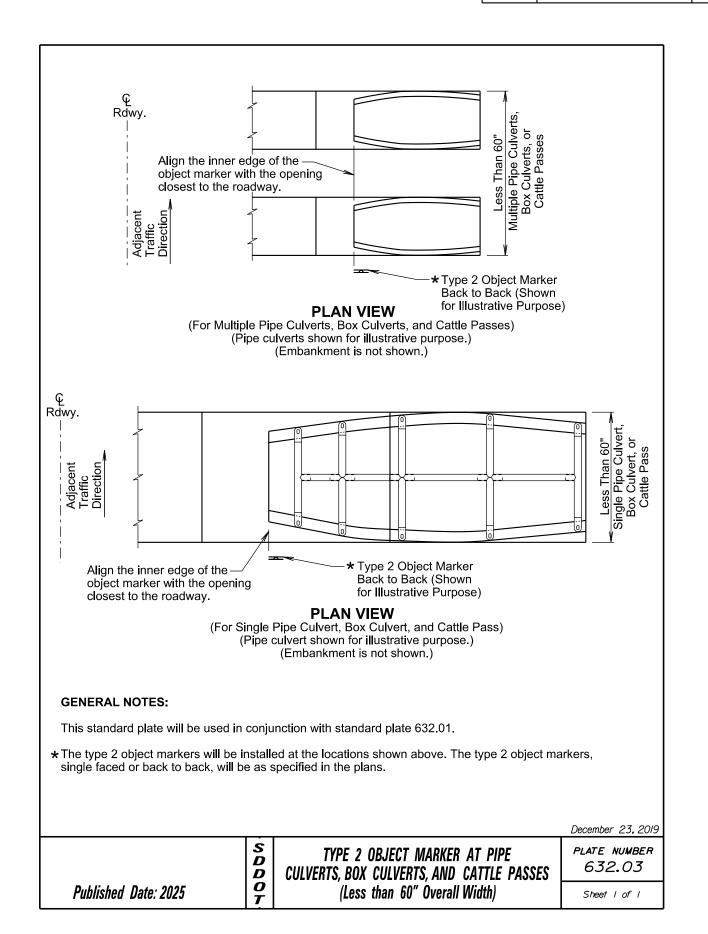
\*\* Dimension A is 4 feet when the Offset \* is 8 feet and less. Dimension B is 4 feet when Offset \* is greater than 8 feet.

The type 2 object marker and the 1.12 lb/ft flanged channel steel post will be in conformance with Specifications Section 982.2 J.

Payment for the type 2 object marker will be in conformance with Specification Section 632.5 B.

December 23, 2019

			Doddingor Lot Lors
	S D D	TYPE 2 OBJECT MARKER	PLATE NUMBER 632.01
Published Date: 2025		(DIRECT DRIVE)	Sheet I of I



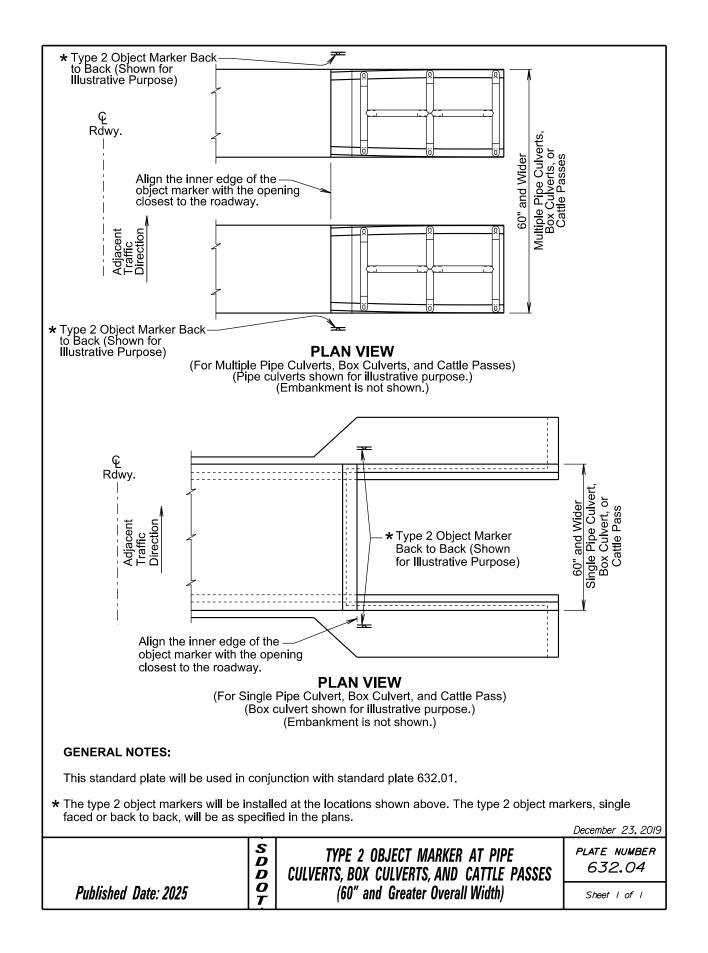


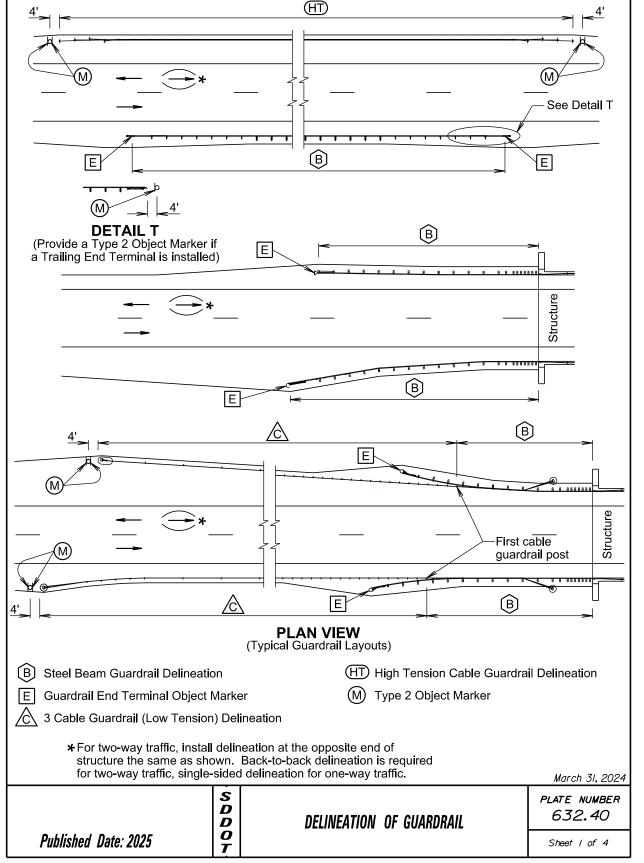
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TOTAL SHEETS

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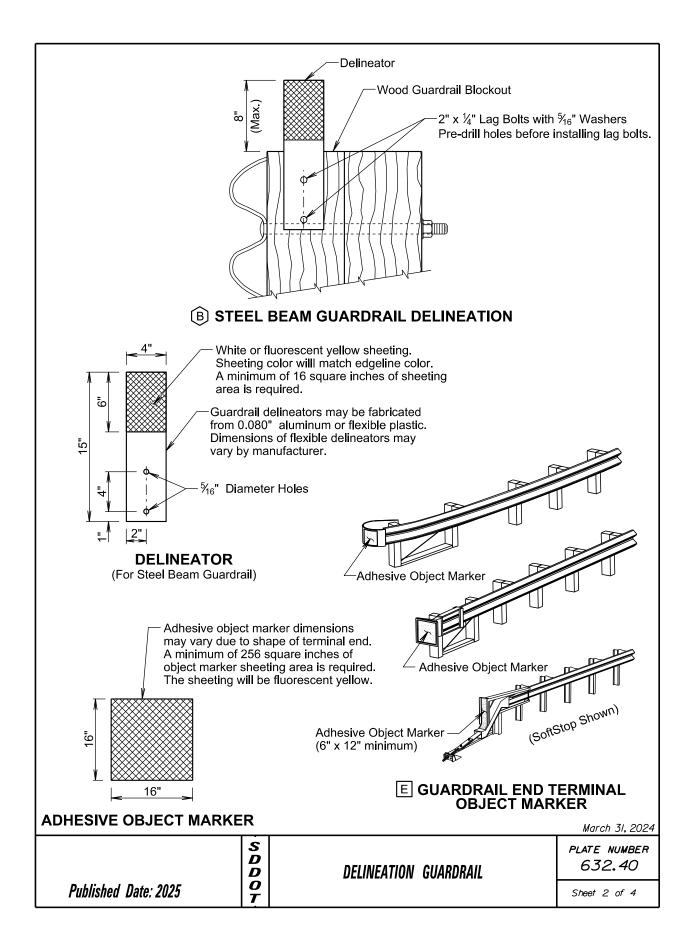


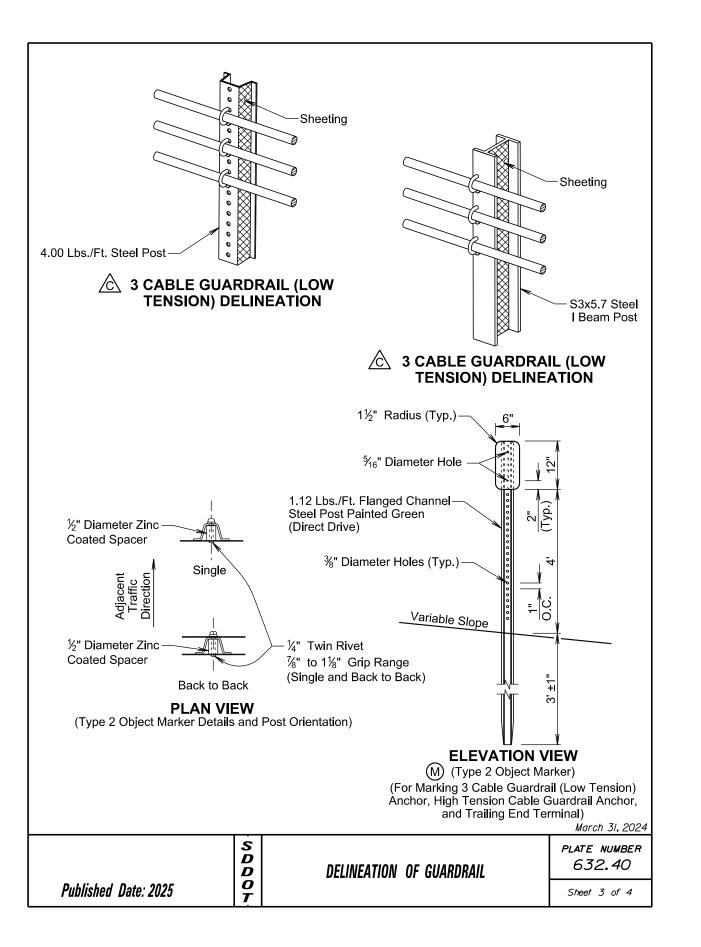
STATE OF SOUTH

PROJECT NH 0012(230)171...+

SHEET TOTAL SHEETS

B82 B85





**GENERAL NOTES:** 

The delineation of high tension cable guardrail will be reflective sheeting placed back to back on every third post cap or cable spacer. Maximum spacing of delineation will not exceed 35 feet. The sheeting will be type XI in conformance with ASTM D4956. The color of the reflective sheeting will be the same as the nearest pavement marking.

The delineators for steel beam guardrail and sheeting on 3 cable guardrail (low tension) posts will be covered with a minimum of 16 square inches of reflective sheeting. The reflective sheeting will be type XI in conformance with ASTM D4956. Along two-way roadways the sheeting will be on both sides of the delineators and guardrail posts and will be white in color. For one-way roadways the sheeting will only be required on the side facing traffic and the color will be the same as the nearest pavement marking, yellow on the left side of the roadway and white on the right side.

When steel beam guardrail is attached to a bridge the first delineator will be attached to the post nearest the bridge.

At bridges with guardrail less than 200 feet in length, a minimum of 4 delineators will be placed in addition to the end terminal yellow object marker. The spacing between the delineators will be approximately one third of the length of the guardrail.

At bridges with guardrail 200 feet and greater in length, including bridges that have steel beam guardrail transitioning to 3 cable guardrail (low tension), the delineators will be placed at a spacing of approximately 50 feet. Delineation will extend throughout the length of the guardrail system.

Steel beam guardrail that is not attached to a bridge and is less than 200 feet in length, a minimum of 4 delineators will be placed in addition to the end terminal yellow object markers. The spacing between the delineators will be approximately one third of the length of the guardrail.

Steel beam guardrail that is not attached to a bridge and is 200 feet and greater in length, including steel beam guardrail transitioning to 3 cable guardrail (low tension), the delineators will be placed at a spacing of approximately 50 feet. Delineation will extend throughout the length of the guardrail system.

All costs for furnishing and installing single or back to back guardrail delineation on 3 cable guardrail and steel beam guardrail will be included in the contract unit price per each for "Guardrail Delineator".

All costs for furnishing and installing the reflective sheeting on the cable spacers or post caps for the high tension cable guardrail will be incidental to the respective high tension cable guardrail contract item.

An adhesive object marker will be placed on the end of the W beam guardrail or MGS end terminal. The adhesive object marker dimensions may vary due to the shape of the terminal end. A minimum of 256 square inches of object marker reflective sheeting area is required on end terminals with sufficient surface area. Other end terminals (SoftStop) will require an adhesive object marker with a minimum size of 6" x 12". The reflective sheeting will be fluorescent yellow type XI sheeting in conformance with ASTM D4956. All costs for furnishing and installing the adhesive object marker will be incidental to various contract items.

A type 2 object marker will be placed adjacent to the 3 cable guardrail (low tension) anchor, high tension cable guardrail anchor, and trailing end terminal at the location noted on sheet 1 of this standard plate. The type 2 object marker (6" x 12") will have fluorescent yellow type XI sheeting in conformance with ASTM D4956. All costs for furnishing and installing the type 2 object marker including the steel post, 6" x 12" reflective panel, and hardware will be included in the contract unit price per each for "Type 2 Object Marker" for single-sided and "Type 2 Object Marker Back to Back" for back to back type 2 object markers.

			March 31, 2024
	S D D	DELINEATION OF GUARDRAIL	PLATE NUMBER 632.40
Published Date: 2025	$\begin{vmatrix} \boldsymbol{o} \\ \boldsymbol{\tau} \end{vmatrix}$		Sheet 4 of 4

FOR BIDDING PURPOSES ONLY DAKOTA

PROJECT

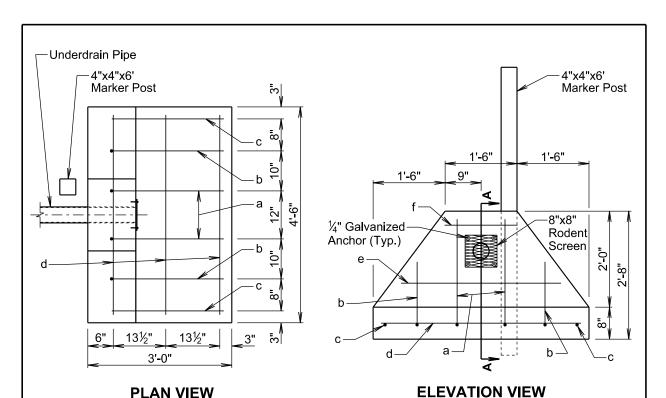
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TOTAL SHEETS

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### **GENERAL NOTES:**

Published Date: 2025

The concrete will be Class M6. The concrete will conform to the requirements of Section 462 of the Specifications except the minimum curing time will be 72 hours. It is estimated that 0.55 cubic yards of concrete is required for each unit.

Four cast-in-place or drilled-in ¼" galvanized anchors will be placed in the headwall. Each galvanized anchor will be placed approximately 1" from the outside corner of the rodent screen. It is preferred that the anchor location be centered at an opening in the rodent screen.

All reinforcing steel will conform to ASTM A615, Grade 60. It is estimated that 25.7 pounds of reinforcing steel is required for each unit.

The underdrain pipe will be placed in the concrete headwall with the pipe end flush with the concrete surface adjacent to the rodent screen.

The 8"x8" rodent screen will be galvanized 13 Ga. steel with a diamond shaped flattened mesh pattern. The size will be  $\frac{1}{2}$ ". The size refers to the measurement across the smallest diamond shaped opening measured from the centers of the wires. The rodent screen will be centered about the hole in the headwall and fastened to the headwall with the appropriate bolts or nuts with washers.

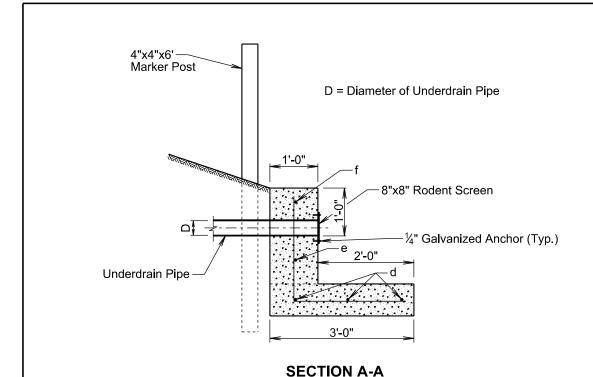
A 4"x4"x6' marker post will be placed at the approximate location as depicted in the above drawings for each concrete headwall. The marker post will project 3'± above the ground line. The marker post will be cedar or treated with a wood preservative and will be painted with two coats of white paint.

All costs for furnishing and installing the concrete headwall including equipment, labor, and materials including concrete, reinforcing steel, rodent screen, anchors, and marker post will be incidental to the contract unit price per each for "Concrete Headwall for Underdrain".

CONCRETE HEADWALL FOR UNDERDRAIN

CONCRETE HEADWALL FOR UNDERDRAIN

Sheet 1 of 2



REINFORCING SCHEDULE							
MK.	No.	. Size Length Type Bending Details					
а	2	4	4'-6"	17A	TI		
b	2	4	3'-9"	17A	TYPE 5TITYPE		
С	2	4	2'-4"	Str.	TYPE TT TYPE		
d	3	4	4'-2"	Str.	<u> </u>		
е	1	4	3'-4"	Str.	a 2'-4" b 2'-4"		
f	1	4	1'-6"	Str.	12 21		
NO	NOTE:						

All dimensions are out to out of bars.

Published Date: 2025

December 23, 2019

D D O CONCRETE HEADWALL FOR UNDERDRAIN PLATE NUMBER *680.01* 

Sheet 2 of 2

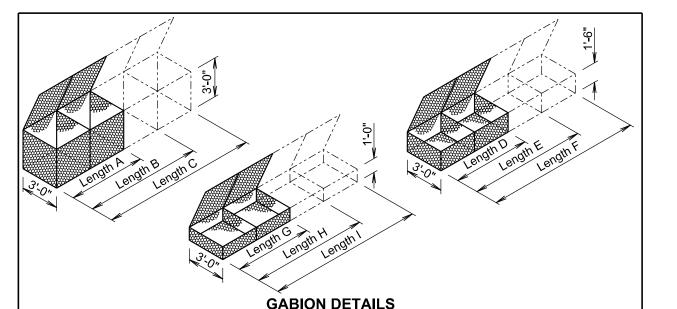
Published Date: 2025

BANK AND CHANNEL PROTECTION GABIONS

PLATE NUMBER 720.01

February 14, 2020

Sheet I of I



### STANDARD SIZES SIZE LENGTH WIDTH HEIGHT NUMBER OF CAPACITY CELLS (Cu. Yd.) 3'-0" 3'-0" 9'-0" 3'-0" 3'-0" 3

2.0 В 3.0 C 12'-0" 3'-0" 3'-0" 4 4.0 D 6'-0" 3'-0" 1'-6" 1.0 3'-0" | 1'-6" 1.5 3'-0" 1'-6" 12'-0" 4 2.0 3'-0" 6'-0" 1'-0" 0.7 9'-0" 3'-0" 1'-0" H 1.0 I 12'-0" 3'-0" 1'-0" 1.3

### **GENERAL NOTES:**

Above dimensions subject to mill tolerances.

Lacing and internal connecting wire will be 0.0866 inch diameter steel wire ASTM A641, Class 3 soft temper measured after galvanizing and for PVC coated gabions will be 0.0866 inch diameter steel wire measured after galvanizing but before PVC coating.

The lacing procedure is as follows:

- 1. Cut a length of lacing wire approximately 1½ times the distance to be laced but not exceeding 5 feet.
- 2. Secure the wire terminal at the corner by looping and twisting.
- 3. Proceed lacing with alternating single and double loops at a spacing not to exceed 6 inches.
- 4. Securely fasten the other lacing wire terminal.

Wire lacing or interlocking type fasteners will be used for gabion assembly and final construction of gabion structures. Interlocking fasteners for galvanized gabions will be high tensile 0.120 inch diameter galvanized steel wire measured after galvanizing. The galvanizing will conform to ASTM A641-92, Class 3 coating. Fasteners will also be in accordance with ASTM A764, Class II, Type III.

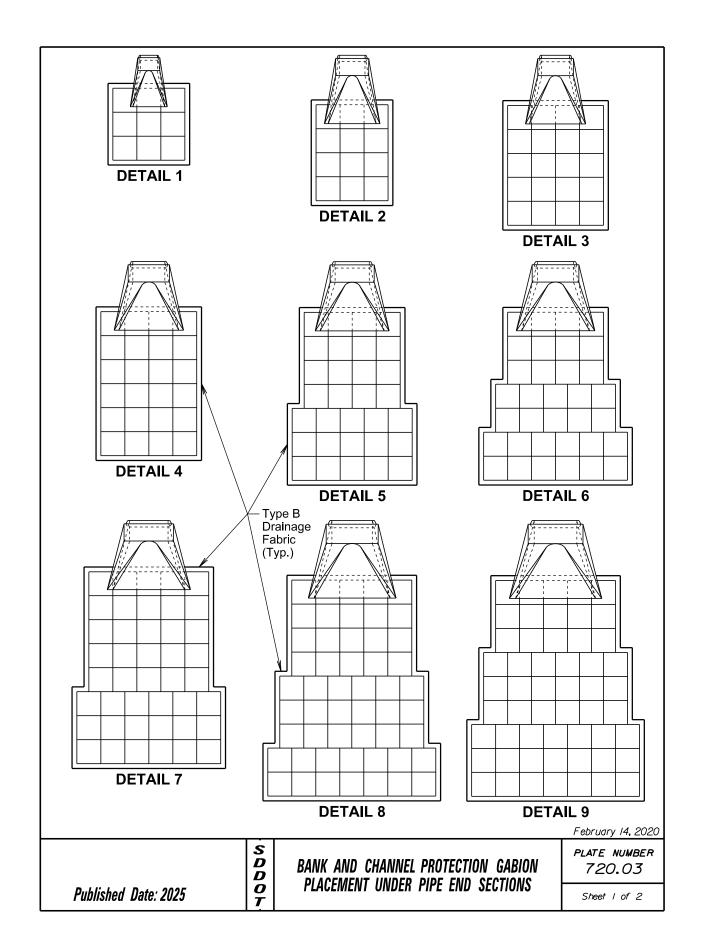
Interlocking fasteners for PVC coated gabions will be high tensile 0.120 inch diameter stainless steel wire conforming to ASTM A313, Type 302, Class 1. The spacing of the interlocking fasteners during all phases of assembly and construction will not exceed 6 inches.

All fasteners will be placed where the mesh weaves around the selvage wire at the vertical and horizontal joints.

S D D 0

STATE OF SOUTH DAKOTA





	* ESTIMATED QUANTITIES								
		Pipe	Gabion	Type B					
	Detail	Diameter		Drainage					
	Detail			Fabric					
		(Inches)	(Cu. Yd.)	(Sq. Yd.)					
	1	12, 18, and 24	4.5	15					
RCP Arch, nd CMP Arch	2	30 and 36	6.0	19					
P C	3	42	10.0	29					
Z ₹	4	48 and 54	12.0	34					
	5	60	15.5	43					
_ =	6	66	17.0	47					
P,C	7	72	21.5	57					
RCP. CMP, a	8	78	26.0	68					
	9	84	27.0	70					

### **GENERAL NOTES:**

Published Date: 2025

Gabions at outlets of CMP and RCP will be placed under the end section a distance of 2 feet from the outlet end. For CMP end section installations, the upper fabric of the gabions will be modified to accommodate the metal end section as approved by the Engineer.

★ Gabion and type B drainage fabric quantities on this standard plate are based on standard gabion sizes D, E, and F as depicted on standard plate 720.01.

Type B drainage fabric will be placed under the gabions and around the exterior sides (perimeter) of the gabions as approved by the Engineer. The type B drainage fabric will be in conformance with Section 831 of the Specifications. Measurement and payment of the type B drainage fabric will be in conformance with Section 720 of the Specifications.

February 14, 2020

BANK AND CHANNEL PROTECTION GABION PLACEMENT UNDER PIPE END SECTIONS

PLATE NUMBER 720.03

Sheet 2 of 2