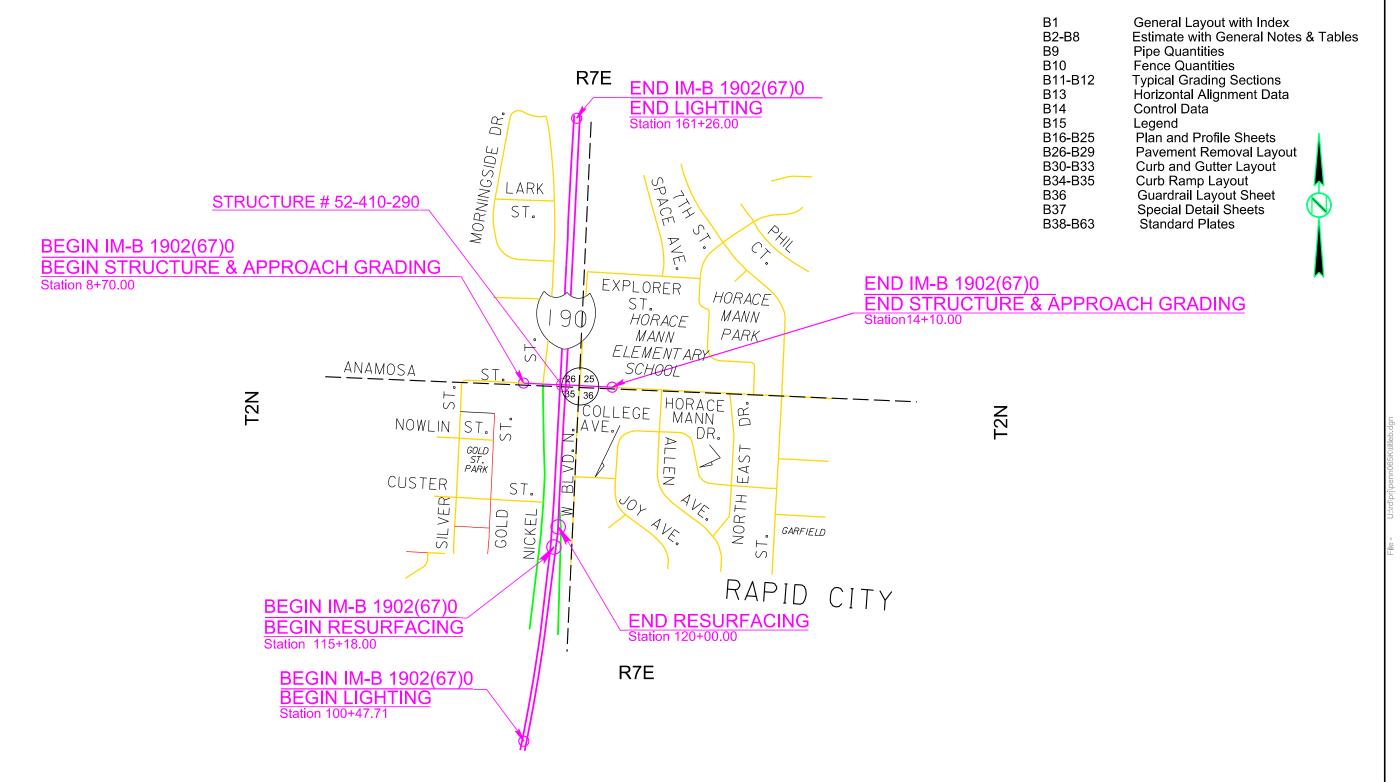
SECTION B: GRADING PLANS

Τ	STATE OF	PROJECT	SHEET	TOTAL	
	SOUTH DAKOTA	IM-B 1902(67)0	B1	B63	

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otted From -

SECTION B ESTIMATE OF QUANTITIES

BID ITEM NUMBER	ITEM	QUANTITY	UNIT
009E0010	Mobilization	Lump Sum	LS
009E3220	Reestablish Right-of-Way and Property Corner	7	Each
009E3230	Grade Staking	0.776	Mile
009E3250	Miscellaneous Staking	0.309	Mile
009E3280	Slope Staking	0.070	Mile
009E3290	Structure Staking	2	Each
009E3301	Engineer Directed Surveying/Staking	40	Hour
009E4200	Construction Schedule, Category II	Lump Sum	LS
110E0300	Remove Concrete Curb and/or Gutter	2,183	Ft
110E0600	Remove Fence	52	Ft
110E1010	Remove Asphalt Concrete Pavement	2,980.3	SqYd
110E1100	Remove Concrete Pavement	444.2	SqYd
110E1120	Remove Concrete Median Pavement	245.2	SqYd
110E1130	Remove Concrete Driveway Pavement	127.4	SqYd
110E1140	Remove Concrete Sidewalk	309.3	SqYd
110E4290	Salvage Beam Guardrail	421.0	- Ft
110E7802	Remove Fence for Reset	1,074	Ft
120E0010	Unclassified Excavation	4.506	CuYd
120E0600	Contractor Furnished Borrow Excavation	75	CuYd
120E2000	Undercutting	2,174	CuYd
380E3520	6" PCC Approach Pavement	88.2	SqYd
380E3540	8" PCC Approach Pavement	40.8	SqYd
380E4050	8" PCC Fillet Section	156.5	SqYd
450E0122	18" RCP Class 2, Furnish	50	Ft
450E0130	18" RCP, Install	50	Ft
450E0142	24" RCP Class 2, Furnish	54	Ft
450E0150	24" RCP, Install	54	Ft
450E0162	30" RCP Class 2, Furnish	48	Ft
450E0170	30" RCP, Install	48	Ft
450E0182	36" RCP Class 2, Furnish	972	Ft
450E0190	36" RCP, Install	972	Ft
450E0424	30" RCP Bend, Furnish	1	Each
450E0425	30" RCP Bend, Install	1	Each
451E6080	Adjust Water Valve Box	4	Each
462E0100	Class M6 Concrete	28.3	CuYd
480E0100	Reinforcing Steel	5,060	Lb
600E0200	Type II Field Laboratory	1	Each
620E4100	Reset Fence	1,074	Ft
621E0050	5' Chain Link Fence with Top Rail	52	Ft
630E0520	Type 2 MGS	300.0	Ft
630E2018	MGS MASH Tangent End Terminal	2	Each
630E2065	MGS Trailing End Terminal	2	Each
650E0060	Type B66 Concrete Curb and Gutter	1,443	Ft
650E1060	Type F66 Concrete Curb and Gutter	414	Ft
650E1110	Type F611 Concrete Curb and Gutter	6	Ft
650E4360	Type D46 Concrete Curb and Gutter	12	Ft
650E4380	Type D48 Concrete Curb and Gutter	30	Ft
650E4410	Type D411 Concrete Curb and Gutter	168	Ft
650E4660	Type P6 Concrete Gutter	44	Ft
650E4680	Type P8 Concrete Gutter	24	Ft
650E4710	Type P11 Concrete Gutter	40	Ft
	4" Concrete Sidewalk	3,998	SqFt
651E0040			

SECTION B ESTIMATE OF QUANTITIES (CONTINUED)

BID ITEM NUMBER	ITEM	QUANTITY	UNIT
670E0200	Type A Frame and Grate	1	Each
670E5340	4' x 11' Precast Concrete Type S Drop Inlet Lid	4	Each
670E5400	Precast Drop Inlet Collar	1	Each
671E5502	2" Adjusting Ring for Manhole	2	Each
671E5506	6" Adjusting Ring for Manhole	4	Each
671E6010	Type A10 Manhole Frame and Lid	2	Each
671E8000	Reconstruct Manhole	1	Each
680E0440	4" Slotted Corrugated Polyethylene Drainage Tubing	158	Ft
680E2500	Porous Backfill	66	Ton

GRADING OPERATIONS

Water for Embankment is estimated at the rate of 15 gallons of water per cubic yard of Embankment minus Waste. The estimated quantity of Water for Embankment is 47 MGal. No separate payment will be made for the Water for Embankment and all costs associated will be incidental to the contract unit price per cubic yard of "Unclassified Excavation".

The estimated cubic yards of excavation and/or embankment required to construct outlet ditches, ditch blocks, and approaches are included in the earthwork balance notes on the profile sheets.

The estimated excavation required for placing the Granular Bridge End Backfill and/or Bridge End Embankment is listed in the Table of Unclassified Excavation. See Section E for Bridge Berm Configuration and Bridge End Backfill excavation limits.

Special ditch grades and other sections of the roadway different than the typical sections will be constructed to the limits shown on the cross sections. If significant changes to the cross sections are necessary during construction, the Engineer will contact the Designer for the proposed change.

Generally, all shallow inlet and outlet ditches as noted on the plan sheets will be cut with a 10-foot wide bottom with 5:1 backslopes. However, the Engineer may direct the Contractor to adjust the ditch width for proper alignment with the drainage structure.

Temporary fence and/or permanent fence will be placed ahead of the grading operation unless otherwise directed by the Engineer.

GENERAL GEOLOGY

The project alignment is underlain by Mowry Shale. The South Dakota Geologic Survey describes the Mowry Shale as outlined below:

The Mowry Shale consists of black to gray, siliceous, fissile shale and siltstone containing bentonite layers and spars sandstone dikes.

Most of the material encountered should be able to be excavated using conventional methods associated with normal Unclassified Excavation.

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TYPE II FIELD LABORATORY

The lab will be equipped with an internet connection such as DSL, cable modem, or other approved service. The internet connection will be provided with a multi-port wireless router. The internet connection will be a minimum speed of 5 Mbps unless limited by job location and approved by the DOT. Prior to installing the wireless router, the Contractor will submit the wireless router's technical data to the Area Office to check for compatibility with the state's computer equipment. The internet connection is intended for state personnel usage only. The Contractor's personnel are prohibited from using the internet connection unless pre-approved by the Project Engineer. These items will be incidental to the contract unit price per each for "Type II Field Laboratory".

UTILITIES

The Contractor will be aware that the existing utilities shown in the plans were surveyed prior to the design of this project and might have been relocated or replaced by a new utility facility prior to construction of this project, might be relocated or replaced by a new utility facility during the construction of this project, or might not require adjustment and may remain in its current location. The Contractor will contact each utility owner and confirm the status of all existing and new utility facilities. The utility contact information is provided elsewhere in the plans or bidding documents.

SALVAGE BEAM GUARDRAIL

Steel beam rail, end terminals, posts, blocks, and hardware items will become the property of the State and will be removed, hauled, and neatly stacked at the DOT South Maintenance Yard at 5801 S. Highway 79 in Rapid City as approved by the Engineer.

Payment for removing, hauling, and stacking the guardrail items will be incidental to the contract unit price per foot for "Salvage Beam Guardrail".

UNDERDRAIN

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Underdrains will be encountered within the excavations for Piers No. 2 & 4 at Station 11+05± from 41.5' Lt. to 36.5' Rt. and at Station 11+95± from 41.5' Lt to 36.5' Rt. The underdrains consist of 4-inch Slotted Corrugated Polyethylene Tubing placed in a 2 foot wide trench of variable depth backfilled with Porous Backfill, Base course, and fill material. Underdrains encountered will be cut and temporally capped or diverted during construction until the removed segments of the drains are replaced. Downstream (right/south) cut end of the drains will be capped to prevent material or debris from infiltrating the underdrain system. Upstream (left/north) cut end of the drains will require diversion to prevent water from entering the excavation or I190 Subgrade. The diversion may include pipes, pumps, sumps, or other methods approved by the Engineer. The contractor must submit a proposed diversion plan and receive approval from the Engineer prior to beginning excavations where underdrains may be encountered. All labor, tools, equipment, and incidentals necessary to maintain or divert existing underdrain system during construction and install new underdrains will be incidental to the contract unit price per foot of "4" Slotted Corrugated Polyethylene Tubing".

> Replace the removed drain segments after substructure work is complete and the subgrade has been rebuilt to grade. The temporary excavations for substructure work will be backfilled with soil and compacted according to Specified Density prior to installation of the replacement underdrain. Replacement underdrains will be placed in a 2-foot-wide trench backfilled with porous backfill positively connected to the existing underdrain.

> Care will be taken to ensure that existing and replacement underdrains are not damaged during construction. Sufficient cover materials is to be placed over the pipes before equipment is allowed over the underdrain system. Damaged pipe will be replaced by the Contractor at no additional cost to the Department.

> The underdrain locations are given based on the best available information to the Geotechnical Engineering Activity. Actual field conditions may require that adjustments be made by the Engineer during construction to provide for sufficient drainage. The Geotechnical Engineering Activity will be available for onsite assistance if necessary.

> Underdrain trenches will be graded to maintain a minimum .01 ft/ft or 1% drop the entire length of the replacement. The Contractor will ensure all segments of the drainage tubing are positively connected and remain soil light during installation of the underdrain.

The estimated quantities for the underdrain system are as follows:

4" Slotted Corrugated Polyethylene Tubing	158	Ft
Porous Backfill	66	Ton

UNDERCUTTING

In all cut sections the earthen subgrade will be undercut 2 feet below the earthen subgrade surface. The undercut material or other suitable material, as directed by the Engineer, will then be replaced and compacted to the density specified for the section being constructed.

Shallow embankment sections, fills less than 2 feet in height measured at the finished subgrade shoulders, will be undercut to ensure a minimum 2-foot height of earth embankment for the entire width of roadbed. The undercut material or other suitable material, as directed by the Engineer, will then be replaced and recompacted to the density specified for the section being constructed.

The plan shown quantity will be the basis of payment. However, if there are additional areas of undercut other than what is shown in the plans, the Engineer will direct removal of these areas and the additional areas will be measured according to the Engineer.

TABLE OF UNDERCUTTING

				Quantity
Alignment	Station	to	Station	(CuYd)
Anamosa St	8+70		14+10	1,287
Nickel St	1+60		2+24.50	186
W Blvd N	10+00		12+75	701
			Total:	2,174

TABLE OF EXCAVATION QUANTITIES BY BALANCES

			Excavation	* Undercut	* Contractor Furnished Borrow Exc.	Total Excavation	** Waste
Alignment	Station to	Station	(CuYd)	(CuYd)	(CuYd)	(CuYd)	(CuYd)
Anamosa St	8+70	14+10	1,364	1,287	75	2,726	776
Nickel St	1+60	2+24	80	186		266	12
W Blvd N	10+00	12+75	379	701		1,080	106
		Totals:	1,823	2,174	75	4,072	894

- The quantities for these items are in the Estimate of Quantities under their respective contract items.
- ** The quantities for these items are for information only.

TABLE OF UNCLASSIFIED EXCAVATION

Excavation Undercut Topsoil Added Temporary Pedestrian Bridge Embankment Excavation Exc. For Bridge End Backfill and/or	(CuYd) 1,823 2,174 370 75
Embankment	
Total	4,506

PROCEDURES FOR DETERMINING UNCLASSIFIED EXCAVATION QUANTITY

When plan quantities are used for payment, the Unclassified Excavation quantity will be used for final payment and the plans quantity of Topsoil and salvaged surfacing items listed in the Table of Unclassified Excavation will not be adjusted according to field measurements.

The following paragraphs are general earthwork information and information in regard to computing the Unclassified Excavation quantity when final cross sections are taken in the field:

The Excavation quantities from individual balances and the Table of Unclassified Excavation have been reduced by the volume of in place surfacing that will be removed and/or salvaged.

The volume of in place Concrete Surfacing and Asphalt Surfacing removed will NOT be paid for as Unclassified Excavation.

The Excavation quantities from individual balances and the table above have been reduced by the volume of in place concrete pavement and asphalt pavement that will be removed.

When finaling a project, the estimated quantity of 344 cubic yards of Concrete Pavement and Asphalt Pavement removed from the cut sections will be subtracted from the Unclassified Excavation quantity for final payment. The quantity of Concrete Pavement and Asphalt Pavement from cut sections subtracted from the Unclassified Excavation quantity will be plans quantity and will not be adjusted according to field measurements.

WASTE EXCAVATION

The quantity of waste in the Table Of Excavation Quantities By Balances is excess excavation material to be disposed of by the Contractor at a site approved by the Engineer.

CONTRACTOR FURNISHED BORROW EXCAVATION

The Contractor will provide a suitable site for Contractor furnished borrow excavation material. The Contractor is responsible for obtaining all required permits and clearances for the borrow site. The borrow material will be approved by the Engineer. The plans quantity for "Contractor Furnished Borrow Excavation" as shown in the Estimate of Quantities will be the basis of payment for this item.

Restoration of the Contractor furnished borrow excavation site will be the responsibility of the Contractor.

REMOVAL OF EXISTING CONCRETE PAVEMENT

Existing asphalt concrete and/or existing asphalt concrete patch work that was placed above the existing concrete pavement is included in the quantity for "Remove Concrete Pavement". The Contractor will dispose of the concrete pavement and asphalt concrete at a site approved by the Engineer.

The existing P.C.C. Pavement is 6-inch thick with no visible reinforcement. P.C.C. fillets may contain some steel reinforcement. This information is from original construction plans and actual pavement thickness may vary.

The existing contraction joints are spaced at approximately 10 feet.

TABLE OF CONCRETE PAVEMENT REMOVAL

Station	to	Station	Description	Quantity (SqYd)
*138+83		139+63	I-190 Shoulder Removal and Curb and Gutter Removal Left	90.4
*138+84		139+63	I-190 Shoulder Removal and Curb and Gutter Removal Right	94.4
**10+14		10+32	Fillet	13.6
**10+15		10+31	Fillet	14.3
**12+49		12+64	Fillet	12.8
**12+48		12+64	Fillet	13.4
**13+00		13+15	Fillet	12.1
**13+00		13+17	Fillet	13.9
**13+00		13+83	24' Wide Concrete Pavement Section Which Also Includes Fillets and Curb and Gutter Removal	183.3
			Total	444.2

Total: 444.2

TABLE OF CONCRETE DRIVEWAY PAVEMENT REMOVAL

					Quantity
Alignment	Station	to	Station	L/R	(SqYd)
Anamosa St	13+52		13+83	L	39.3
Anamosa St	13+75		14+13	R	57.0
W Blvd N	9+93		10+16 (WBlvdN)	R	31.1
				Total:	127.4

TABLE OF ASPHALT CONCRETE PAVEMENT REMOVAL

					Quantity
Alignment	Station	to	Station	L/R	(SqYd)
Anamosa St	8+70		10+57	L&R	739.0
Anamosa St	12+33		14+12	L&R	639.3
Nickel St	1+60		2+28	L&R	243.1
Nickel St	2+58		2+95	L&R	131.9
I-190	138+82		141+52	L	100.0
I-190	136+83		139+65	R	90.2
W Blvd N	3+17		10+00	L	83.2
W Blvd N	10+00		11+65	L&R	655.3
W Blvd N	12+00		12+75	L&R	298.3
				_	

Total: 2,980.3

TABLE OF CONCRETE CURB AND/OR GUTTER REMOVAL

					Quantity
Alignment	Station	to	Station	L/R	(Ft)
I-190	100+41		102+48	L	207.0
I-190	100+41		102+48	R	207.0
I-190	137+93		138+10	R	16.5
I-190	140+34		140+50	L	16.0
I-190	141+23		141+44	L	21.3
Anamosa St	8+70		9+81	R	119.1
Anamosa St	8+70		9+76	L	115.1
Anamosa St	10+32		10+67	R	35.5
Anamosa St	10+32		10+67	L	35.2
Anamosa St	12+33		12+48	R	14.5
Anamosa St	12+33		12+49	L	16.1
Anamosa St	13+17		14+12	R	95.1
Anamosa St	13+17		14+10	L	92.9
Anamosa St	1+60		2+07	R	46.4
Anamosa St	1+60		2+12	L	53.0
W Blvd N	9+93		11+48	R	155.0
W Blvd N	3+17		11+47	L	829.8
W Blvd N	12+26		12+75	R	48.7
W Blvd N	12+16		12+75	L	58.9
				Total:	2,183

TABLE OF SIDEWALK REMOVAL

					Quantity
Alignment	Station	to	Station	L/R	(SqYd)
Nickel St	1+60		2+25	L	35.6
Anamosa St	10+18		10+67	R	28.7
Anamosa St	10+17		10+67	L	30.2
Anamosa St	12+33		12+62	R	18.2
Anamosa St	12+33		12+60	L	14.3
Anamosa St	13+03		13+80	R	53.0
Anamosa St	13+03		13+80	L	48.3
W Blvd N	10+50		11+43	R	59.2
W Blvd N	12+26		12+75	R	21.8

Total: 309.3

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TABLE OF CONCRETE MEDIAN PAVEMENT REMOVAL

					Quantity
Alignment	Station	to	Station	L/R	(SqYd)
I-190	100+41		102+48	L&R	245.2
				Total:	245.2

DROP INLETS

Where drop inlets are constructed within areas of curb and gutter, the Contractor will construct weep holes of at least 3 inches in diameter in the drop inlet walls. The weep holes will be constructed at the same elevation as the adjacent top of the earthen subgrade and will be maintained clean and open at all times until the permanent surfacing is placed. The drop inlets will be covered throughout construction operations as necessary with an Engineer approved cover to provide safe travel for motorists and to prevent materials from entering the storm sewer system. After the permanent surfacing has been placed, the Contractor will seal the weep holes with grout and remove all debris from the drop inlet. All costs involved with the coverings, weep holes, and removing debris from the drop inlets will be incidental to the contract unit prices for the components of the drop inlets.

The plan shown quantities of the drop inlet components such as Class M6 Concrete, Reinforcing Steel, Type A Frame and Grate Assembly, Precast Drop Inlet Collar, and Precast Concrete Type S Drop Inlet Lid will be the basis of payment for these items.

If additions or reductions to the number of drop inlets are ordered by the Engineer, payment for the components required to construct the drop inlets will be made at the contract unit prices for the components of the drop inlets.

TABLE OF DROP INLETS AND QUANTITIES

						Precast	
				Class		Drop	Frame
	L	Drop	Drop	M6	Reinf.	Inlet	and
	/	Inlet	Inlet	Concrete	Steel	Collar	Grate/Lid
**Station	R	Size	Type	(CuYd)	(Lb)	(Each)	Type
*12+60	L	4'x11'	S	5.55	887		S
*13+04	L	4'x11'	S	5.00	818		S
*13+46	L	4'x11'	S	3.63	657		S
*13+46	R	4'x11'	S	3.08	568		S
*14+07	L	3'x4'	В	1.72	264	1	A
			Totals:	18.98	3.194	1	

Total Type A Frame and Grate Assembly 1
Total 4'x11' Precast Concrete Type S Drop Inlet Lid 4

- * Drop inlet requires watertight joints in accordance with the STORM SEWER notes.
- ** All Stationing is referenced off of Anamosa Street Alignment

^{*} Stations are referenced off of I-190 alignment

^{**} Stations are referenced off of Anamosa Street Alignment

TABLE OF JUNCTION BOXES AND QUANTITIES

_**Station	L/R	Size L'xW'xH'	Frame and Lid (Type)	Class M6 Concrete (CuYd)	Reinforcing Steel (Lb)	Adjusting Rings
*6+17	L	5'x5'x6.0'	A10	4.88	958	2" & 2-6"
*9+17	L	5'x5'x5.0'	A10	4.39	908	2" & 2-6"
			Totals:	9.27	1,866	
Total Type A10 Manhole Frame and Lid 2						
Total 2" A	djusting	Ring for Ma		2		
Total 6" A	djusting	g Ring for Ma		4		

- * Junction boxes require watertight joints in accordance with the STORM SEWER notes.
- ** All Stations are referenced off of the W Blvd N Alignment

CONCRETE PIPE CONNECTIONS

Pipe connections to existing pipes, manholes, junction boxes, and drop inlets will be done by breaking a hole into the existing structure and inserting the pipe. A concrete collar will then be poured around the pipe in the area of the connection.

When it is not possible to use a normal pipe joint (male-female ends), connections to existing pipe will be made by placing a 2' wide by 6" thick M6 concrete collar around the outside of the connection. The concrete collar will be reinforced with 6x6 W2.9 x W2.9 wire mesh.

All costs for constructing the concrete collars including materials and labor will be incidental to the contract unit price per foot for the corresponding pipe contract item.

STORM SEWER

Reinforced concrete pipe may be bell and spigot. The pipe sections will be adjoined such that the ends are fully entered and the inner surfaces are reasonably flush and even.

Lift holes in the reinforced concrete pipe will be plugged with grout.

Watertight joints are required for reinforced concrete pipe, drop inlets, manholes, and junction boxes where storm sewers run parallel to and within 10 feet horizontally from existing or proposed water mains.

Watertight joints are required where reinforced concrete pipes, drop inlets, manholes, or junction boxes cross water mains and are separated a distance of 18 inches or less, above or below, the water main.

If watertight joints are required then the watertight joints will extend for a distance of 10 feet beyond the water main. This measurement will be from the sealed concrete joint to the outer most surface of the water main.

Watertight joint seals will conform to the following requirements:

1. Reinforced Concrete Pipe (Circular): Gasketed pipe will conform to the requirements of ASTM C443 and the gasket will be in conformance

with Section 990 of the Specifications. Non-gasketed concrete pipe will be sealed with a mastic joint seal conforming to the requirements of ASTM C990 and encased with a minimum 2-foot wide by 6-inch thick M6 concrete collar reinforced with 6x6 W2.9 x W2.9 wire mesh.

- 2. Reinforced Concrete Pipe (Arch): Gasketed pipe will conform to the requirements of ASTM C443 and the gasket will be in conformance with Section 990 of the Specifications. Non-gasketed concrete pipe joints will be sealed with a hydrophilic flexible water stop seal and wrapped with a 1-foot wide strip of fabric above the cradle. The fabric will conform to the requirements of Section 831 of the Specifications for Type A Drainage Fabric. The hydrophilic flexible water stop will be from the list below.
- 3. <u>Drop Inlets, Manholes, and Junction Boxes</u>: Joints will be sealed with one of the following methods:
 - A. A flexible strip seal placed in the joints conforming to the requirements of ASTM C990 and the perimeter encased with a minimum 2-foot wide by 6-inch thick M6 concrete collar reinforced with 6x6 W2.9 x W2.9 wire mesh.
 - A hydrophilic flexible water stop seal placed in the joints and a 1-foot wide strip of fabric wrapped around the perimeter of the pipe. The fabric will conform to the requirements of Section 831 of the Specifications for Type A Drainage Fabric. The hydrophilic flexible water stop will be from the list below.
 - A self-adhesive external joint seal wrap. The seal wrap will be from the list below.

Approved List of Self-adhesive Joint Wrap

<u>Product</u>	<u>Manufacturer</u>
Mar Mac Seal Wrap	Mar Mac Construction Products McBee, SC 843-335-5909 www.marmac.com
ConWrap CS-217	Concrete Sealants, Inc. Tipp City, OH 800-332-7325 conseal.com

Approved List of Hydrophilic Flexible Water Stop Seal:

<u>Product</u>	<u>Manufacturer</u>
Waterstop RX	Cetco Hoffman Estates, I
	800-527-9948
	www.cetco.com

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Conseal CS-231

Concrete Sealants, Inc. Tipp City, OH 800-332-7325 conseal.com

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Gaskets and seals (mastic, waterstop, and seal wraps) will be installed in accordance with the Manufacturer's recommendations.

The cost for furnishing and installing all gaskets, mastic joint seal, water stop seal, seal wrap, concrete collars, and for plugging the lift holes will be incidental to the contract unit price per foot for the corresponding pipe contract item.

RECONSTRUCTION OF MANHOLES

The Contractor will reconstruct manholes to the extent necessary on this project. Reconstructing the manholes may consist of removing the upper course of brick or removing the concrete walls, replacing the removed materials with brick or Class M6 concrete, placing adjusting rings if necessary, and resetting the manhole frame and lid. The elevation of the lid will be set at the same elevation of the adjacent new pavement or surrounding ground. All manhole frames, lids, and rings that are cracked or broken due to carelessness of the Contractor will be replaced with new manhole frames, lids, and rings that conform with the Specifications at the Contractor's expense. Manholes will be adjusted to the satisfaction of the Engineer. All costs involved in reconstructing the manholes will be incidental to the contract unit price per each for "Reconstruct Manhole".

The Engineer may direct reconstruction of manholes that were not included in these plans. Payment for reconstructing manholes that were not included in the plans will be at the contract unit price per each for "Reconstruct Manhole".

TABLE OF RECONSTRUCT MANHOLES

Alignment	Station	L/R	Type of Adjustment
Anamosa St	12+83	R	Raise 1.28'

TABLE FOR ADJUSTMENT OF WATER VALVES

		Quantity		
Alignment	Station	(Each)	Adjustment	
Anamosa St	13+00	1	Raise 1.57'	
Anamosa St	13+06	1	Raise 0.66'	
Anamosa St	13+23	1	Lower 0.10'	
Anamosa St	13+65	1	Lower 0.30'	
		4	_	

TABLE OF TYPE B66 CONCRETE CURB AND GUTTER

				Quantity
_Alignment	Station to	Station	L/R	(Ft)
Anamosa St	8+70.00	9+57.90	R	87.90
Anamosa St	10+40.21	10+44.04	R	3.83
Anamosa St	8+70.00	9+52.61	L	82.61
Anamosa St	10+38.94	10+44.06	L	5.12
Anamosa St	12+56.31	12+64.60	R	21.03
Anamosa St	12+56.29	12+64.21	L	20.14
Anamosa St	13+25.07	13+74.81	R	49.74
Anamosa St	13+24.97	13+56.12	L	31.16
Nickel St	1+60.00	1+98.41	R	37.46
Nickel St	1+60.00	2+04.82	L	45.76
Nickel St	2+87.97	2+95.45	R	7.49
Nickel St	2+82.31	2+92.39	L	10.11
W Blvd N	9+93.21	9+96.12	R	2.90
W Blvd N	10+14.21	11+39.95	R	125.74
W Blvd N	3+17.30	11+38.92	L	821.61
W Blvd N	12+34.46	12+75.00	R	40.54
W Blvd N	12+25.06	12+75.00	L .	49.94
			Total:	1,443.1

TABLE OF TYPE D46 CONCRETE CURB AND GUTTER

				Quantity
Alignment	Station to	Station	L/R	(Ft)
Anamosa St	13+74.81	13+86.62	R	11.81
			Total:	11.8

TABLE OF TYPE D48 CONCRETE CURB AND GUTTER

Alignment	Station to	Station	L/R	Quantity (Ft)
Alignment	Station to	Station	L/R	(Ft)
Anamosa St	13+80.12	14+10.00	L _	29.87
			Total:	29.9

TABLE OF TYPE D411 CONCRETE CURB AND GUTTER

Alignment	Station to	Station	L/R	Quantity (Ft)
I-190	138+83.20	139+63.35	L	80.15
I-190	138+83.54	139+62.99	R	79.45
I-190	140+34.44	140+38.44	L	4.00
I-190	138+05.79	138+09.79	R	4.00
			Total:	 167.6

TABLE OF TYPE F66 CONCRETE CURB AND GUTTER

				Quantity
Alignment	Station to	Station	L/R	(Ft)
I-190	100+40.71	102+47.72	L	207.00
I-190	100+40.71	102+47.70	R _	207.00
			Total:	414.0

TABLE OF TYPE F611 CONCRETE CURB AND GUTTER

				Quantity
Alignment	Station to	Station	L/R	(Ft)
I-190	141+38.44	141+44.44	L _	6.00
			Total:	6.00

TABLE OF TYPE P6 CONCRETE GUTTER

				Quantity
Alignment	Station to	Station	L/R	(Ft)
Anamosa St	12+99.34	12+99.43	R	18.00
Anamosa St	13+86.62	14+12.62	R _	26.00
			Total:	44.0

TABLE OF TYPE P8 CONCRETE GUTTER

				Quantity
Alignment	Station to	Station	L/R	(Ft)
Anamosa St	13+56.12	13+80.12	L _	24.00
			Total:	24.0

TABLE OF TYPE P11 CONCRETE GUTTER

Alianmont	Station to	Station	L/D	Quantity
Alignment	Station to	Station	L/R	(Ft)
I-190	140+38.44	140+50.43	L	11.99
I-190	141+23.27	141+38.44	L	15.17
I-190	137+93.33	138+05.79	R _	12.47
			Total:	39.6

8" PCC FILLET SECTIONS

Payment for "8" PCC Fillet Section" will be based on plans quantity. If additions or reductions to the area of PCC fillet sections are ordered by the Engineer, payment will be made in accordance with the contract unit price per square yard for "8" PCC Fillet Section".

TABLE OF 8" PCC FILLET SECTION

				Radius	Quantity	
Alignment	Station to	Station	L/R	(Ft)	(SqYd)	
Anamosa St	9+57.90	9+82.80	R	25	23.06	
Anamosa St	9+52.61	9+77.61	L	25	25.90	
Anamosa St	10+15.27	10+40.21	R	25	26.63	
Anamosa St	10+13.94	10+38.94	L	25	29.15	
Anamosa St	13+00.02	13+25.07	R	25	25.80	
Anamosa St	13+00.06	13+24.97	L	25 _	25.96	_
				Total:	156 5	

T	STATE OF	PROJECT	SHEET	TOTAL SHEETS
ı	SOUTH DAKOTA	IM-B 1902(67)0	D6	
1	DAKUTA	1802(07)0	B6	B63

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TABLE OF 6" PCC APPROACH PAVEMENT

Station	L/R	Opening (Ft)	Type	Quantity (SqYd)	
10+05.21 (W Blvd N)	R	18	А	31.2	
13+99.62 (Anamosa	R St)	26	A	57.0	
			Total·	88.2	_

TABLE OF 8" PCC APPROACH PAVEMENT

		Opening	Type	Quantity	
Station	L/R	(Ft)		(SqYd)	
*13+67.50 (Anamosa St)	L	24	A	40.8	_
			Total:	40.8	

^{*} Portion of approach pavement adjacent to sidewalk must meet ADA sidewalk standards.

TABLE OF 4" CONCRETE SIDEWALK

				Quantity	
Alignment	Station to	Station	L/R	(SqFt)	
Anamosa St	9+44.91	9+80.70	R	479.6	
Anamosa St	10+18.08	10+44.04	R	278.0	
Anamosa St	12+55.26	12+61.93	R	159.8	
Anamosa St	13+02.99	13+80.00	R	539.0	
Anamosa St	9+45.04	9+74.95	L	457.8	
Anamosa St	10+16.74	10+44.06	L	462.0	
Anamosa St	12+54.59	12+61.55	L	292.4	
Anamosa St	13+03.71	13+56.23	L	350.1	
Nickel St	1+60.00	1+96.60	L	205.6	
W Blvd N	10+50.09	11+43.58	R	535.3	
W Blvd N	12+27.92	12+75.00	R _	238.0	

Total: 3997.6

TYPE 1 DETECTABLE WARNINGS

Detectable warnings will be in compliance with the Americans with Disabilities Act regulations.

The detectable warnings will be installed according to the manufacturer's installation instructions.

A concrete thickness equal to the adjacent concrete sidewalk thickness and 2 inches of granular cushion material will be placed below the Type 1 Detectable Warnings. When concrete is placed below the detectable warnings then the concrete thickness will be transitioned at the rate of 1" per foot to match the adjacent concrete sidewalk thickness.

The detectable warnings will be a brick red color for application in concrete curb ramps.

Type 1 Detectable Warning Panels will be one of the following products:

Type 1 Detectable Warnings

<u>Product</u>	<u>Manufacturer</u>
Detectable Warning Plate Cast Iron Plate	Neenah Foundry Company Neenah, WI 800-558-5075 http://www.neenahfoundry.com/
Detectable Warning Plate Cast Iron Plate	Deeter Foundry Lincoln, NE 800-234-7466 http://www.deeter.com/
TufTile (wet-set) Cast Iron Replaceable Tile	TufTile 1200 Flex Court Lake Zurich, IL 60047 888-960-8897 http://www.tuftile.com/

TABLE OF TYPE 1 DETECTABLE WARNINGS

		Width of Curb Opening	Quantity
* Station	L/R	(Ft)	(SqFt)
9+67.93	26.10' L	10	20
9+72.76	24.02' R	10	20
10+23.67	27.79' L	10	20
10+23.83	27.89' R	6	12
12+54.60	27.60' L	10	20
12+55.27	27.37' R	6	12
13+09.45	26.72' R	5	10
13+09.63	36.85' L	10	20
		Total:	134

^{*} All Stations are referenced off of Anamosa Street Alignment

PUBLIC LANDS SURVEY SYSTEM, RIGHT OF WAY, AND PROPERTY CORNERS

The Contractor will have a Land Surveyor, licensed in the State of South Dakota, to set, reestablish or verify public land survey system (PLSS) corners, right of way (ROW) corners, and property corners as directed by the appropriate SDDOT Region Land Surveyor. It is estimated that 7 ROW and property corners will be set, reestablished, or verified for this project. The Contractor's Land Surveyor, under the direction of the Region Land Surveyor, will set, reestablish, or verify all corner monuments after surfacing and fencing operations are completed in accordance with the PUBLIC LANDS SURVEY SYSTEM CORNERS section and the RIGHT OF WAY AND PROPERTY CORNERS section in Chapter 8 of the SDDOT Survey Manual.

< https://dot.sd.gov/doing-business/engineering/design-services/surveyors >

All costs associated with furnishing and installing PLSS caps, rebar, and all other materials associated with setting, reestablishing, or verifying PLSS, ROW corners, and property corners in accordance with the SDDOT Survey Manual will be incidental to the contract unit price per each for "Reestablish Right-of-Way and Property Corner".

TABLE OF GUARDRAIL

	Salvage Beam Guardrail	Type 2 MGS	MGS Trailing End	MGS MASH Tangent
Location			Terminal	End Terminal
	(Ft)	(Ft)	(Each)	(Each)
Sta.138+74.17 to	202	150	1	1
Sta. 141+02.67 Lt. (I-190)				
Sta.137+44.68 to	219	150	1	1
Sta. 139+82.59 Rt (I-190)				
Totals:	421	300	2	2

STATE OF	PROJECT	SHEET	TOTAL SHEETS
SOUTH DAKOTA	IM-B 1902(67)0	B7	B63

Plotting Date: 04/04/2023 Rev 4/04/2023 BT

TABLE OF SUPERELEVATION

_Alignment	Station	to	Station		
Nickel St	1+60		1+90	-	Superelevation Transition To Match Existing Roadway
Nickel St	1+90		1+90	-	Normal Crown
Nickel St	1+90		2+25	-	Superelevation Transition
Nickel St	2+25		2+63	-	Reverse Crown
Anamosa St	8+70		9+10	-	Superelevation Transition To Match Existing Roadway
Anamosa St	9+10		14+10	-	Normal Crown
W Blvd N	10+00		10+50	-	Superelevation Transtion To Match Existing Roadway
W Blvd N	10+50		11+19	-	Normal Crown Section
W Blvd N	11+19		11+39	-	Superelevation Transition
W Blvd N	11+39		12+49	-	Reverse Crown Section
W Blvd N	12+49		12+69	-	Superelevation Transtion
W Blvd N	12+69		12+69	-	Normal Crown Section
W Blvd N	12+69		12+75	-	Superelevation Transition To Match Existing Roadway

TABLE OF CONSTRUCTION STAKING
(See Special Provision for Contractor Staking)

						G	rade Staking	3			
Roadway and Description	Begin Station	End Station	Number of Lanes	Length (Ft)	Length (Mile)	Lane Factor	*Sets of Stakes	**Grade Staking Quantity (Mile)	Miscellaneous Staking Quantity (Mile)	Slope Staking Quantity (Mile)	Structure Staking Quantity (Each)
Anamosa Street (2 Lanes AC Pavement)	8+70	10+64	2	194	0.037	1	2	0.074	0.037	0.037	
Str No 52-410-290											1
Anamosa Street (2 Lanes AC Pavement)	12+36	14+10	2	174	0.033	1	2	0.066	0.033	0.033	
Temporary Pedestrian Bridge											*** 1
Nickel Street (2 Lanes AC Pavement)	1+60	2+24	2	64	0.012	1	2	0.024	0.012		
W Blvd N (2 Lanes AC Pavement)	10+00	11+60	2	164	0.031	1	2	0.062	0.031		
W Blvd N (2 Lanes AC Pavement)	12+04	12+75	2	75	0.014	1	2	0.028	0.014		
I-190 SB (2 Lanes PCCP)	115+21	120+04	2	483	0.091	1	2	0.182	0.091		
I-190 NB (2 Lanes PCCP)	115+21	120+04	2	483	0.091	1	2	0.182	0.091		
I-190 (Median Concrete Barrier w/ Shoulders)	115+21	120+04	2	483	0.091	1	1	0.091			
I-190 (Median Concrete Barrier w/ Shoulders)	138+83	139+63	2	80	0.015	1	1	0.015			
I-190 (Median Concrete Barrier w/ Shoulders)	155+64	158+39	2	275	0.052	1	1	0.052			
							Totals:	0.776	0.309	0.070	2

^{* 2 =} Blue Top and Paving Hub Stakes (PCC Pavement) or 2 Sets of Blue Tops (AC Pavement)

STATE OF SOUTH	PROJECT	SHEET	TOTAL SHEETS
DAKOTA	IM-B 1902(67)0	B8	B63

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^{**} Grade Staking Quantity = (Length) x (Lane Factor) x (Sets of Stakes)

^{***} Structure Staking Quantity will include any Temporary Grade Staking to construct Temporary Pedestrian Bridge Berm

PIPE QUANTITIES

1	STATE OF	PROJECT	SHEET	TOTAL	
١	SOUTH			SHEETS	
ı	DAKOTA	IM-B 1902(67)0	В9	B63	

12/28/2022

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•				Reinfo	rced Concrete		
		Circular			Circular Bends		
		18"	24"	30"	36"	30"	
		Cl. 2	Cl. 2	Cl. 2	Cl. 2	42.5°	
Station	Offset (L/R)	Ft	Ft	Ft	Ft	Each	
*3+17.25-20.24' L to 6-	+14.86-20.79' L (WBlvdN)				298		
*6+19.86-20.79' L to 9-	+14.87-21.15' L (WBlvdN)				296		
*9+19.87-21.15' L to 12	2+53.87-22.30' L (WBlvdN)				334		
12+59.37-20.30' L to	12+45.00-19.78' R (WBlvdN)				44		Pipes denoted with an asterisk()
*13+04.20-57.28' L to	13+40.88-31.27' L			48		1	indicate that the entire length or a
*13+51.88-31.27' L to	14+04.51-28.61' L		54				portion of the pipe requires watertight joints in accordance with
*13+46.38-29.27' L to	13+46.38-19.66' R	50					the STORM SEWER plan note.
	Subtotal:	50	54	48	972	1	

FENCE QUANTITIES

STATE (STATE OF	PROJECT	SHEET	TOTAL SHEETS
SOUTH		IM-B 1902(67)0	B10	B63

Plotting Date:

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			Right-of-Way Fence		Fence	
Station ·	to Station	Side (L/R)	5' Chain Link /with top rail (Ft)	Reset (Ft)	Remove (Ft)	**** Remove for Reset (Ft)
***13+23	13+80	R				
**12+26	12+78	R	52		52	
*138+41	138+95	L		57	·	57
*130+34	138+95	R		865		865
*139+55	140+19	L		72		72
*139+55	140+19	R		80		80
		TOTALS:	52	1074	52	1074

Post Type and Sequence:

Right-of-way fence shall be constructed using alternate wood and steel posts except as noted.

- * I-190 Stationing
- ** WBlvdN Stationing
- *** Fence and Landscape Block will be removed by Landowner. Contractor should work with Landowner to coordinate this work. **** The Contractor will replace fence removed for pipe installation no more than 2 weeks after removal at any given location. Exceptions will be for construction of the new structure over I-190 and the temporary pedestrian bridge where the Contractor will make a reasonable effort to replace fence and make temporary or permanent

connections to the structures.

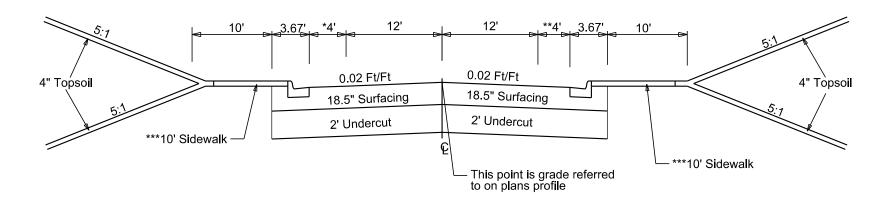
TYPICAL GRADING SECTION

T	STATE OF	PROJECT	SHEET	TOTAL SHEETS	
	SOUTH DAKOTA	IM-B 1902(67)0	B11	B63	

Plotting Date:

12/28/2022

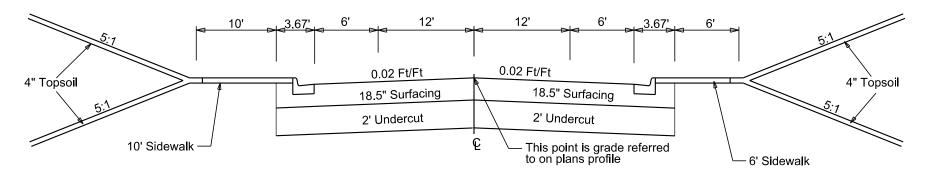
Anamosa Street 8+70 to 9+77.59



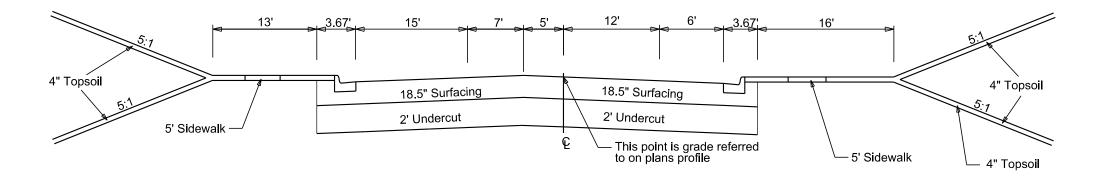
Transitions:

- * 8+70 to 9+00 3.59' to 4'
 ** 8+70 to 9+00 3.79' to 4'
- *** 10' Sidewalk starts at 9+45

<u>Anamosa Street</u> 9+77.59 to 10+64.05 12+36.05 to 12+99.97



Anamosa Street 12+99.27 to 14+10.00



TYPICAL GRADING SECTION

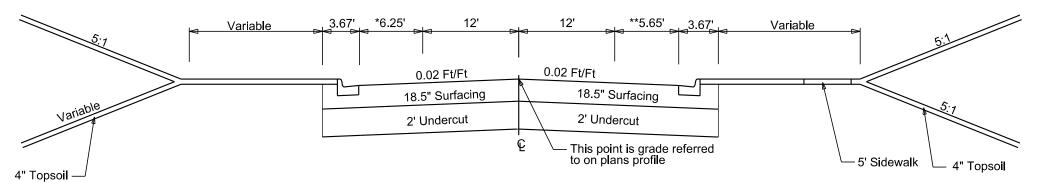
STATE OF	PROJECT	SHEET	TOTAL SHEETS	
SOUTH DAKOTA	IM-B 1902(67)0	B12	B63	

Plotting Date: 12/21/2022

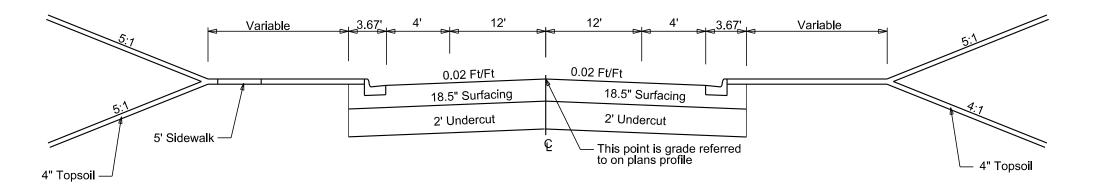
W Blvd N 10+00.00 to 11+60.45 12+03.82 to 12+75.00

Transitions:

* 10+50 to 11+60.45 - 4.50 to 6.25 ** 10+50 to 11+60.45 - 7.00 to 5.65



Nickel St 1+60 to 2+24



HORIZONTAL ALIGNMENT DATA

Type

POE

162+18.55

Station

STATE OF	PROJECT	SHEET	TOTAL SHEETS	
SOUTH DAKOTA	IM-B 1902(67)0	B13	B63	

Easting

1207169.988

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Plotting Date:

Northing

658285.828

М	Δ	INI	IN	F

Type	Station			Northing	Easting
POB	6+99.60			656006.318	1206635.796
DI	0.00.45	TL= 299.55	S 88°07'49" E	055000 544	4000005 400
PI	9+99.15	TL= 150.90	S 88°02'15" E	655996.544	1206935.186
PI	11+50.05	16- 150.90	3 00 02 13 L	655991.376	1207085.997
		TL= 132.16	S 87°59'31" E		
PI	12+82.21			655986.745	1207218.074
		TL= 317.79	S 88°02'07" E		
POE	16+00.00			655975.850	1207535.680
			Nickel St		
Type	Station			Northing	Easting
POB	0+00.00			655753.206	1206942.190
DI	0.04.47	TL= 224.17	N 2°31'46" W	655977.153	4000000 007
PI	2+24.17	TL= 169.98	N 6°04'45" E	055977.153	1206932.297
POE	3+94.15	12 100.00	11 0 04 40 E	656146.177	1206950.298
			W Blvd N		
Type	Station			Northing	Easting
POB	2+26.06			655031.486	1207178.635
ΡI	3+51.08	TL= 125.02	N 2°35'01" E	655156.379	1207194 274
P1	3+31.00	TL= 648.92	N 2°16'21" E	000100.379	1207184.271
PI	10+00.00	16-040.02	142 1021 L	655804.791	1207210.003

N 2°32'38" E

N 2°10'10" E

N 2°14'36" E

TL= 182.16

TL= 127.84

TL= 219.26

11+82.16

13+10.00

15+29.26

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POE

POB	100+03.71			652103.451	1206659.657
		TL= 240.53	N 14°46'24" E		
PC	102+44.24			652336.031	1206720.993
PI	104+98.03	R = 3560.00	Delta = 8°09'19" L	652581.430	1206785.708
PT	107+50.96			652833.526	1206814.957
		TL= 770.25	N 6°37'05" E		
PI	115+21.21			653598.648	1206903.730
		TL= 966.06	N 6°36'39" E		
PC	124+87.28			654558.289	1207014.948
PI	127+62.24	R = 6770.00	Delta = 4°39'06" L	654831.422	1207046.603
PT	130+36.90			655106.222	1207056.004
		TL= 865.61	N 1°57'33" E		
PC	139+02.51			655971.325	1207085.598
PI	139+02.66	R = 14100.00	Delta = 0°00'04" R	655971.474	1207085.604
PT	139+02.80			655971.623	1207085.609
		TL= 361.51	N 1°57'38" E		
PI	142+64.31			656332.919	1207097.976

TL= 1954.24

I-190

TEMP PED BRIDGE

N 2°06'42" E

Туре	Station			Northing	Easting
POB	10+14.43			656048.046	1206952.241
		TL= 249.82	S 88°00'50" E		
POE	12+64.25			656039.388	1207201.912

1207218.088

1207222.927

1207231.510

655986.768

656114.520

656333.609

Plotting Date:

12/21/2022

HORIZONTAL AND VERTICAL CONTROL POINTS							
POINT	STATION	OFFSET	DESCRIPTION	NORTHING	EASTING	ELEVATION	
CP01	8+41	4353' R	NAIL	651651.1	1206635	3236.23	
CP02	9+64	2955' R	NAIL	653044.7	1206804	3241.24	
CP03	10+96	1425' R	NAIL	653963.9	1207175	3294.38	
CP04	13+08	2023' R	NAIL	654568.9	1206983	3287.71	
CP05	12+55	75' R	NAIL	655913.2	1207188	3310.58	
CP06	Off	Project	NAIL	651444.2	1205921	3240.85	
RCB2036	9+66	43' R	NAIL	655955.03	1206900	3309.92	
RCB2035R	8+74	1951' R	NAIL	654050.34	1206746	3259.31	

^{*}Stations and Offsets are referenced off of Mainline

LEGEND

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STATE OF	PROJECT	SHEET	TOTAL SHEETS
SOUTH DAKOTA	IM-B 1902(67)0	B15	B63

Plotting Date: 12/21/2022

Anchor	-
Antenna	
Approach	
Assumed Corner Azimuth Marker	⑦ ▲
BBQ Grill/ Fireplace	<u>a</u>
Bearing Tree	<u>-</u>
Bench Mark	<u> </u>
Box Culvert	
Bridge	
Brush/Hedge	62523
Buildings	
Bulk Tank	
Cattle Guard	===
Cemetery	t
Centerline	
Cistern	©
Clothes Line	. Control
Concrete Symbol	
Control Point	A
Creek Edge	
Curb/Gutter Curb	
Dam Grade/Dike/Levee	
Deck Edge	
Ditch Block	<u>7700</u>
Doorway Threshold	
Drainage Profile	
Drop Inlet	
Edge Of Asphalt	
Edge Of Concrete	
Edge Of Gravel	
Edge Of Other	
Edge Of Shoulder	
Electric Transformer/Power Junctio	n Box 🕑
Fence Barbwire	
Fence Chainlink	
Fence Electric	
Fence Miscellaneous	
Fence Rock	000000000000000000000000000000000000000
Fence Snow Fence Wood	
Fence Woven	
Fire Hydrant	В
Flag Pole	}
Flower Bed	7777
Gas Valve Or Meter	4
Gas Pump Island	<u> </u>
Grain Bin	(B)
Guardrail	0-0-
Gutter	2222
Guy Pole	P
Haystack	
Highway ROW Marker	7-0
Interstate Close Gate	\vec{A}
Iron Pin	©
Irrigation Ditch	

Lake Edge

Lawn Sprinkler

NA-St.
Mailbox
Manhole Electric
Manhole Gas
Manhole Miscellaneous
Manhole Sanitary Sewer Manhole Storm Sewer
Manhole Telephone
Manhole Water
Merry-Go-Round
Microwave Radio Tower
Miscellaneous Line
Miscellaneous Property Corner
Miscellaneous Post
Overhang Or Encroachment
Overhead Utility Line
Parking Meter
Pedestrian Push Button Pole
Pipe With End Section
Pipe With Headwall
Pipe Without End Section
Playground Slide
Playground Swing
Power And Light Pole
Power And Telephone Pole
Power Meter
Power Pole
Power Pole And Transformer
Power Tower Structure
Propane Tank
Property Pipe
Property Pipe With Cap
Property Stone
Public Telephone
Railroad Crossing Signal
Railroad Milepost Marker Railroad Profile
Railroad ROW Marker
Railroad Signs
Railroad Switch
Railroad Track
Railroad Trestle
Rebar
Rebar With Cap
Reference Mark
Retaining Wall
Riprap
River Edge
Rock And Wire Baskets
Rockpiles
Satellite Dish
Septic Tank
Shrub Tree
Sidewalk
Sign Face
Sign Post

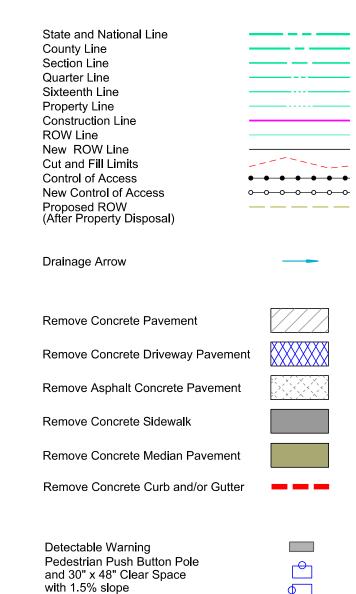
Slough Or Marsh

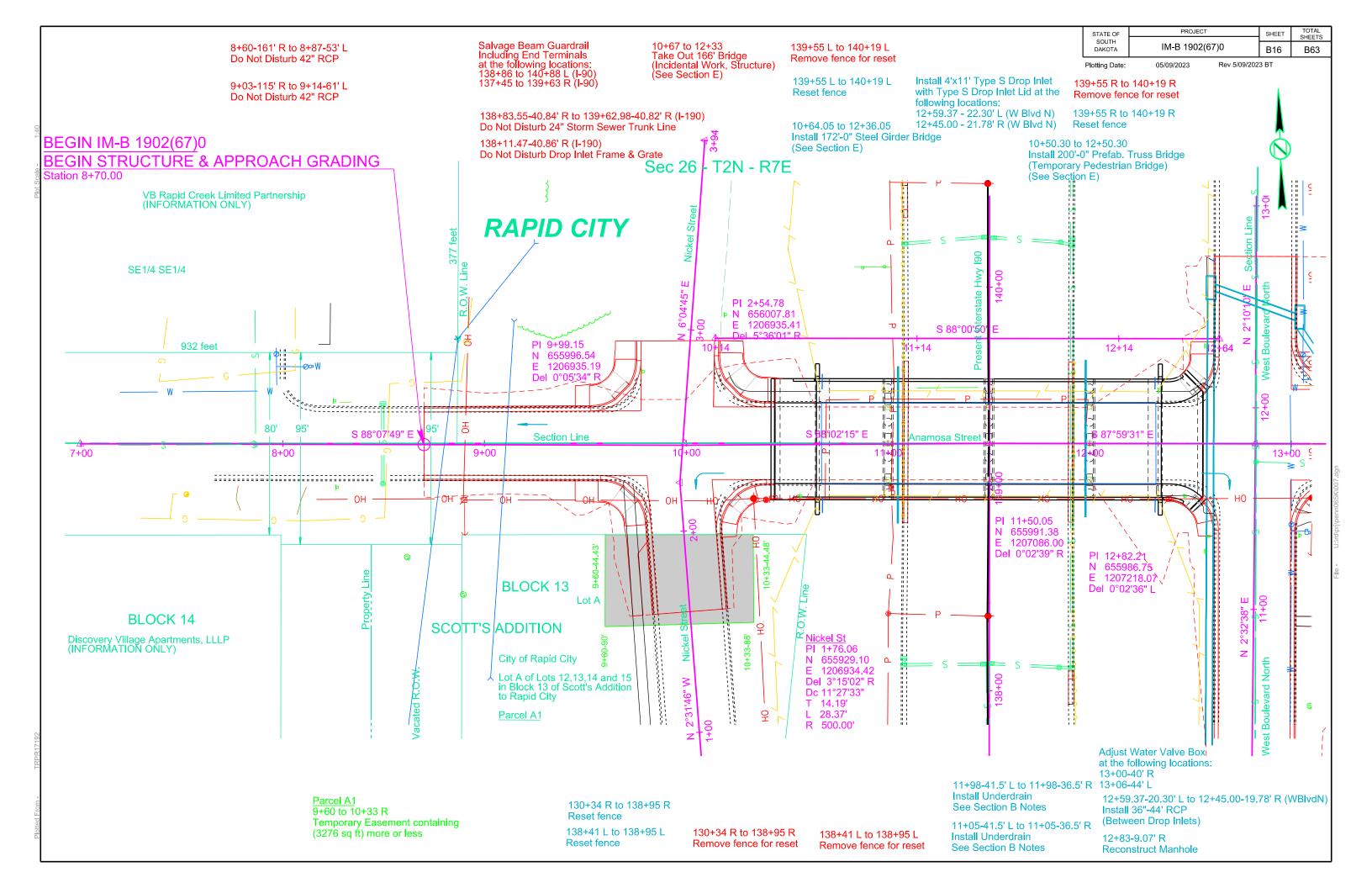
Stream Gauge

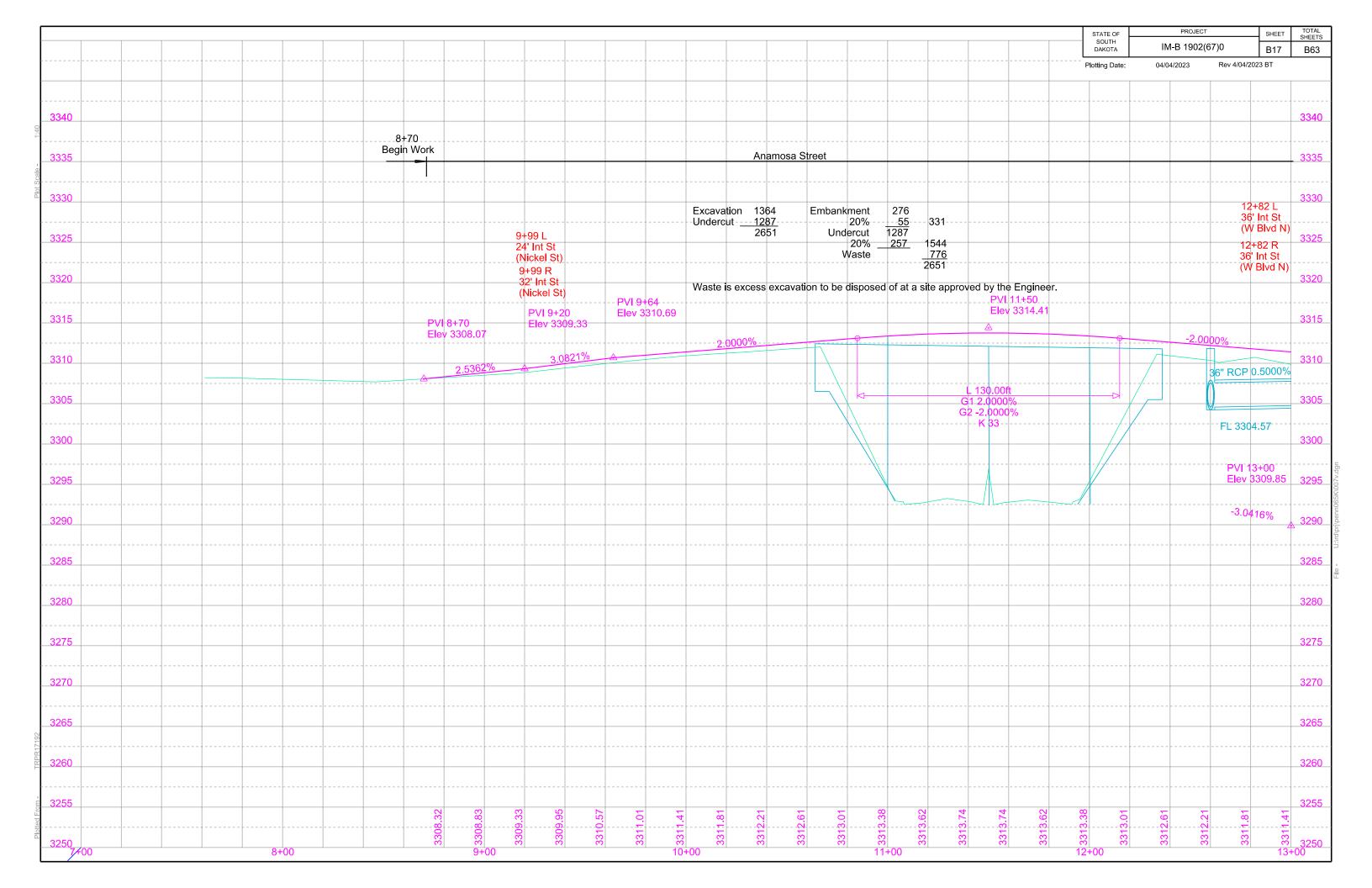
Street Marker

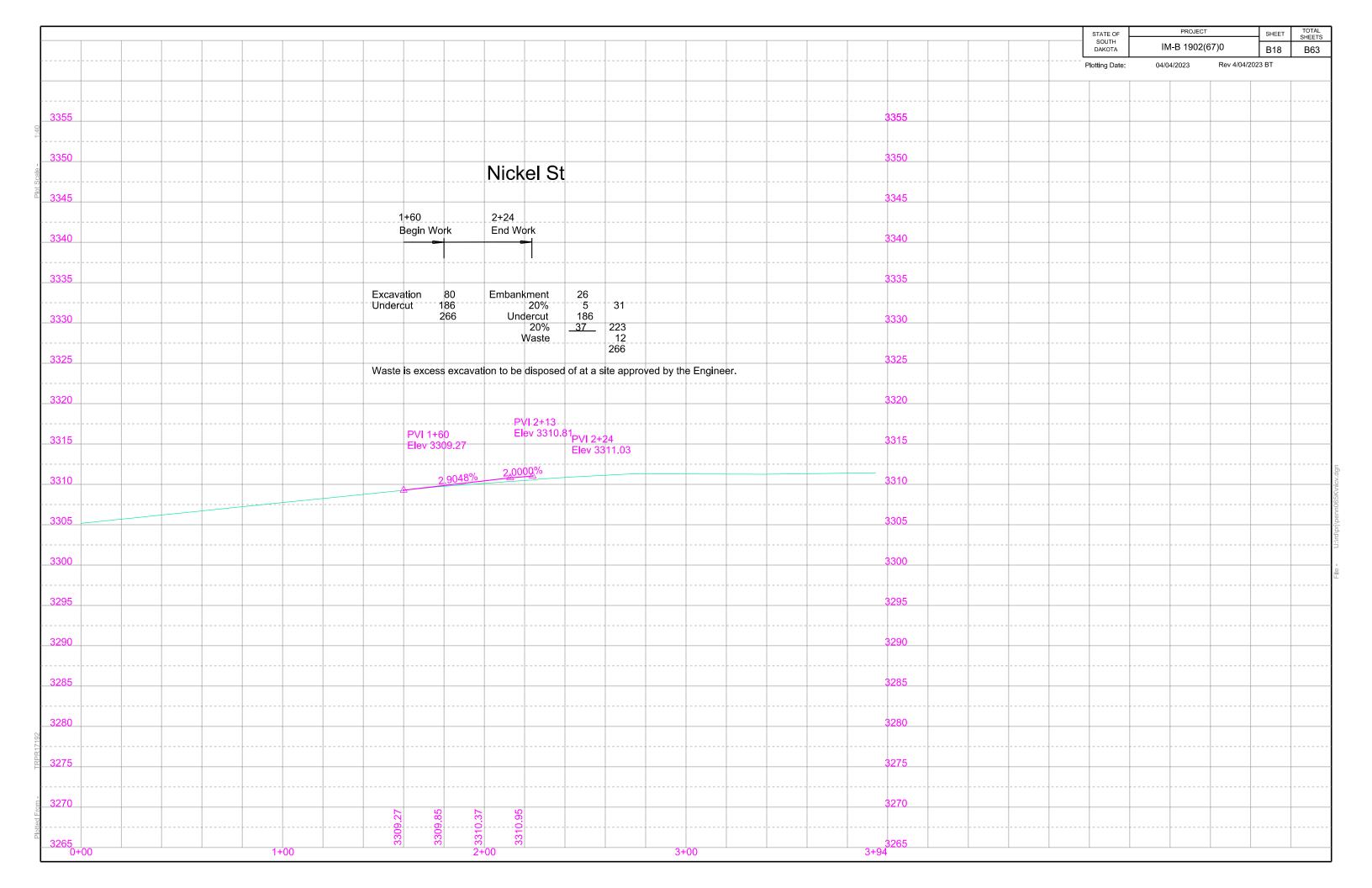
Spring

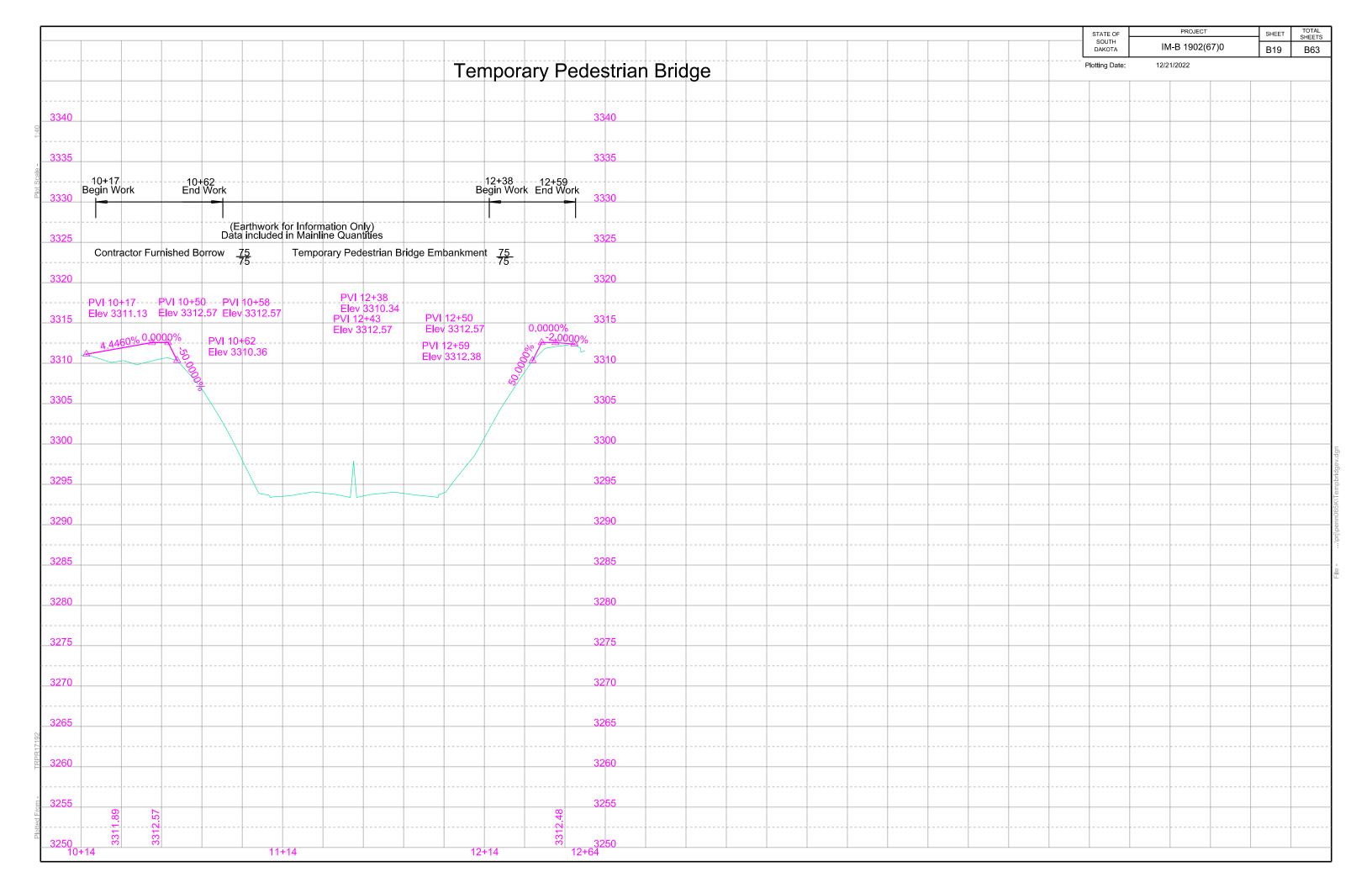
Subsurface Utility Exploration Test Hole	•
Telephone Fiber Optics	— T/F —
Telephone Junction Box	\blacktriangledown
Telephone Pole	Ø
Television Cable Jct Box	∞
Television Tower	夲
Test Wells/Bore Holes	<u>(A)</u>
Traffic Sign Double Face	Ħ
Traffic Sign One Post	þ
Traffic Sign Two Post	8
Traffic Signal	₩
Trash Barrel	•
Tree Belt	~~~
Tree Coniferous	*
Tree Deciduous	<u> </u>
Tree Stumps	A
Triangulation Station	Δ
Underground Electric Line	— P —
Underground Gas Line	— G —
Underground High Pressure Gas Line	— HG —
Underground Sanitary Sewer	— s —
Underground Storm Sewer	= s =
Underground Tank	
Underground Telephone Line	— т —
Underground Television Cable	- TV $-$
Underground Water Line	- w -
Water Fountain	Ţ
Water Hydrant	0
Water Meter	M
Water Tower	
Water Valve	Ø
Water Well	•
Weir Rock	
Windmill	8
Wingwall	
Witness Corner	((C)

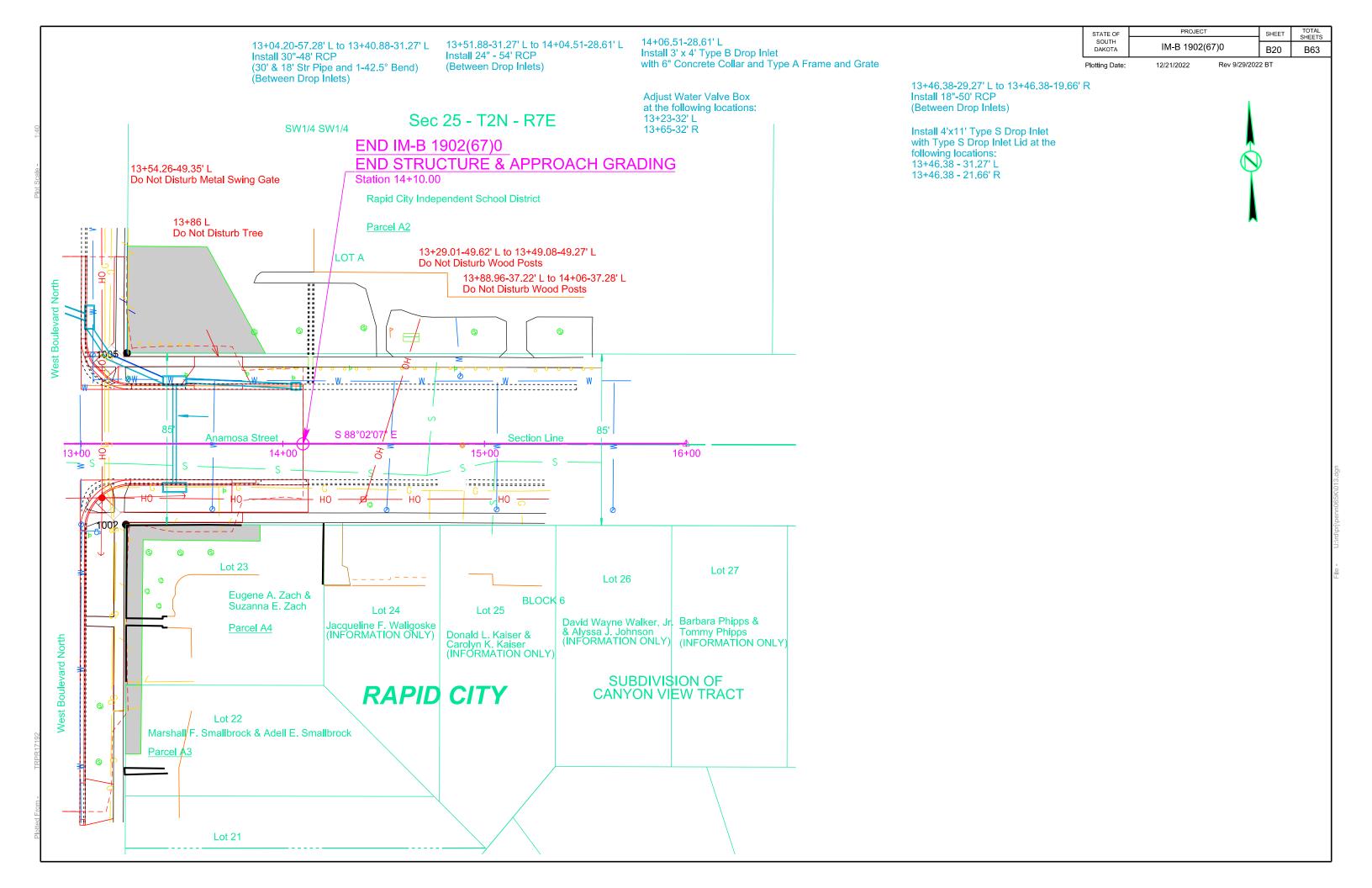


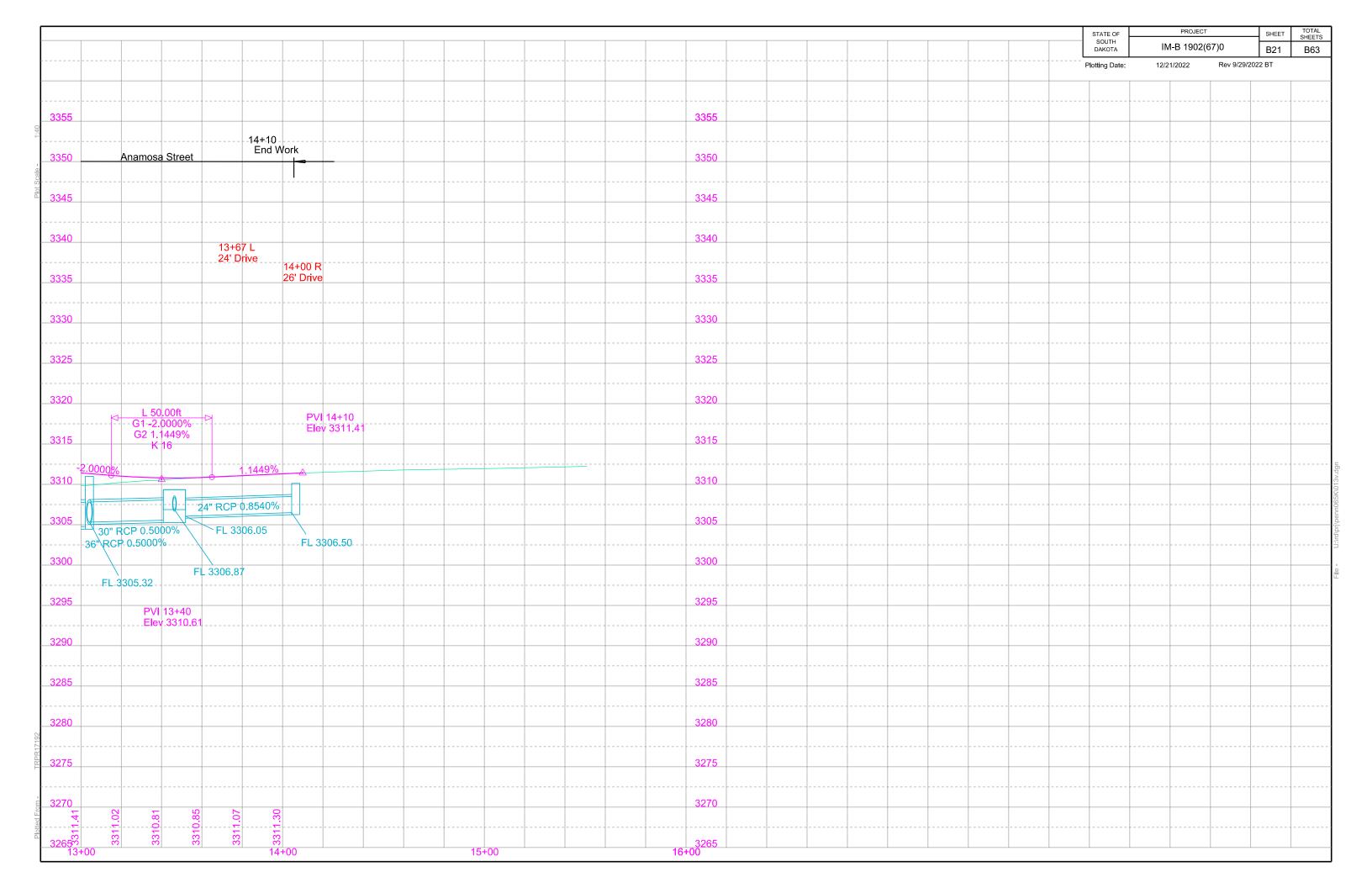


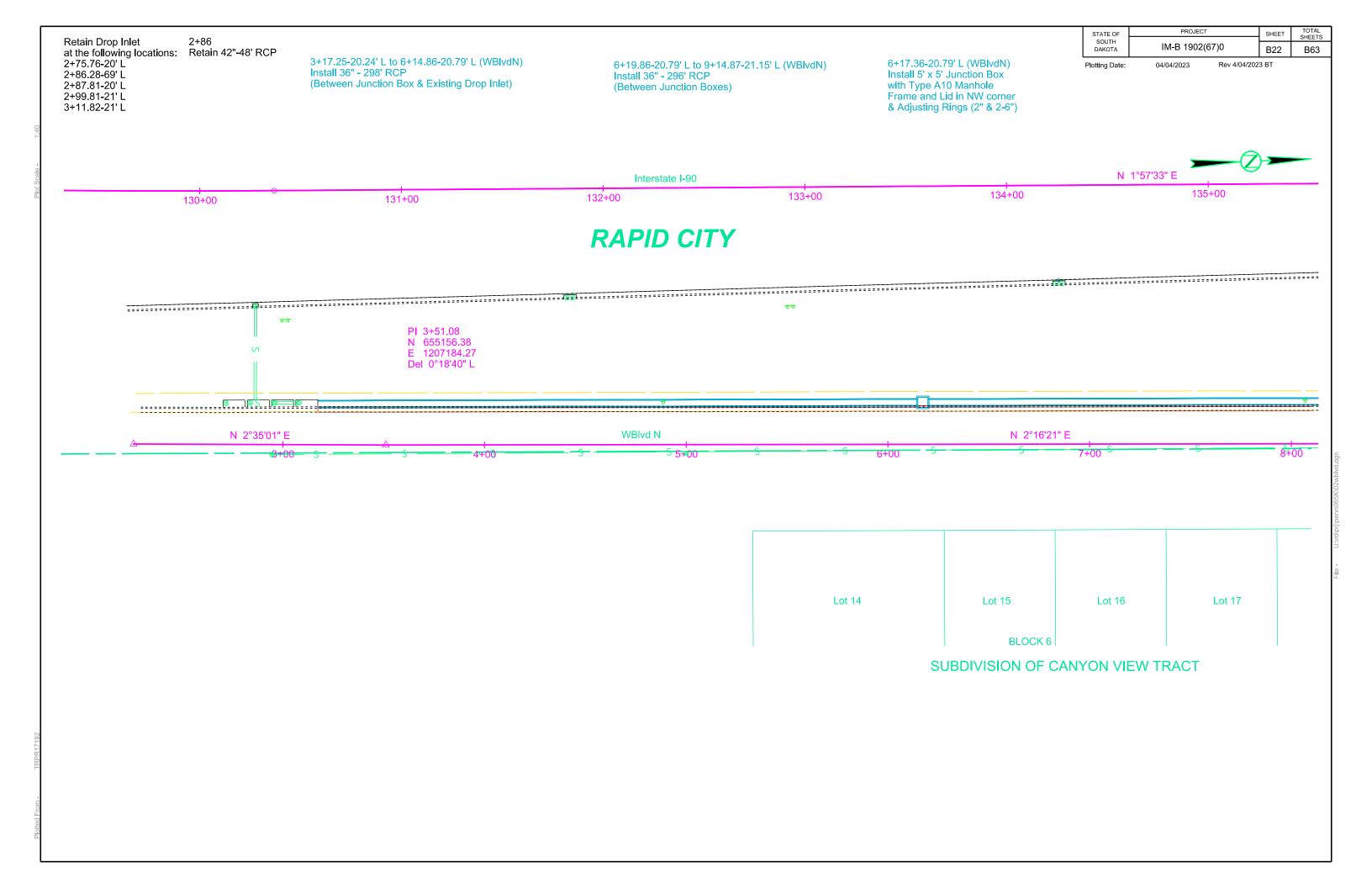


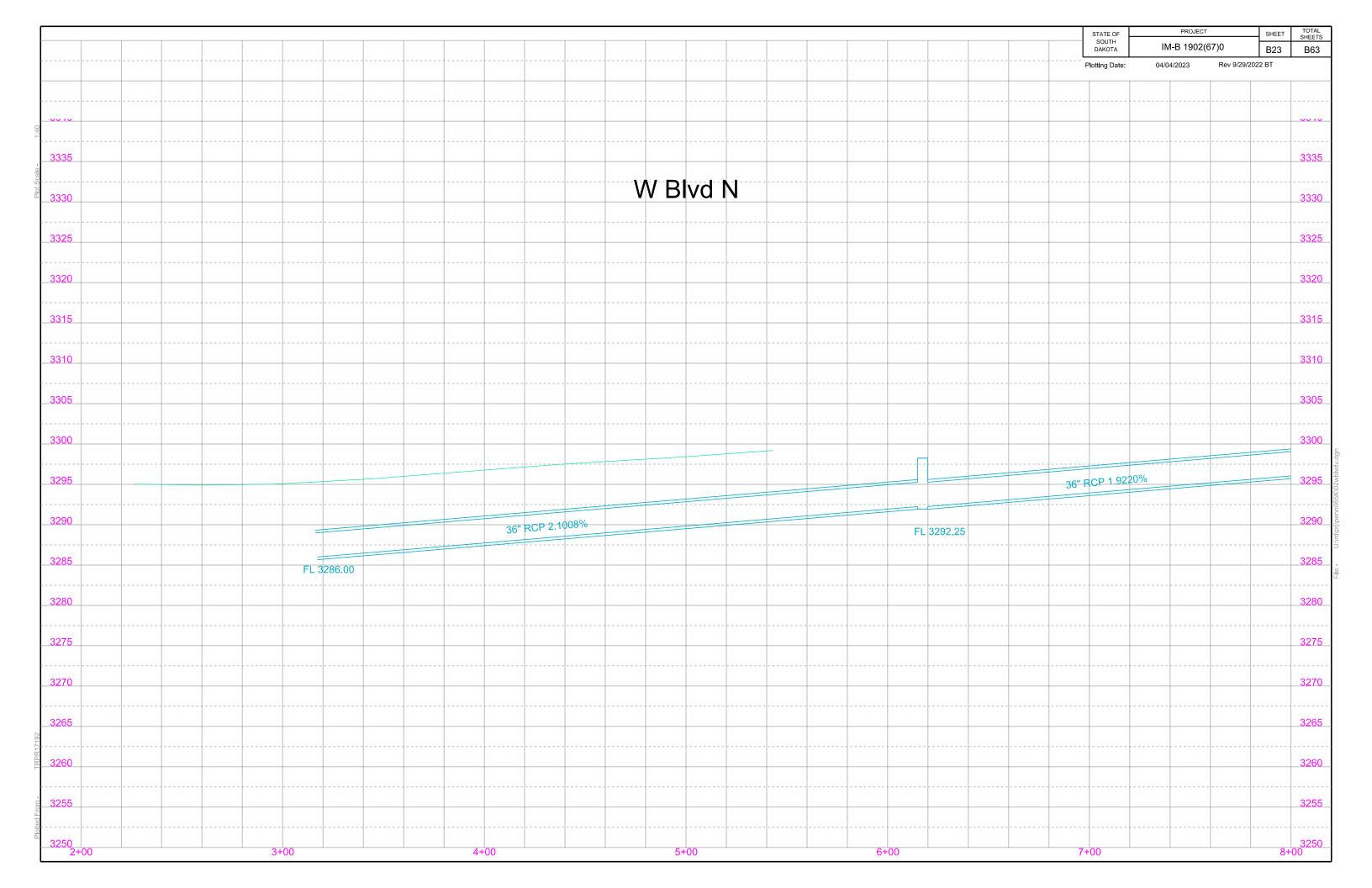


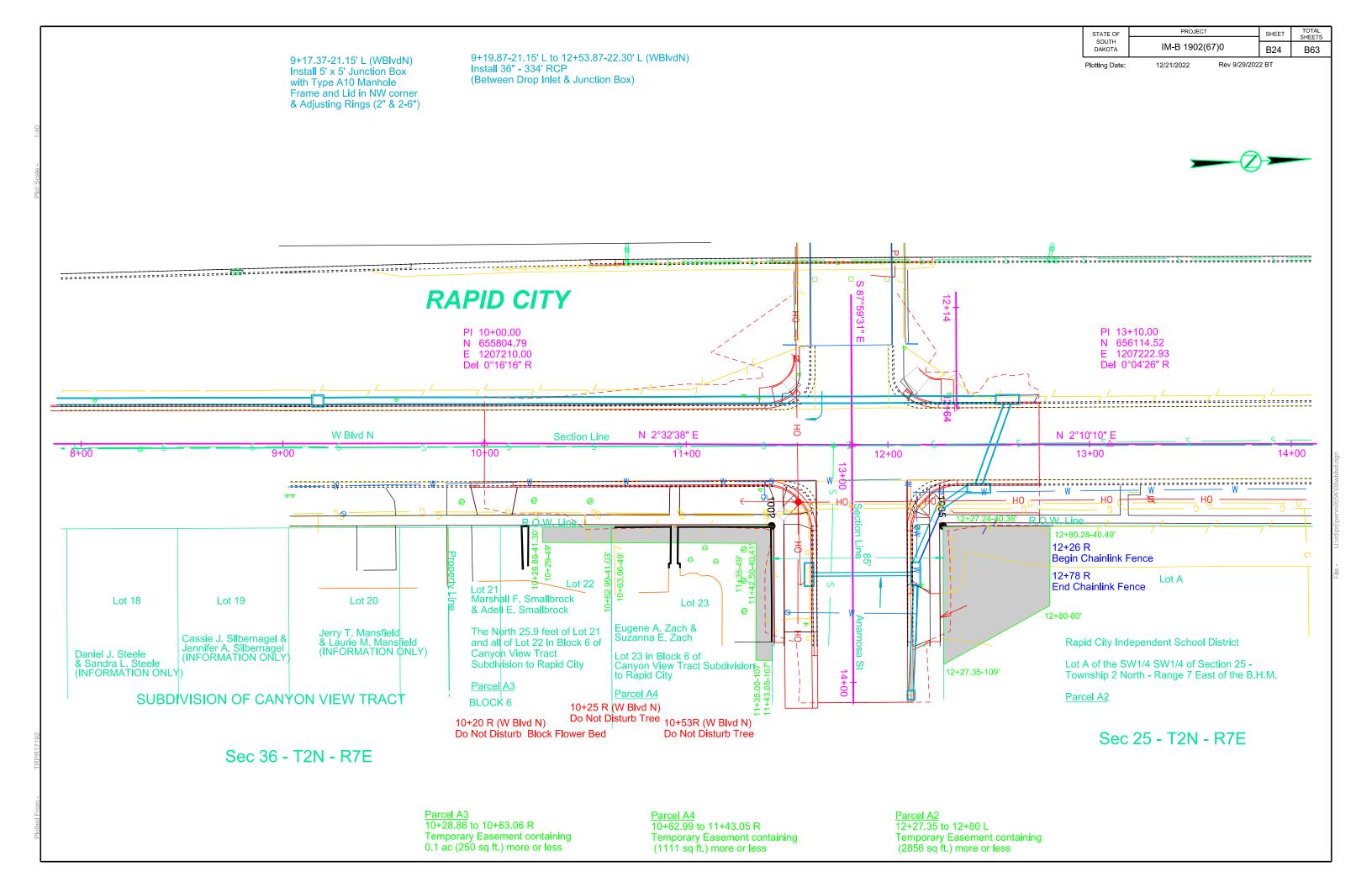


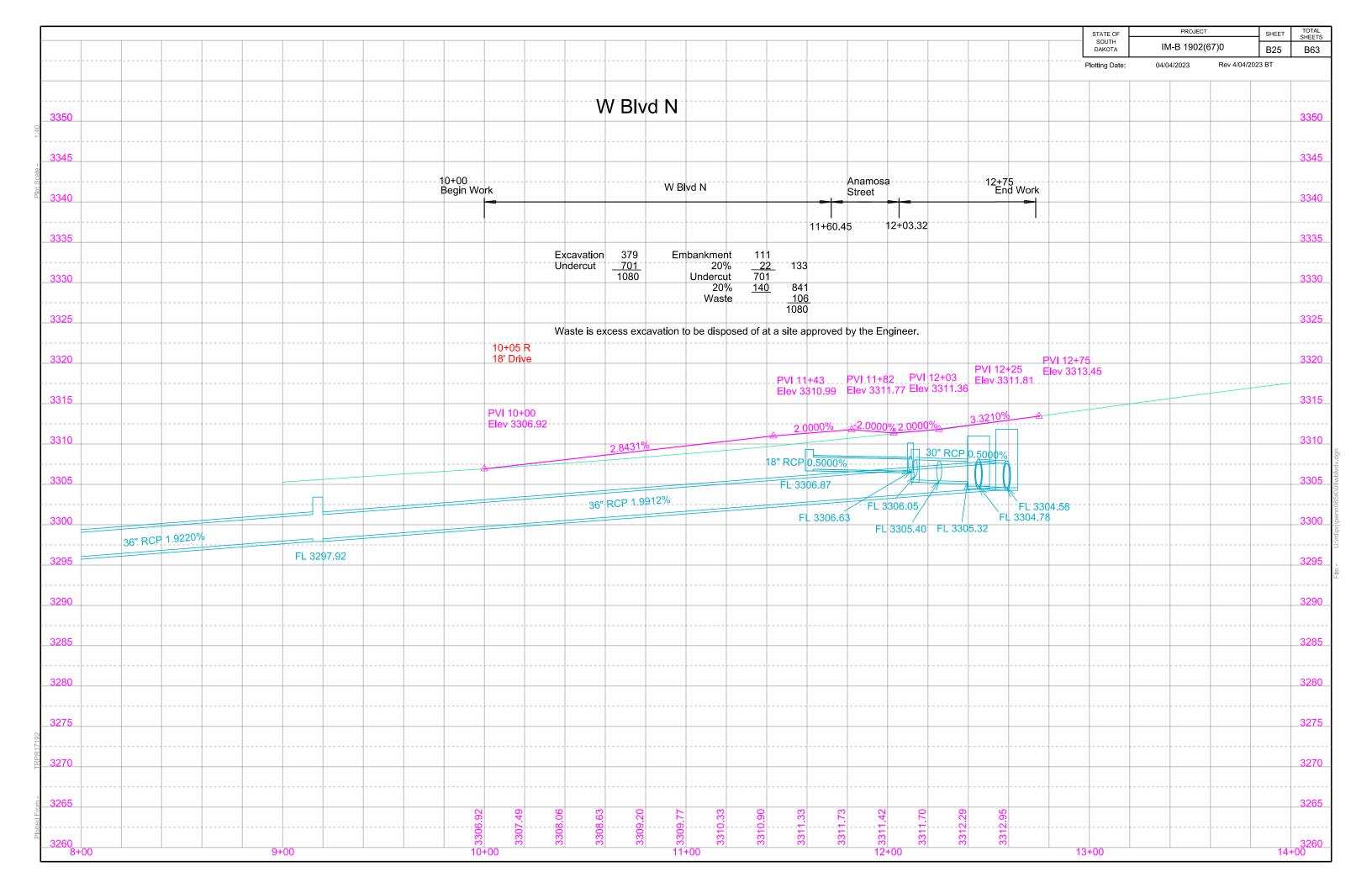


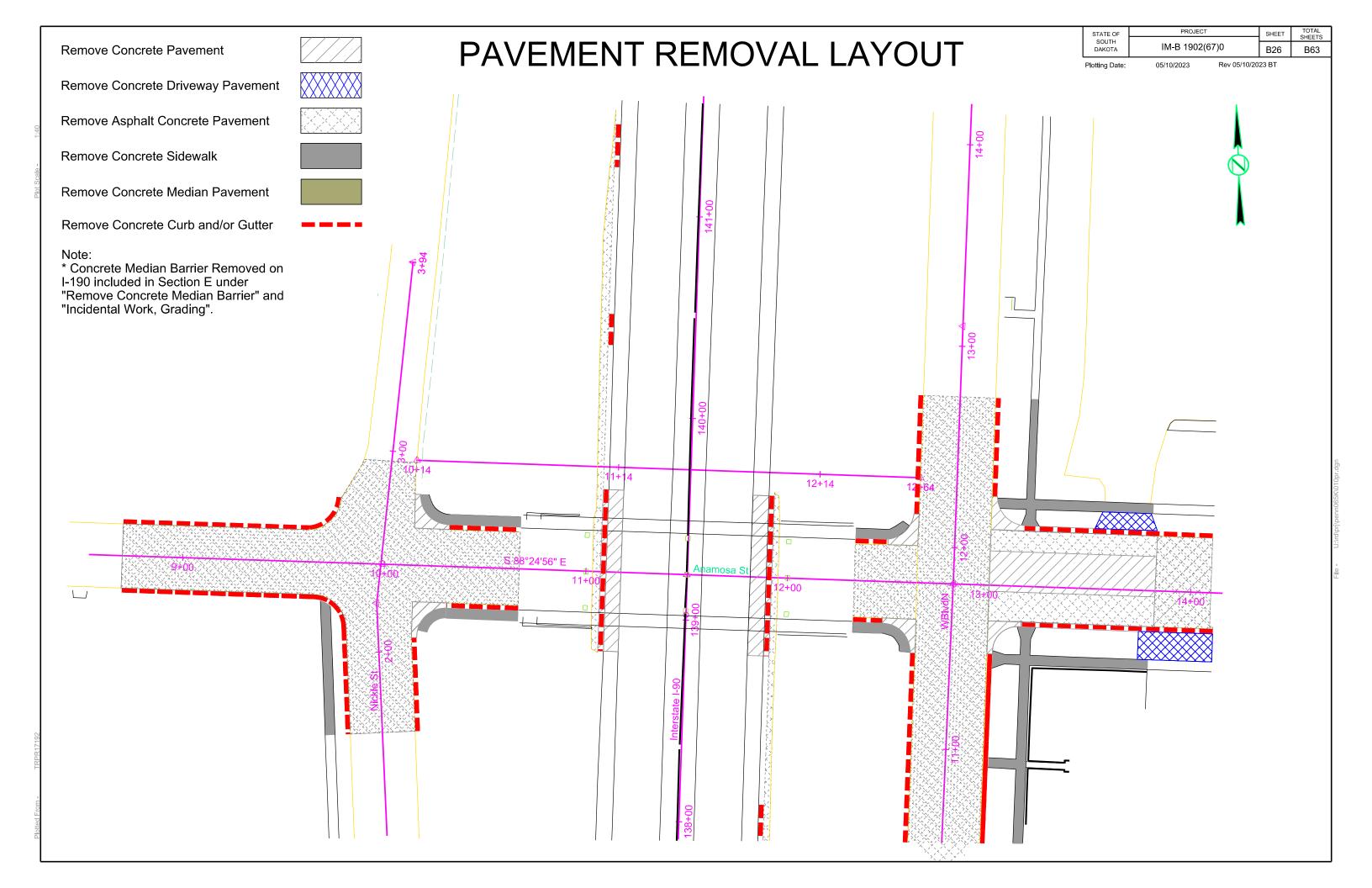












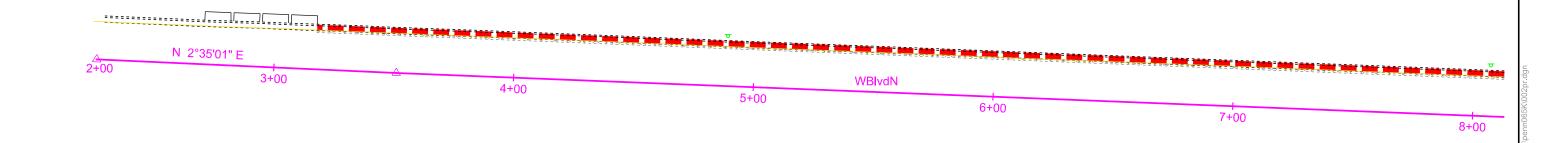
PAVEMENT REMOVAL LAYOUT
W Blvd N

 STATE OF SOUTH DAKOTA
 PROJECT
 SHEET
 TOTAL SHEETS

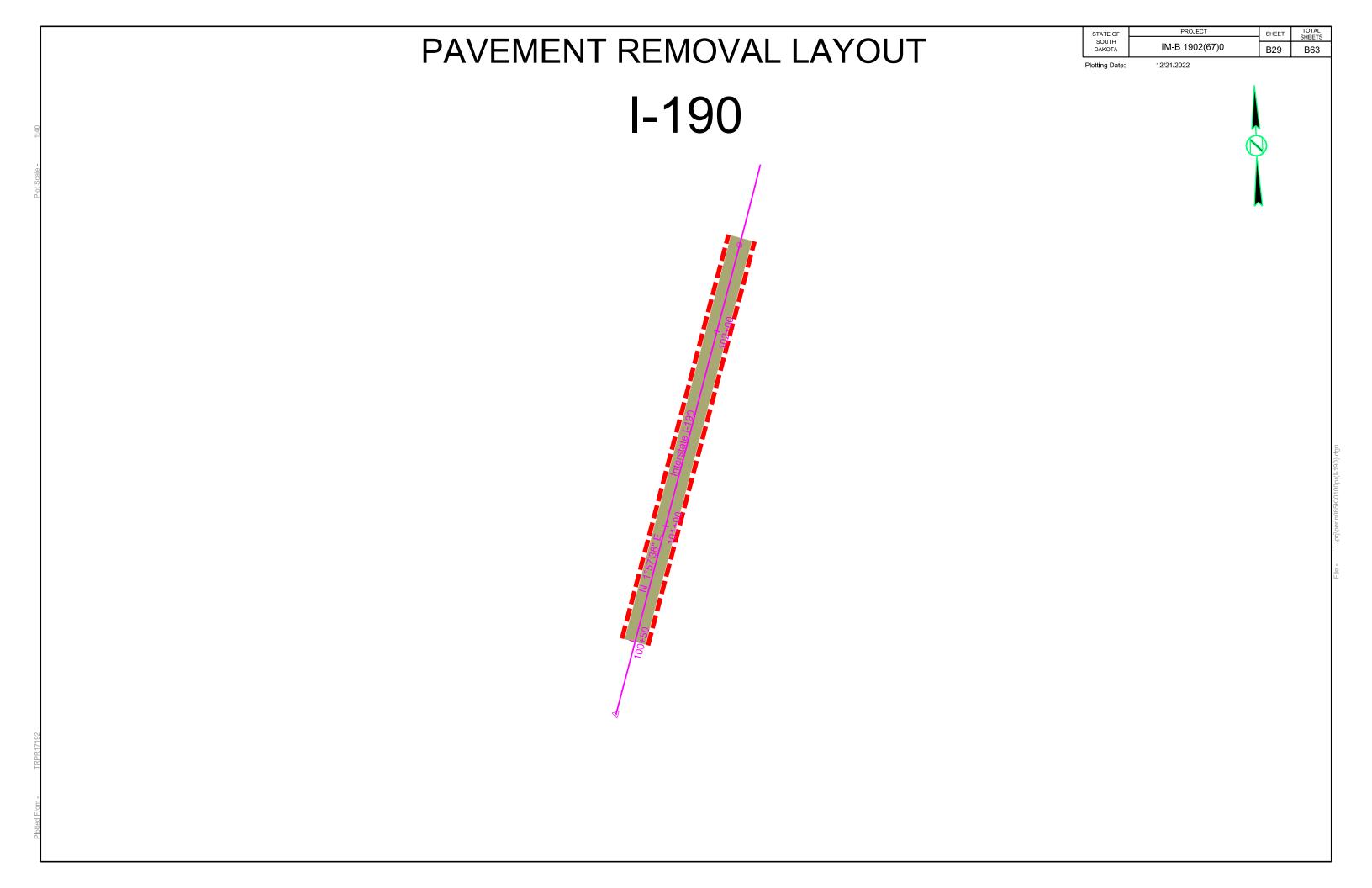
 B27
 B63

otting Date: 1





STATE OF SOUTH DAKOTA PAVEMENT REMOVAL LAYOUT IM-B 1902(67)0 B28 Plotting Date: 12/21/2022 W Blvd N WBlvdN N 2°16'21" E 8+00



Note: All curb and gutter shown on this sheet is Type B66 except as noted.

STATE OF SOUTH DAKOTA PROJECT
IM-B 1902(67)0

SHEET TOTAL SHEETS

B30 B63

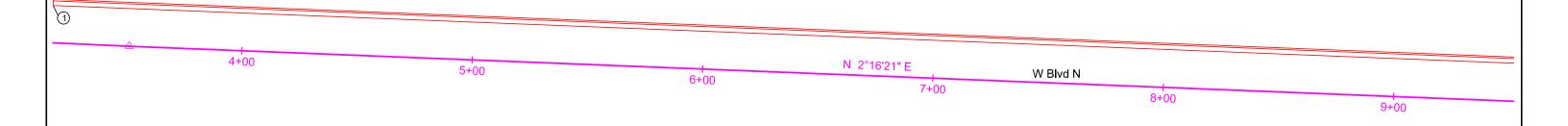
Plotting Date:



Plotting Date:



1 3+17.30-16.17' R Begin Str C & G TC Elev (Match Existing)



Note: All curb and gutter shown on this sheet is Type F66 except as noted.

STATE OF SOUTH DAKOTA PROJECT TOTAL SHEETS SHEET IM-B 1902(67)0 B31 B63

Plotting Date:

12/21/2022

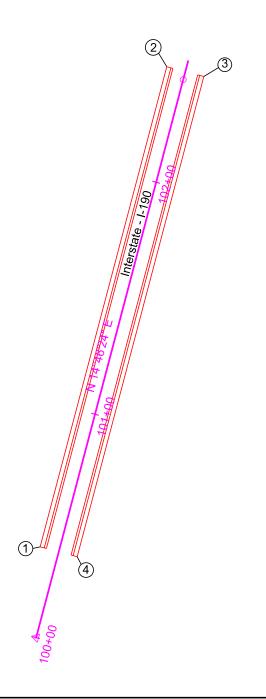


1 100+40.71-8.00' L Begin Str Type F66 C & G TC Elev (Match Existing)

2 102+47.72-8.00' L End Str Type F66 C & G TC Elev (Match Existing)

3 102+47.70-8.00' L Begin Str Type F66 C & G TC Elev (Match Existing)

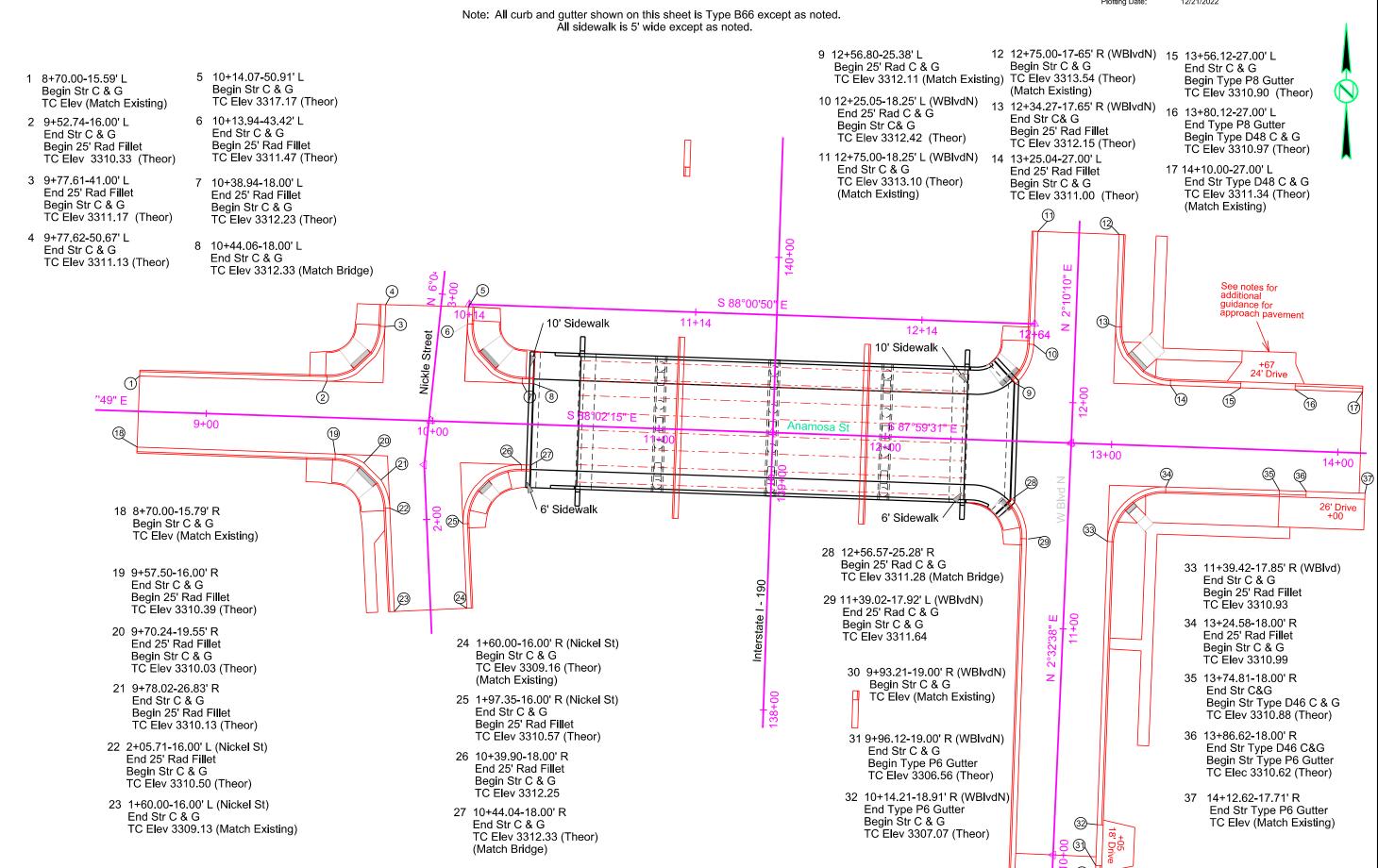
4 100+40.71-8.00' L End Str Type F66 C & G TC Elev (Match Existing)



PROJECT TOTAL SHEETS STATE OF SHEET IM-B 1902(67)0 B32 B63 DAKOTA

Plotting Date:

12/21/2022

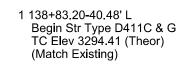


Plotting Date:

12/21/2022



142+00



2 139+63.35-40.28' L End Str Type D411 C & G TC Elev 3293.58 (Theor) (Match Existing)

- 3 140+34.44-40.55' L Begin Str Transition to Type P11 Gutter TC Elev 3294.90 (Theor) (Match Existing)
- 4 140+38.44-40.58' L End Str Transition to P11 Gutter Begin Str Type P11 Gutter TC Elev 3295.24 (Theor)
- 5 140+50.43-40.62' L End Str Type P11 Gutter TC Elev 3295.48 (Theor) (Match Existing)

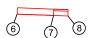
140+00

- 6 141+23.27-40.50' L Begin Str Type P11 Gutter TC Elev 3296.99 (Theor) (Match Existing)
- 7 141+38.44-40.46' L End Str Type P11 Gutter Begin Str Transition to Type F611 C & G TC Elev 3297.39 (Theor)
- 8 141+44.44-40.44' L End Str Transition to Type F611 C & G TC Elev 3297.89 (Theor) (Match Existing)



N 1°57'38" E

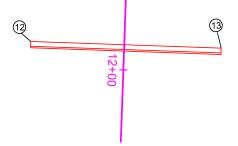
141+00







- 9 137+93.33-39.29' R Begin Str Type P11 Gutter TC Elev 3291.20 (Theor) (Match Existing)
- 10 138+05.79-39.26' R End Str Type P11 Gutter Begin Str Transition to Type D411 C & G TC Elev 3291.53 (Theor)
- 11 138+09.79-39.25' R End Str Transition to Type D411 C & G TC Elev 3291.63 (Theor) (Match Existing)



139+00

- 12 138+83.54-39.12' R
 Begin Str Type D411 C & G
 TC Elev (Theor)
 (Match Existing)
- 13 139+62.99-39.20' R End Str Type D411 C & G TC Elev 3293.47 (Theor) (Match Existing)

CURB RAMP LAYOUT

* Turning Space with 1.5% slope ** Curb Ramp with 7.5% slope and 1.5% cross slope
 STATE OF SOUTH DAKOTA
 PROJECT
 SHEET
 TOTAL SHEETS

 B34
 B63

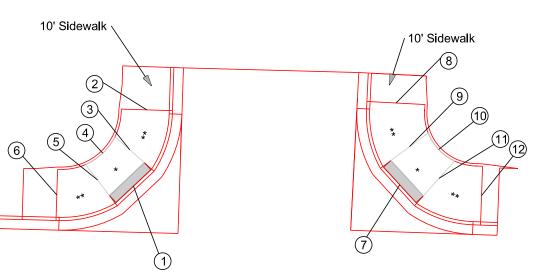
Plotting Date: 12/21/2022



1 9+68.00-25.68' L Center Type 3 Curb Ramp 4 9+61.38-32.25' L Back of Turning Space

2 9+70.28-41.67' L Begin Ramp Slope 5 9+60.49-26.72' L Begin Ramp Slope

3 9+66.97-33.25' L End Ramp Slope 6 9+52.10-23.34' L End Ramp Slope



7 10+23.42-27.80' L Center Type 3 Curb Ramp

8 10+22.08-44.83' L Begin Ramp Slope

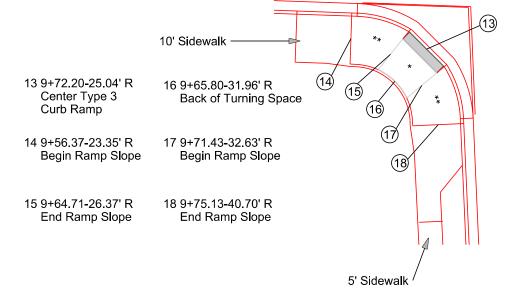
9 10+25.18-35.84' L 12

10 10+31.26-35.46' L Back of Turning Space

11 10+31.53-29.43' L Begin Ramp Slope

10+25.18-35.84' L 12 10+40.53-26.14' L End Ramp Slope End Ramp Slope

S 88°02'10" E Anamosa St 10+00 11+00



9 * 22 23 24 * 22 6' Sidewalk

19 10+23.65-27.99' R Center Type 3

Center Type 3 Back of Turning Space Curb Ramp

20 10+21.17-39.53' R Begin Ramp Slope 23 10+28.89-29.14' R Begin Ramp Slope

22 10+28.80-32.78' R

21 10+25.17-33.13' R End Ramp Slope

24 10+34.90-24.62' R End Ramp Slope

CURB RAMP LAYOUT

 STATE OF SOUTH DAKOTA
 PROJECT
 SHEET
 TOTAL SHEETS

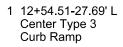
 B35
 B63

Plotting Date:

e: 12/21/2022

* Turning Space with 1.5% slope

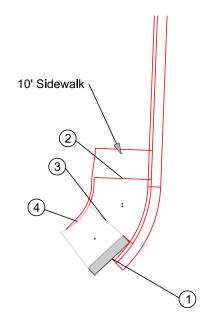
** Curb Ramp with 7.5% slope and 1.5% cross slope

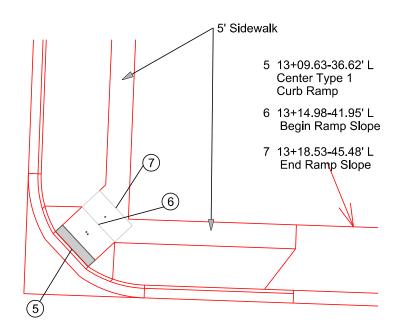


3 12+53.01-35.59' L End Ramp Slope

4 12+46.98-35.21' L Back of Turning Space

2 12+5615-44.49' L Begin Ramp Slope End Kamp Slope



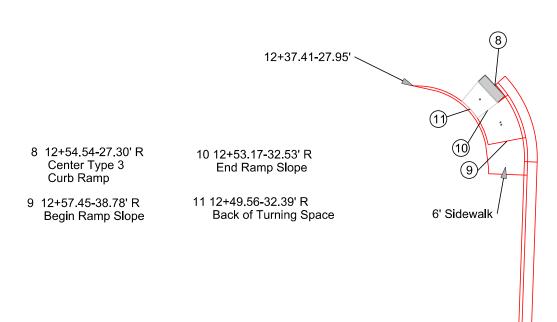


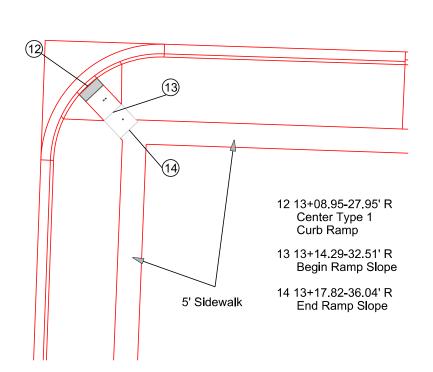
Anamosa St

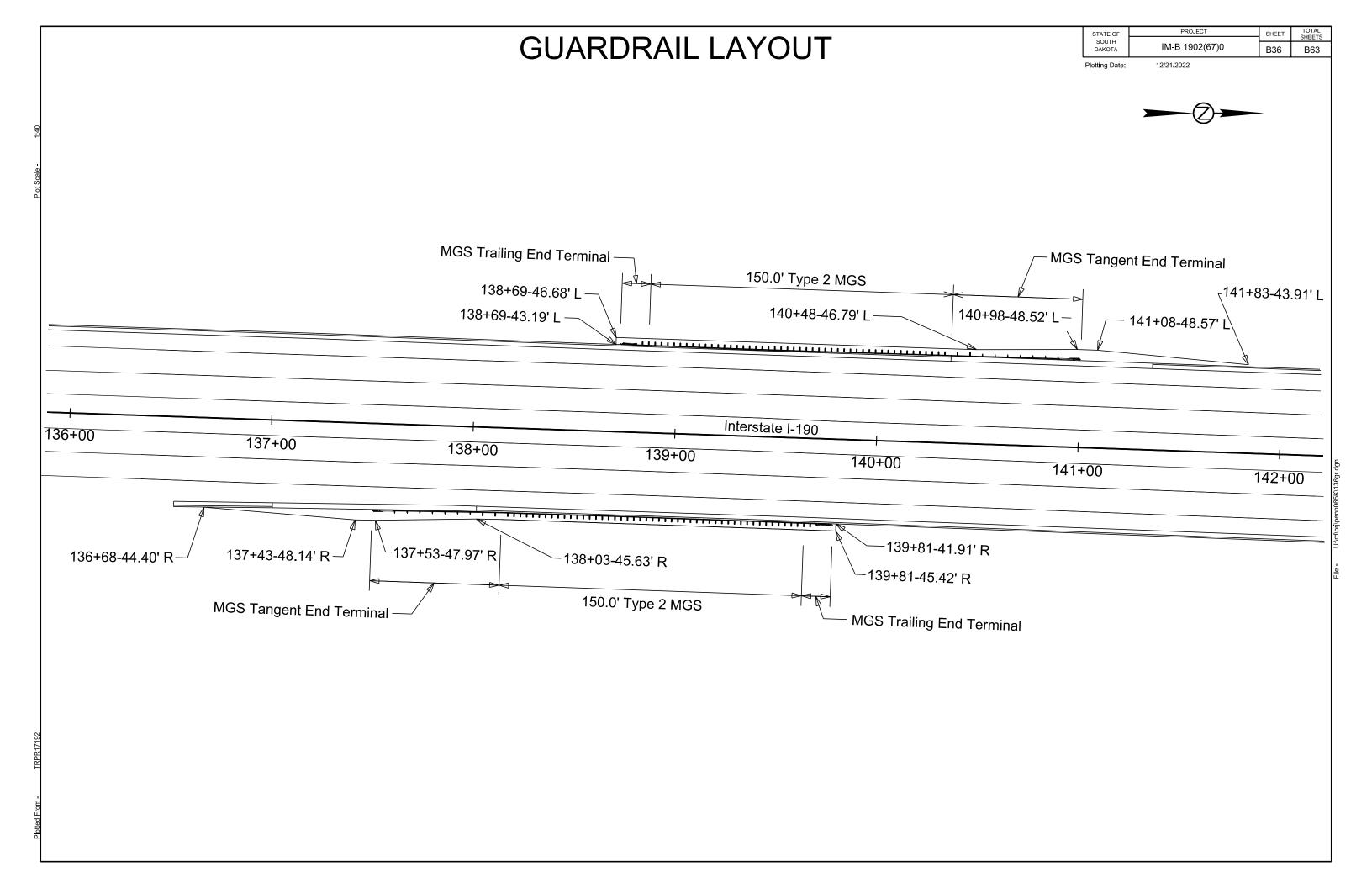
S 88°02'10" E

13+00

14+00







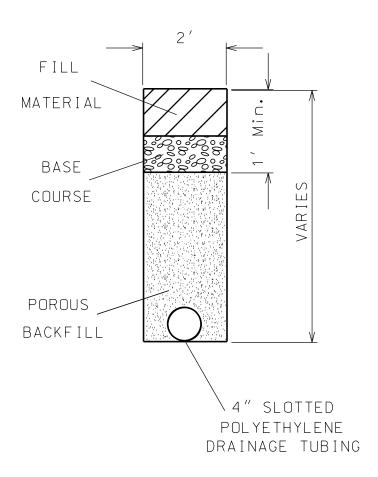
Plotting Date:

Date: 12/21/2022

TYPICAL UNDERDRAIN INSTALLATION

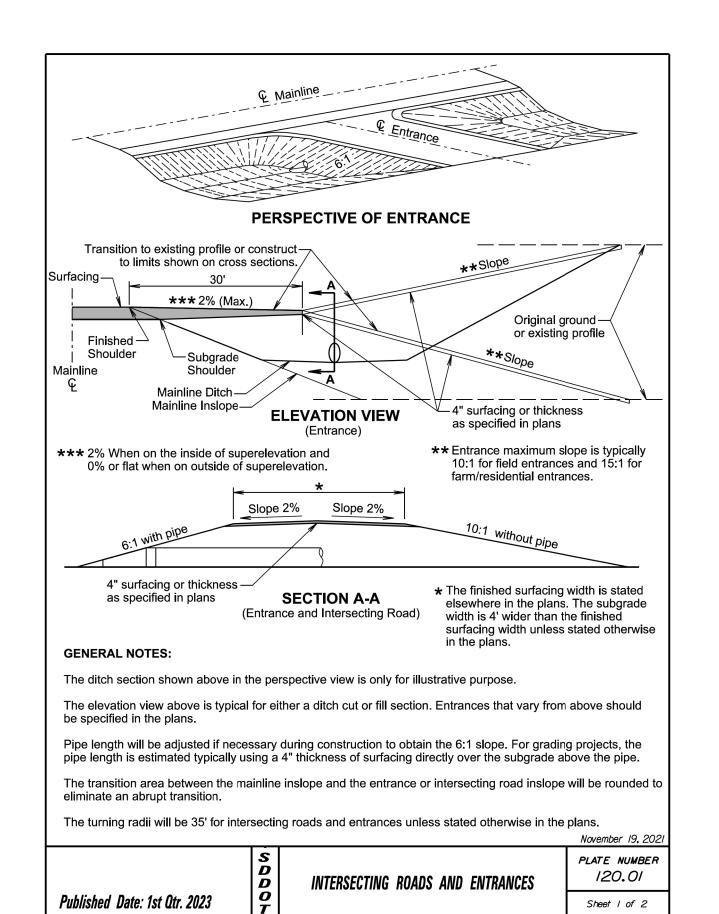
II90 REPLACEMENT UNDERDRAIN
ANAMOSA STREET
STATION II+05 41.5' LT. TO 36.5' RT.
STATION II+95 41.5' LT. TO 36.5' RT.

Longitudinal Drain Behind Curb



UNDERDRAINS SHALL BE CONSTRUCTED IN ACCORDANCE WITH SECTION 680 OF THE SPECIFICATIONS

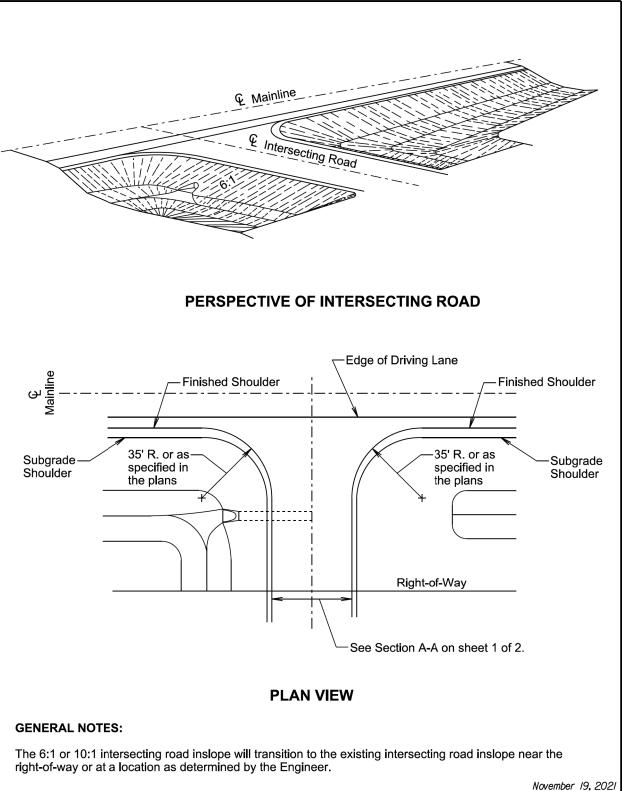
Published Date: 1st Qtr. 2023



Sheet I of 2

STATE OF PROJECT TOTAL SHEETS SHEET IM-B 1902(67)0 B38 B63 DAKOTA

Plotting Date: 04/04/2023



S D D O PLATE NUMBER 120.01 INTERSECTING ROADS AND ENTRANCES Published Date: 1st Qtr. 2023 Sheet 2 of 2

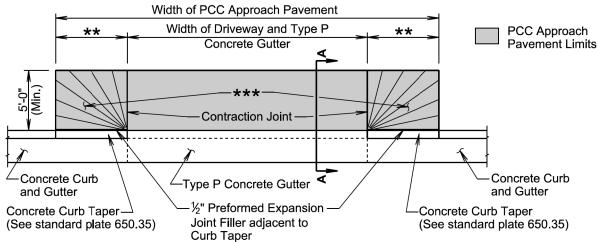
½" Preformed— **Expansion Joint** ←Concrete Curb and Gutter **GENERAL NOTES:** STATE OF PROJECT SHEET TOTAL SHEETS IM-B 1902(67)0 B39 B63 DAKOTA

04/04/2023

Plotting Date:

Sawed joint filled with hot-%" (Min.) -The minimum elevation of this point poured elastic joint sealer will be at the same elevation as the theoretical top of mainline curb elevation. 5'-0" (Min.) -Theoretical Top of Mainline Curb Elevation See Detail B <u>.</u>% 10% (Max.) **DETAIL B** ∠PCC Approach 4" Granular Pavement Material ∠Type P Concrete Gutter **SECTION A-A**

- * 8" at Commercial Approaches
- ** Width for 6" high curb is 6' (See standard plate 650.35)
- *** Within these areas, the surface of the type A PCC approach pavement will be sloped transitionally as approved by the Engineer.



The concrete for the type A PCC approach pavement and adjacent driveway will comply with the requirements of the Specifications for class M6 concrete unless otherwise stated in the plans.

PLAN VIEW

Contraction joints in the type A PCC approach pavement will be 1½ inches deep if formed in the fresh concrete using a suitable grooving tool. If a saw is used to cut the contraction joints, then the depth of the joint will be at least ¼ the thickness of the approach pavement. Additional contraction joints not shown in the Plan View will be spaced as follows:

> One joint at the center of the approach for driveways 16 feet to 24 feet wide. Two joints spaced at equal intervals for driveways greater than 24 feet to 40 feet wide.

All costs for furnishing and placing the type A PCC approach pavement and constructing the expansion and contraction joints including labor, equipment, excavation, and materials including the earthen backfill and granular material, will be incidental to the contract unit price per square yard for the corresponding PCC Approach Pavement contract item.

June 26, 2019

SDDO PLATE NUMBER TYPE A 380.40 PCC APPROACH PAVEMENT Published Date: 1st Qtr. 2023 Sheet I of I

PROJECT STATE OF SHEET TOTAL SHEETS SOUTH IM-B 1902(67)0 B40 B63

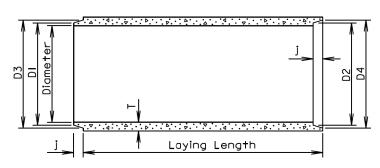
Plotting Date:

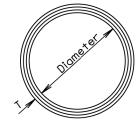
04/04/2023

TOLERANCES IN DIMENSIONS

Diameter: $\pm 1.5\%$ for 24" Dia. or less and $\pm 1\%$ or $\frac{3}{8}$ " whichever is more for 27" Dia. or greater. Diameters at joints: $\pm \frac{3}{6}$ " for 30" Dia. or less and $\pm \frac{1}{4}$ " for 36" or greater. Length of joint (j): $\pm \frac{1}{4}$ ".

Wall thickness (T): not less than design T by more than 5% or $\frac{3}{16}$ ", whichever is greater. Laying length: shall not underrun by more than $\frac{1}{2}$ ".





LONGITUDINAL SECTION

END VIEW

GENERAL NOTES:

Construction of R.C.P. shall conform to the requirements of Section 990 of the Specifications.

Not more than 2 four-foot sections shall be permitted near the ends of any culvert. Four-foot lengths shall be used only to secure the required length of culvert.

	Approx.						
Diam.	W+. /F+.		J ,	DI ,	D2	D3	D4
(in.)	(ID.)	(in.)	(in .)	(in.)	(in.)	(in.)	(in .)
12	92	2	13/4	131/4	135/8	131/8	141/4
15	127	21/4	2	161/2	16%	171/4	175/8
18	168	21/2	21/4	195/8	20	203/8	203/4
21	214	23/4	21/2	22 7/8	231/4	233/4	241/8
24	265	3	23/4	26	26¾	27	273/8
27	322	31/4	3	29 ¹ / ₄	295/8	30 ¹ / ₄	305/8
30	384	31/2	31/4	323/8	32¾	331/2	33%
36	524	4	3¾	38¾	39 ¹ / ₄	40	401/2
42	685	41/2	4	45 ¹ / ₈	45%	461/2	47
48	867	5	41/2	511/2	52	53	531/2
54	1070	51/2	41/2	57%	58 %	59¾	59%
60	1296	6	5	641/4	64¾	66	661/2
66	1542	61/2	51/2	70%	711/8	721/2	73
72	1810	7	6	77	771/2	79	791/2
78	2098	71/2	61/2	83%	83%	85%	861/8
84	2410	8	7	89¾	901/4	921/8	925/8
90	2740	81/2	7	95¾	961/4	981/8	985/8
96	2950	9	7	1021/8	1025/8	1041/2	105
102	3075	91/2	71/2	109	1091/2	1111/2	112
108	3870	10	71/2	1151/2	116	118	1181/2

June 26, 2015

PLATE NUMBER

450.01

Sheet I of I

S D D O T REINFORCED CONCRETE PIPE Published Date: 1st Qtr. 2023

	Laying Length at	Laying Length at						Radius of Curve	Weight of Section
D	Center of Pipe	Outside of Curve	Т	А	В	С	E		
(in.)	(i∩₌)	(in <u>.</u>)	(in.)	(in .)	(in .)	(in <u>.</u>)	(in.)	(f+ <u>.</u>)	(lbs.)
12	73/4	8	2	43/4	2	5¾	3	4.9	70
15	111/4	121/2	21/4	5 ¹ / ₄	43/4	6 ¹ / ₂	6	7.2	120
18	121/8	135⁄8	21/2	51/2	51/8	7	65%	7.7	170
21	91/2	111/4	2¾	51/2	21/4	71/4	4	6.1	170
24	913/16	113/4	3	5%	25/8	71/2	41/4	6.2	215
27	911/16	121/8	31/4	5 ½6	25/16	75/8	41/2	6.2	260
30	10	123/8	31/2	55/16	25/16	711/16	4"/16	6.4	320
33	113/6	137/8	3¾	5 ¹⁵ / ₆	2%	85/8	51/4	7.1	420
36	123/6	151/16	4	61/2	25/16	93/8	511/16	7.7	530
42	1 4½ ₆	171/2	41/2	6 ¹³ / ₁₆	313/16	10%	73/16	8.9	800
48	161/16	201/4	5	715/16	4"/16	113/4	81/2	10.5	1190
54	18½ ₆	225/16	51/2	75/8	63//6	11 7/8	101/6	11.5	1600
60	201/2	251/4	6	85/8	71/8	133/8	11 7/8	13.0	2210
66	215/8	26 ¹⁵ / ₁₆	61/2	9	73/8	I 45/16	125/8	13.8	2790
72	225/8	281/4	7	93/8	75/8	13 ¹ / ₄	15	14.4	3420

March 31, 2000

SDDOT REINFORCED CONCRETE PIPE SHORT RADIUS BEND Published Date: 1st Qtr. 2023

PLATE NUMBER 450.03

Sheet I of I

GENERAL NOTE:

(in.)

36¹⁵/₃₂

 $36\frac{1}{2}$

 $24\frac{1}{2}$

 $24\frac{1}{2}$

251/32

251/32

251/32

2415/16

 $24^{13}/_{16}$

2421/32

2419/32

245/8

 $24^{21}/_{32}$

2411/16

 $24\frac{1}{8}$

 $24^{1}/_{4}$

235/16

D

(in.)

12

15

18

21

24

27

30

33

36

42

48

54

60

66

72

84

96

Published Date: 1st Qtr. 2023

(in.)

2

 $2^{1}/_{4}$

 $2\frac{1}{2}$

23/4

3

 $3^{1}/_{4}$

 $3\frac{1}{2}$

33/4

4

 $4\frac{1}{2}$

5

 $5\frac{1}{2}$

6

 $6\frac{1}{2}$

7

8

9

Centerline laying length: 4'-0

(in.)

371/32

373/4

26

261/4

 $26^{31}/_{32}$

 $27\frac{7}{32}$

 $27^{15}/_{32}$

27%

27¹/₁₆

 $28\frac{1}{32}$

 $28^{13}/_{32}$

291/32

291/32

29¹³/₁₆

291/8

30¾

30¹¹/₁₆

Radius of Curve: 30.5

В

(in.)

 $10^{15}/_{32}$

101/4

22

213/4

211/32

 $20\frac{25}{32}$

 $20^{17/32}$

201/6

201/6

19²/₃₂

19¹⁹/₃₂

 $19^{1/8}$

 $18^{21}/_{32}$

183/16

 $18\frac{1}{8}$

17¹/4

175/₁₆

REINFORCED CONCRETE PIPE LONG RADIUS BEND PLATE NUMBER 450.04

March 31,2000

Weight of

Section

(lbs.)

368

508

672

856

1060

1288

1536

1808

2096

2740

3468

4280

5184

6168

7240

9640

12400

Ε

(in.)

11¹⁷/₃₂

 $11\frac{1}{2}$

 $23\frac{1}{2}$

 $23\frac{1}{2}$

 $22^{31}/_{32}$

 $22^{31}/_{32}$

 $22^{31}/_{32}$

 $23\frac{1}{16}$

231/6

 $23\frac{1}{32}$

 $23^{13}/_{32}$

233/8

2311/32

23 1/6

23 1/8

233/4

24¹¹/₁₆

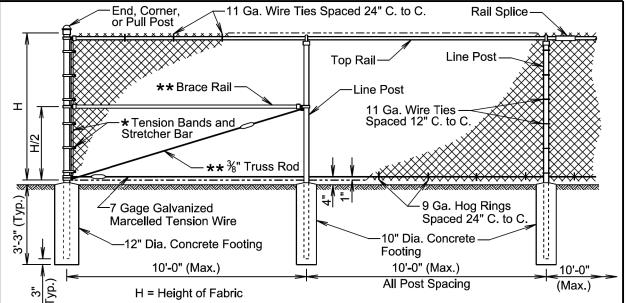
Sheet I of I

 STATE OF SOUTH DAKOTA
 PROJECT
 SHEET
 TOTAL SHEETS

 IM-B 1902(67)0
 B41
 B63

Plotting Date:

oate: 04/04/2023



* Tension bands will be spaced 12" c. to c.

** Are not required for 3' through 5' height fences.

Tightening device such as shown on standard plate 621.03

COMPONENT	END, CORNER, and PULL POST			LINE POST			TOP and BRACE RAIL	
Type of Fabrication	Round Pipe Nominal	Roll Formed Steel	Round Pipe Nominal	"C" Section	H Beam Steel	Round Pipe Nominal	Roll Formed Steel	
Size	3.00" O. D.	3.5"x3.5"	2.50" O. D.	1.875"x1.625"	2.25"x1.70"	1.625" O. D.	1.625"x1.25"	
Weight (lb. / Ft.)	5.79 or 4.64	5.14	3.65 or 3.12	2.34	3.43	2.27 or 1.84	1.35	

GENERAL NOTES:

Published Date: 1st Qtr. 2023

Specific details of the component parts of the fence will be approved by the Engineer. Commercially available items produced specifically for the use intended will be used wherever possible in the construction of the fence.

Height of the fabric will be as shown in the plans. Fabric is available at the following heights: 36", 42", 48", 60", 72", 84", 96", 108", 120", and 144". Fabric heights 60 inches and less will be knuckled at both selvages. Fabric heights 72 inches and higher will be knuckled at one selvage and twisted at the other selvage.

Chain link fabric will be 2-inch mesh, No. 9 gage galvanized wire securely fastened to tension wire, line post, rails, braces, and stretcher bars.

Fence may be constructed with either round pipe, "C" section, "H" beam, or roll formed steel components as shown in the table above. Line posts may be round pipe, "C" section, or "H" beam. The corner post and rails will be either round pipe or roll formed steel. The type of components used must be approved by the Engineer prior to installation.

Where fence must cross small bodies of water such as drainage areas or ponds that could freeze during the winter, use 11 gage hog rings. Provide only two ties per tension wire and top rail between line posts.

A suitable method of rail splicing will be used to allow for expansion and contraction while maintaining proper position of the top rail.

Fence grounding will be as shown on standard plate 620.11.

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November 19, 2022

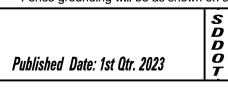
CHAIN LINK FENCE WITH TOP RAIL

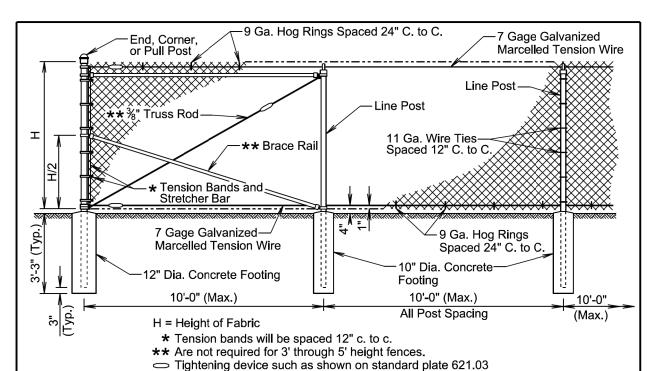
PLATE NUMBER
621.01

Sheet 1 of 1

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COMPONENT	END, CORNER, and PULL POST			LINE POST			BRACE RAIL	
Type of Fabrication	Round Pipe Nominal	Roll Formed Steel	Round Pipe Nominal	"C" Section	H Beam Steel	Round Pipe Nominal	Roll Formed Steel	
Size	3.00" O. D.	3.5"x3.5"	2.50" O. D.	1.875"x1.625"	2.25"x1.70"	1.625" O. D.	1.625"x1.25"	
Weight (lb. / Ft.)	5.79 or 4.64	5.14	3.65 or 3.12	2.34	3.43	2.27 or 1.84	1.35	

GENERAL NOTES:

Specific details of the component parts of the fence will be approved by the Engineer. Commercially available items produced specifically for the use intended will be used wherever possible in the construction of the fence.

Height of the fabric will be as shown in the plans. Fabric is available at the following heights: 36", 42", 48", 60", 72", 84", 96", 108", 120", and 144". Fabric heights 60 inches and less will be knuckled at both selvages. Fabric heights 72 inches and higher will be knuckled at one selvage and twisted at the other selvage.

Chain link fabric will be 2-inch mesh, No. 9 gage galvanized wire securely fastened to tension wire, line post, rails, braces, and stretcher bars.

Fence may be constructed with either round pipe, "C" section, "H" beam, or roll formed steel components as shown in the table above. Line posts may be round pipe, "C" section, or "H" beam. The corner post and rails will be either round pipe or roll formed steel. The type of components used must be approved by the Engineer prior to installation.

All posts will have a means to securely hold the top tension wire in position and allow for the removal and replacement of a post without damaging the top tension wire.

Where fence must cross small bodies of water such as drainage areas or ponds that could freeze during the winter, use 11 gage hog rings. Provide only two ties per tension wire between line posts.

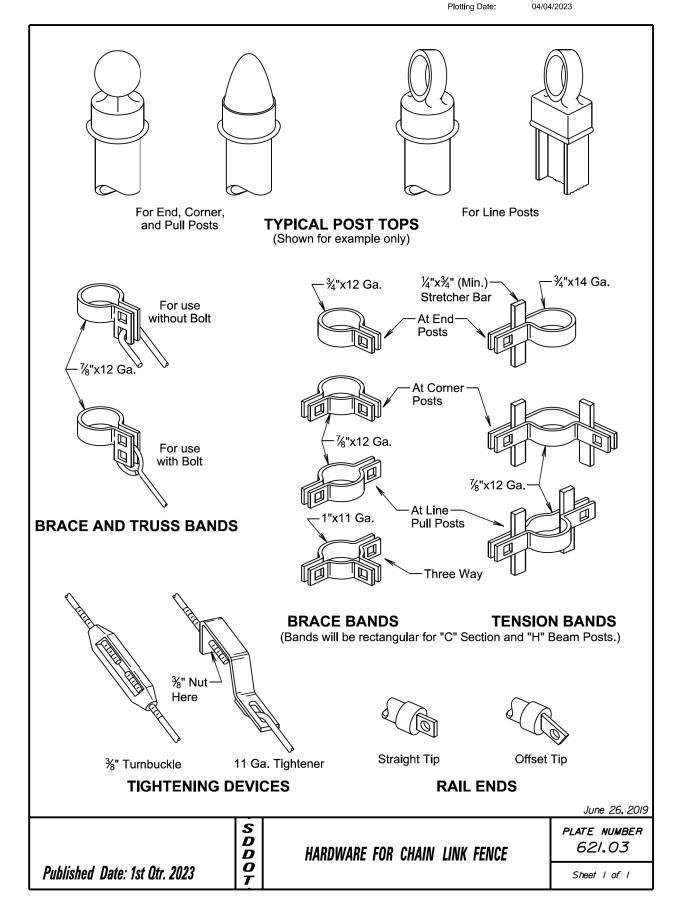
Fence grounding will be as shown on standard plate 620.11.

November 19, 2022

PLATE NUMBER

 STATE OF SOUTH DAKOTA
 PROJECT
 SHEET
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 B42
 B63



	TYPE AND DETAILS OF MGS								
Type of MGS	W Beam Rail Single or Double (Nested)	Blockout Size	Blockout Material		Post Material	Post Spacing			
1	Single	6"x12"x14"	Wood	6"x8"x6'-0"	Wood	6'-3"			
1C	Single	6"x12"x14"	Wood	6"x8"x7'-6"	Wood	6'-3"			
2	Single	6"x12"x14"	Wood	6"x8"x6'-0"	Wood	3'-1½"			
3	Single	6"x12"x14"	Wood	6"x8"x6'-0"	Wood	1'-6¾"			
4	Double	6"x12"x14"	Wood	6"x8"x6'-0"	Wood	6'-3"			

STANDARD PLATE REFERENCE						
Type of MGS	See Standard Plate(s)					
1	630.20, 630.22					
1C	630.20, 630.25					
2	630.20					
3	630.20					
4	630.20					

GENERAL NOTES:

Asphalt concrete will be the same type used elsewhere on the project or will be as specified in the plans. If asphalt concrete is not specified in the plans, the asphalt concrete will conform to the Specifications for "Asphalt Concrete Composite".

Granular material will be the same type used elsewhere on the project or will be as specified in the plans. If granular material type is not specified in the plans, the material will conform to the Specifications for "Base Course". The granular material will be placed the same thickness as the mainline surfacing or as specified in the plans.

Topsoil is not shown in the transverse section drawing on sheet 2 of 6.

All W beam rail will be Type 1 and Class A (12 Ga.) unless specified otherwise in the plans.

W beam rail section lengths may be 12'-6" and/or 25'-0". The combination of section lengths used will be compatible with the total length of rail per site as shown in the plans.

Slots in the rails will be provided as specified in the plans and by the manufacturer. A drilled hole through the rail is not allowed as a replacement for a slot. If the Contractor must create a slot, a cutting torch or plasma cutter is not allowed. The slot edges will be smooth and free of burrs or notches.

All costs for constructing the MGS including labor, equipment, and materials including all posts, blockouts, steel beam rail, and hardware will be incidental to the contract unit price per foot for the respective MGS contract item.

September 14, 2019

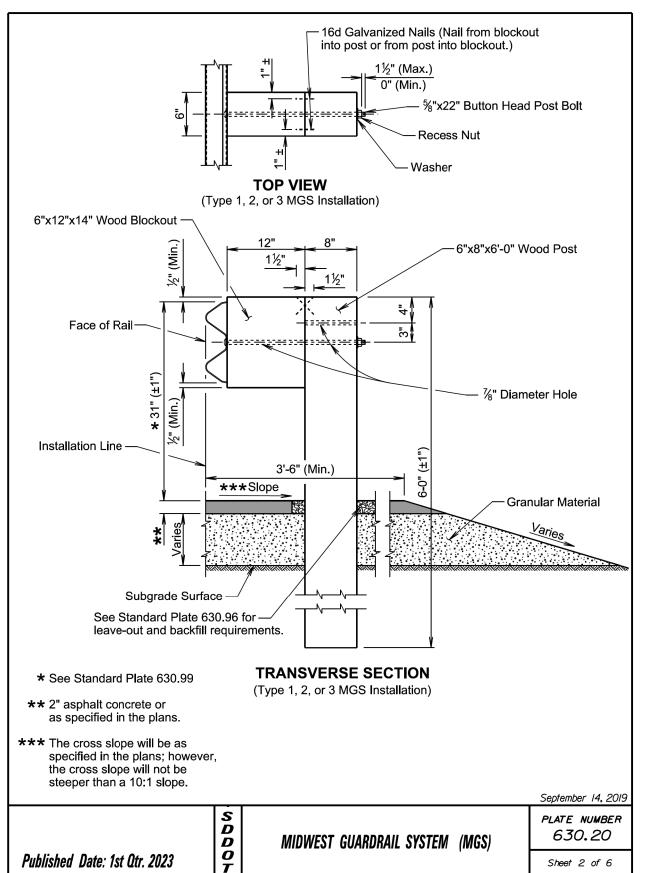
Published Date: 1st Qtr. 2023

| Sheet | of 6

 STATE OF SOUTH DAKOTA
 PROJECT
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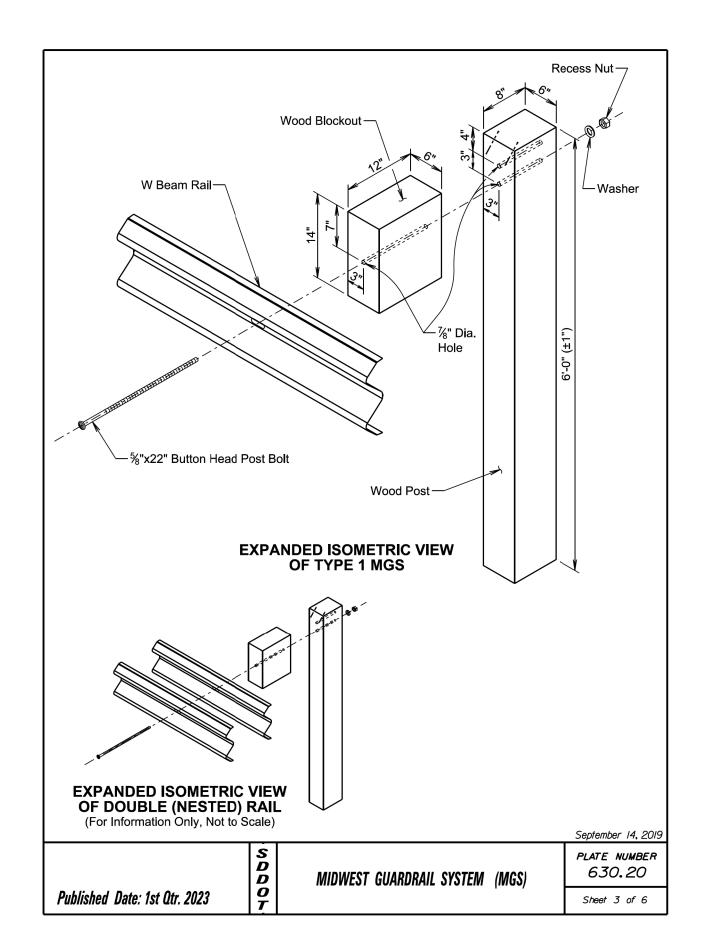
 B43
 B63

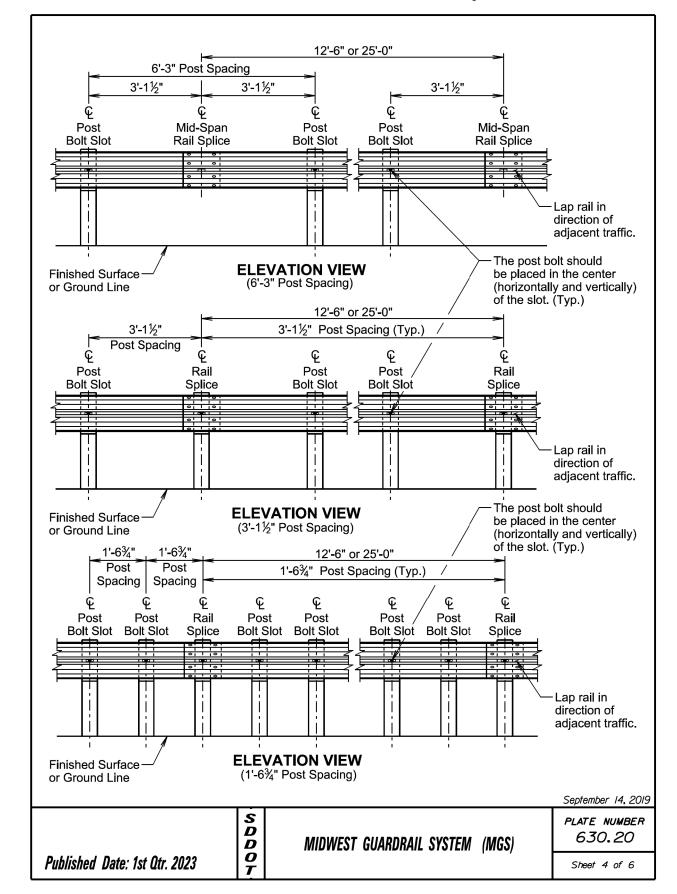
Plotting Date: 04/04/2023



Plotting Date:

Date: 04/04/2023





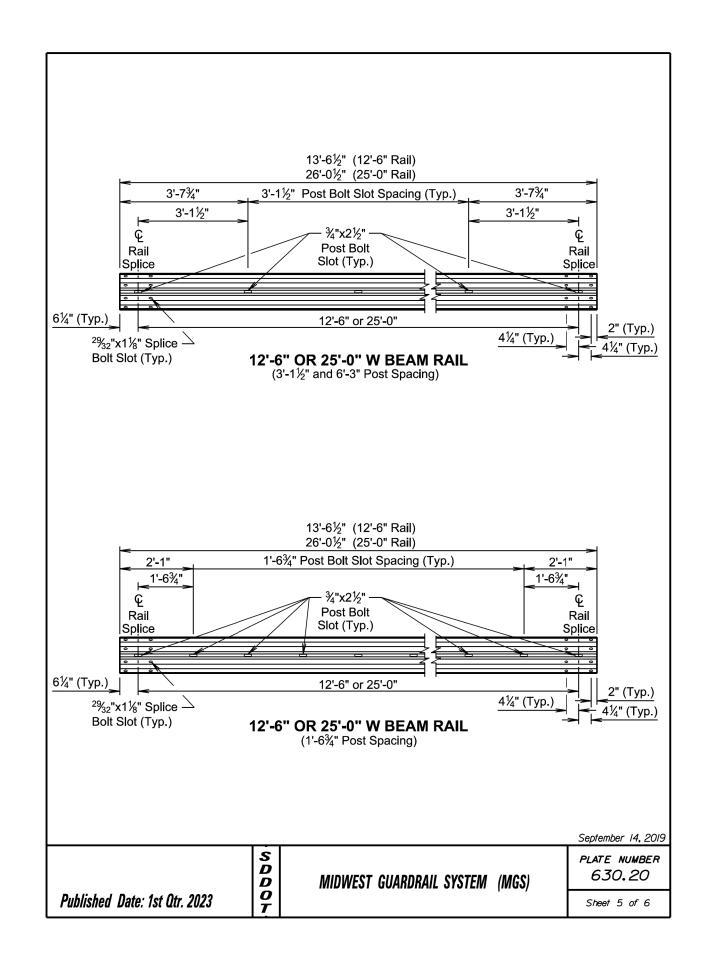
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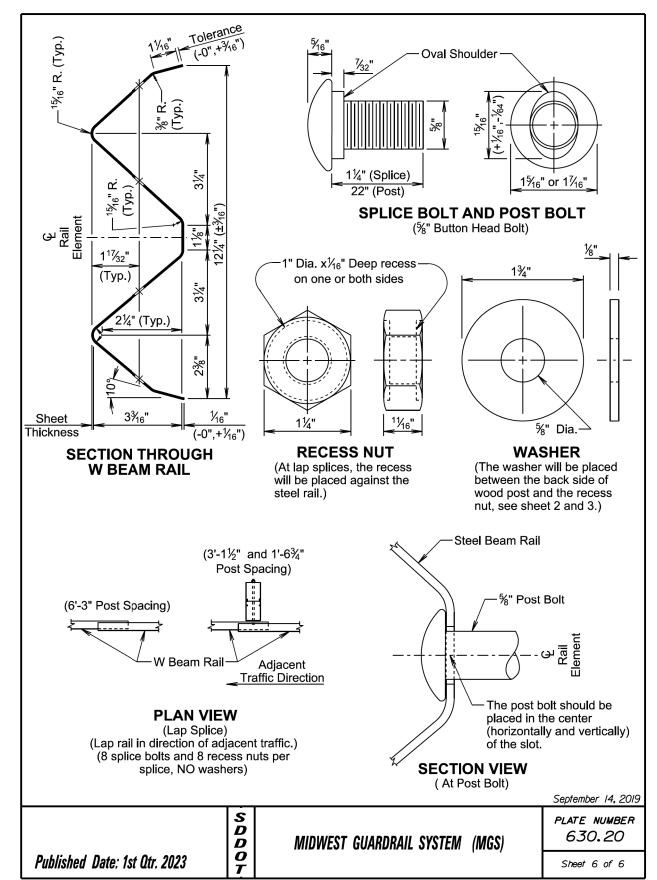
 STATE OF SOUTH DAKOTA
 PROJECT IM-B 1902(67)0
 SHEET SHEETS
 TOTAL SHEETS

 B45
 B63

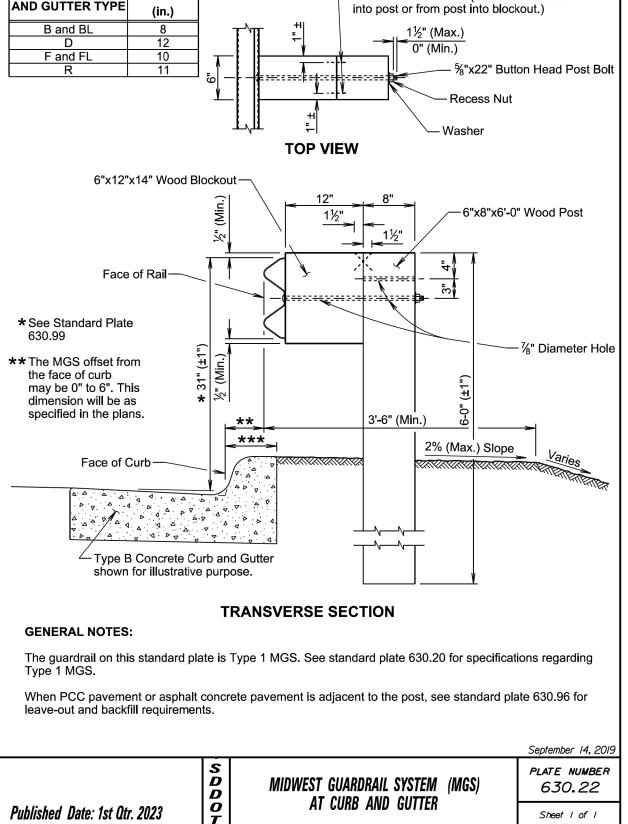
Plotting Date:

Date: 04/04/2023





T-0004



16d Galvanized Nails (Nail from blockout

DIMENSION

CONCRETE CURB

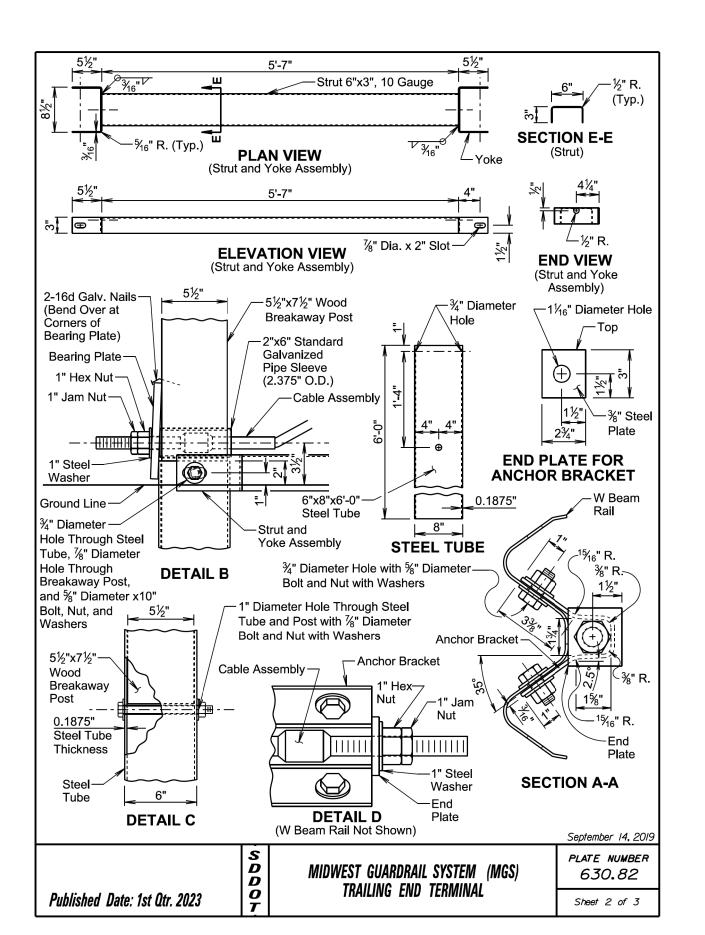
AND GUTTER TYPE

PROJECT STATE OF SHEET TOTAL SHEETS IM-B 1902(67)0 B46 B63 DAKOTA

04/04/2023

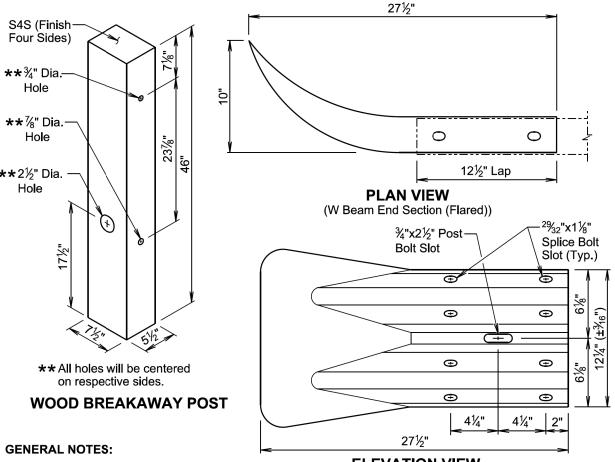
Plotting Date:

standard plate 630.99 ***** 31" Guardrail as specified in the plans. 6"x8"x6'-0" Wood 6"x8"x6'-0" Wood -Finished Surface or Ground Line s swage connected. Diameter (6x19) Galvanized Cable - 6"x12"x14" Wood Blockout One-Way Traffic Directi 6"x12"x14" Wood Blockout 3'-1½" ndard Swaged Fitting and Stud CABLE ASSEMBLY %"x22" Button— Head Post Bolt **7** 7 ... 2 of 3) -Wood Breakaway Post Wood Breakaway Post will be 3%" [Detail D (Sheet 3'-1½" "MGS Trailing End Terminal" **ELEVATION VIEW LAN VIEW** $-1^{1}\%_{16}$ " Anchor Bracket 0/0 Yoke Assembly Cable Assembly (Tight) See Detail C (Sheet 2 of 3) See Detail B (Sheet 2 of 3) Pay Limit for and Nut $1\%_{6}$ " Diameter Hole Bolt Cable Assembly Strut and Steel Plate am End Section (Flared) Wood Breakaway Post **BEARING PLATE** Post Breakaway 3"_ Lap rail in — direction of adjacent traffic. Bearing Plate 6"x8"x6'-0"-Steel Tube September 14, 2019 SDDO PLATE NUMBER MIDWEST GUARDRAIL SYSTEM (MGS) 630.82 TRAILING END TERMINAL Published Date: 1st Qtr. 2023 Sheet I of 3



PROJECT TOTAL SHEETS STATE OF SHEET IM-B 1902(67)0 B47 B63 DAKOTA

Plotting Date: 04/04/2023



The MGS trailing end terminal will only be used in a

ELEVATION VIEW (W Beam End Section (Flared))

W beam end section (flared) will be 12 gauge.

one-way traffic situation on the downstream traffic flow end.

The cable will be ½", Type II, with Class A coating in conformance with AASHTO M30.

The steel tube will meet the requirements of ASTM A500, Grade B, and will be galvanized after fabrication in accordance with the requirements of AASHTO M111.

All hardware will be galvanized in accordance with ASTM A153.

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The anchor bracket, strut and yoke assembly, and bearing plate will be fabricated from steel that meets ASTM A36 Specifications. They will be galvanized after fabrication in accordance with ASTM A123.

Slots in the rails will be provided as specified in the plans and by the manufacturer. A drilled hole through the rail is not allowed as a replacement for a slot. If the Contractor must create a slot, a cutting torch or plasma cutter is not allowed. The slot edges will be smooth and free of burrs or notches.

All costs for furnishing and constructing the MGS trailing end terminal including labor, equipment, materials which includes W beam rail section, two wood breakaway posts, steel tubes, strut and yoke assembly, cable assembly, bearing plate, anchor bracket, W beam end section (flared), one MGS wood post and blockout, hardware, and incidentals will be included in the contract unit price per each for "MGS Trailing End Terminal". September 14, 2019

Published Date: 1st Qtr. 2023

MIDWEST GUARDRAIL SYSTEM (MGS) TRAILING END TERMINAL

PLATE NUMBER 630.82

Sheet 3 of 3

 \odot \odot * Inslope transition (If necessary) (If necessary) 15 specified in the plans. Slope will not be steeper than a 10:1 slope. 5' (Min.) 5' (Min.) specified in the plans. -(4) -(4) 0 5' (Min.) 5' (Min.) 0 Shown) Suardrail (Guardrail Not Flared) (MSKT-SP-MGS MASH Tangent End Terminal Shown) ardrail (Guardrail Not Flared)
(SoftStop MGS MASH Tangent End Terminal ö material Length of Flared Embankment MGS MASH Tangent End Terminal Pay Limits MGS MASH Tangent End Terminal Pay Limits Provide and install same hardware as Type 1 MGS. thickness granular Same inslope as mainline inslope or as specified in the plans. Provide and install same hardware φ Finished Edge of Surfacing slope or as Surf specified in the plans of Guardrail Installation Line of Non-Flared Guardrail Edge Inslope as specified in the plans. slope as roadway cross nstallation Line of Non-Flared ② 4:1 inslope or as of Sice Center Lap Spl -Type 1 MGS Pay Limits -Type 1 MGS Pay Limits (9) (e) Same \odot (e) 4 3'-6" 3'-6" November 19, 2021 S D D PLATE NUMBER EMBANKMENT, SURFACING, AND PAYMENT 630.89 LIMITS FOR MGS MASH TANGENT END TERMINAL 0 Published Date: 1st Qtr. 2023 Sheet I of 2

PROJECT TOTAL SHEETS STATE OF SHEET IM-B 1902(67)0 B48 B63 DAKOTA

04/04/2023

Plotting Date:

* Inslope transition (If necessary) 3 (1) 2 50' Length of Flared Embankment 10' Type 1 MGS -MGS MASH Tangent End Terminal Pay Limits Pay Limits Finished Edge of Surfacing-12'-6" Center of Optional Flared Embankment 15 প∖Lap Splice 0 4 (Min.) -Provide and install same hardware as Type 1 MGS. Min. Installation Line of ū Flared Guardrail $\bar{\Omega}$ **PLAN VIEW** (Guardrail Flared) (SoftStop MGS MASH Tangent End Terminal Shown) * Inslope transition (If necessary) 3 1 50' Length of Flared Embankment 10' 、 Type 1 MGS-MGS MASH Tangent End Terminal Pay Limits Pay Limits Finished Edge of Surfacing 12'-6" Center of -Optional Flared Embankment 15 ৃ Lap Splice (Min.) Provide and install same hardware as Type 1 MGS. Installation Line of Flared Guardrail $\bar{\Omega}$ 2 **PLAN VIEW** (Guardrail Flared) (MSKT-SP-MGS MASH Tangent End Terminal Shown) **GENERAL NOTES:** The MGS MASH tangent end terminals above are for illustrative purpose only. Pay limit length of the MGS

MASH tangent end terminal is 62'-6".

- ★ The length of inslope transition varies with the amount of change between inslopes. The length of the transition will change 100' for every whole number change in the inslope. For Example: If the inslope changes from a 5:1 to a 4:1 the length of the inslope transition would be 100'. If the inslope changes from a 6:1 to a 4:1 the length of the inslope transition would be 200'.
- 🐞 The installation reference line for MGS MASH tangent end terminals will always be parallel to the roadway.

Asphalt concrete will be the same type used elsewhere on the project or will be as specified in the plans. If asphalt concrete is not specified in the plans, the asphalt concrete will conform to the Specifications for "Asphalt Concrete Composite."

Granular material will be the same type used elsewhere on the project or will be as specified in the plans. If granular material type is not specified in the plans, the material will conform to the Specifications for "Base Course". The granular material will be placed the same thickness as the mainline surfacing or as specified in the plans. November 19, 2021

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PLATE NUMBER

Published Date: 1st Qtr. 2023

EMBANKMENT, SURFACING, AND PAYMENT LIMITS FOR MGS MASH TANGENT END TERMINAL

630.89 Sheet 2 of 2

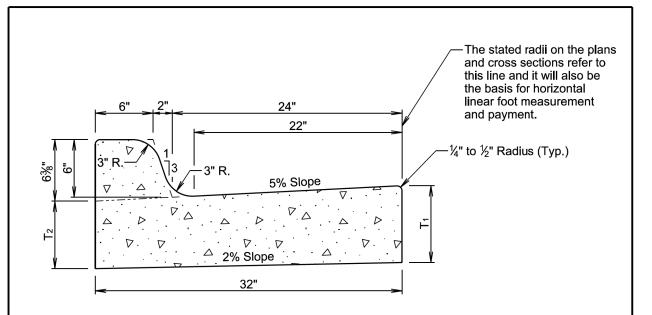
PROJECT SHEET IM-B 1902(67)0 B49

TOTAL SHEETS

B63

Plotting Date:

04/04/2023



TYPE B	TYPE B CONCRETE CURB AND GUTTER							
Туре	T ₁ (Inches)	T ₂ (Inches)	Cu. Yd. Per Lin. Ft.	Lin. Ft. Per Cu. Yd.				
B66	6	51/ ₁₆	0.057	17.7				
B67	7	6¼ ₆	0.065	15.4				
B68	8	7½ ₆	0.073	13.7				
B68.5	8.5	7 % ₁₆	0.077	13.0				
B69	9	81/16	0.081	12.3				
B69.5	9.5	8%6	0.085	11.7				
B610	10	91/16	0.090	11.2				
B610.5	10.5	9%6	0.094	10.7				
B611	11	101/16	0.098	10.2				
B611.5	11.5	10%6	0.102	9.8				
B612	12	111/16	0.106	9.4				

GENERAL NOTES:

Published Date: 1st Qtr. 2023

When concrete curb and gutter longitudinally adjoins new concrete pavement, the method of attachment will be by one of the methods shown on standard plate 380.11.

See standard plate 650.90 for expansion and contraction joints in the curb and gutter.

S D D O T

December 23, 2019

TYPE B CONCRETE CURB AND GUTTER

Sheet I of I

PLATE NUMBER 650.01

The stated radii on the plans and cross sections refer to this line and it will also be the basis for horizontal linear foot measurement 20" and payment. $-\frac{1}{4}$ " to $\frac{1}{2}$ " Radius (Typ.) −3" R. 4.17% Slope (½" per Ft.) 32"

TYPE D CONCRETE CURB AND GUTTER							
Туре	T ₁ (Inches)	T ₂ (Inches)	Cu. Yd. Per Lin. Ft.	Lin. Ft. Per Cu. Yd.			
D46	6	5 ⁵ ⁄ ₁₆	0.056	18.0			
D47	7	6¾ ₆	0.064	15.7			
D48	8	75⁄ ₁₆	0.072	13.9			
D48.5	8.5	7 ¹³ ⁄ ₁₆	0.076	13.1			
D49	9	8 5⁄16	0.080	12.5			
D49.5	9.5	8 ¹³ / ₁₆	0.084	11.9			
D410	10	95⁄ ₁₆	0.088	11.3			
D410.5	10.5	9 ¹ ¾ ₁₆	0.093	10.8			
D411	11	10 ⁵ ⁄ ₁₆	0.097	10.3			
D411.5	11.5	10 ¹³ / ₁₆	0.101	9.9			
D412	12	11½ ₆	0.105	9.5			

GENERAL NOTES:

Published Date: 1st Qtr. 2023

When concrete curb and gutter longitudinally adjoins new concrete pavement, the method of attachment will be by one of the methods shown on standard plate 380.11.

See standard plate 650.90 for expansion and contraction joints in the curb and gutter.

S D D O

December 23, 2019

PLATE NUMBER *650.15* TYPE D CONCRETE CURB AND GUTTER

Sheet I of I

The stated radii on the plans and cross sections refer to this line and it will also be the basis for horizontal linear foot measurement and payment. $\frac{1}{4}$ " to $\frac{1}{2}$ " Radius (Typ.)-TYPE P CONCRETE GUTTER Cu. Yd. Lin. Ft. T₂ T1 Per Type (Inches) (Inches) 8.33% Slope 5% Slope

. . D

32" TRANSVERSE SECTION

2% Slope

D.

Approach and/or Type P Concrete Gutter Limits Driveway

Lin. Ft. Cu. Yd. P6 6 6%0.047 21.2 P7 7% 0.055 18.1 P8 8 8% 0.064 15.7 P8.5 8.5 8% 0.068 14.8 P9 9 9% 0.072 13.9 P9.5 9.5 9%0.076 13.2 P10 10 10% 0.080 12.5 P10.5 10.5 10% 0.084 11.9 P11 11 11% 0.088 11.3 11% P11.5 11.5 0.092 10.8 P12 12 12% 0.096 10.4

Width of Type P Concrete Gutter Payment limits for Payment limits for Concrete Curb and Concrete Curb and Gutter Line Gutter Gutter Concrete Curb Taper * ½" Preformed-½" Preformed * (See Standard Plate 650.35) Expansion Joint Filler **Expansion Joint Filler**

PLAN VIEW

*Joint will not be needed if concrete curb and gutter and type P concrete gutter is placed at the same time. If the $\frac{1}{2}$ " preformed expansion joint filler is provided, then the joint will be sealed in accordance with standard plate 650.90.

GENERAL NOTES:

The concrete for the type P concrete gutter will comply with the requirements of the specifications for class M6 concrete.

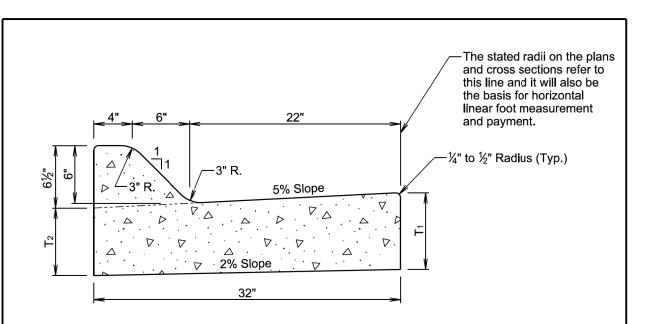
When concrete gutter longitudinally adjoins new concrete pavement, the method of attachment will be by one of the methods shown on standard plate 380.11.

Transverse contraction joints will be constructed at 10-foot intervals in the concrete gutter except when concrete gutter is constructed adjacent to mainline PCC pavement. When concrete gutter is constructed adjacent to mainline PCC pavement, a transverse contraction joint will be constructed in the concrete gutter at each mainline PCC pavement transverse contraction joint location.

When concrete gutter is placed monolithically with mainline PCC pavement, the transverse contraction joints in the concrete gutter will be sawed and sealed the same as the transverse contraction joints in the mainline PCC pavement.

When concrete gutter is not placed monolithically with the mainline PCC pavement and when the adjacent mainline surfacing is not PCC concrete, the transverse contraction joints in the concrete gutter will be 11/3 inches deep if formed in the fresh concrete using a suitable grooving tool. If a saw is used to cut the contraction joints, then the depth of the joint will be at least ¼ the thickness of the concrete. December 23, 2019

PLATE NUMBER D *650.30* D TYPE P CONCRETE GUTTER 0 Published Date: 1st Qtr. 2023 Sheet I of I



TYPE F CONCRETE CURB AND GUTTER								
Туре	T ₁ (Inches)	T ₂ (Inches)	Cu. Yd. Per Lin. Ft.	Lin. Ft. Per Cu. Yd.				
F66	6	5½ ₆	0.057	17.6				
F67	7	61⁄ ₁₆	0.065	15.4				
F68	8	71⁄16	0.073	13.6				
F68.5	8.5	7% ₁₆	0.077	12.9				
F69	9	81/16	0.082	12.3				
F69.5	9.5	8% ₁₆	0.086	11.7				
F610	10	91/16	0.090	11.1				
F610.5	10.5	9%6	0.094	10.7				
F611	11	10¼ ₆	0.098	10.2				
F611.5	11.5	10%6	0.102	9.8				
F612	12	111/16	0.106	9.4				

GENERAL NOTES:

When concrete curb and gutter longitudinally adjoins new concrete pavement, the method of attachment will be by one of the methods shown on standard plate 380.11.

See standard plate 650.90 for expansion and contraction joints in the curb and gutter.

D D O

December 23, 2019

PLATE NUMBER 650.20

Published Date: 1st Qtr. 2023

TYPE F CONCRETE CURB AND GUTTER

Sheet I of I

D D O T

SECTION C-C

1/8" to 1/4"

* Low Modulus

SECTIONAL VIEW

(Curb and Gutter Placed Monolithic with Adjacent Mainline PCC Pavement)

Bottom of Sawed Joint

* Low Modulus

SECTIONAL VIEW

(Curb and Gutter not Placed Monolithic with Adjacent Mainline PCC Pavement or Mainline

Surfacing is not PCC Pavement)

* Low Modulus-

Silicone Sealant

Silicone Sealant

Bottom of

Sawed Joint

Silicone Sealant

Sawed Joint Filled with Hot

Poured Elastic Joint Sealer

- * Low Modulus

SECTION A-A

Sawed Joint Filled with Hot

Poured Elastic Joint Sealer

SECTION B-B

*The silicone sealant will be placed such

that it completely seals the joint and is

approved by the Engineer.

bonded to the sides of the clean joint as

⅓" to ¼"

Silicone Sealant 1/8" to 1/4"

JOINTS IN CONCRETE CURB AND GUTTER

PLATE NUMBER 650.90

December 23, 2019

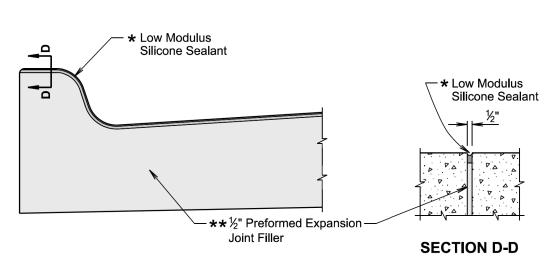
Published Date: 1st Qtr. 2023

Sheet I of 2

STATE OF PROJECT TOTAL SHEETS SHEET IM-B 1902(67)0 B51 B63 DAKOTA

Plotting Date:

04/04/2023



SECTIONAL VIEW

(Curb and Gutter at ½" Preformed Expansion Joint Filler Location)

> ★ The silicone sealant will be placed such that it completely seals the joint and is bonded to the sides of the clean joint as approved by the Engineer.

GENERAL NOTES:

For illustrative reason, only the type B curb and gutter is shown.

** A ½-inch preformed expansion joint filler will be placed transversely in the curb and gutter at the following

At each junction between the radius return of curb and gutter, and curb and gutter which is parallel to the project centerline.

At each junction between new curb and gutter and existing curb and gutter.

Transverse contraction joints will be constructed at 10 foot intervals in the concrete curb and gutter except when the concrete curb and gutter is constructed adjacent to mainline PCC pavement. When concrete curb and gutter is constructed adjacent to mainline PCC pavement, a transverse contraction joint will be constructed in the concrete curb and gutter at each mainline PCC pavement transverse contraction joint location.

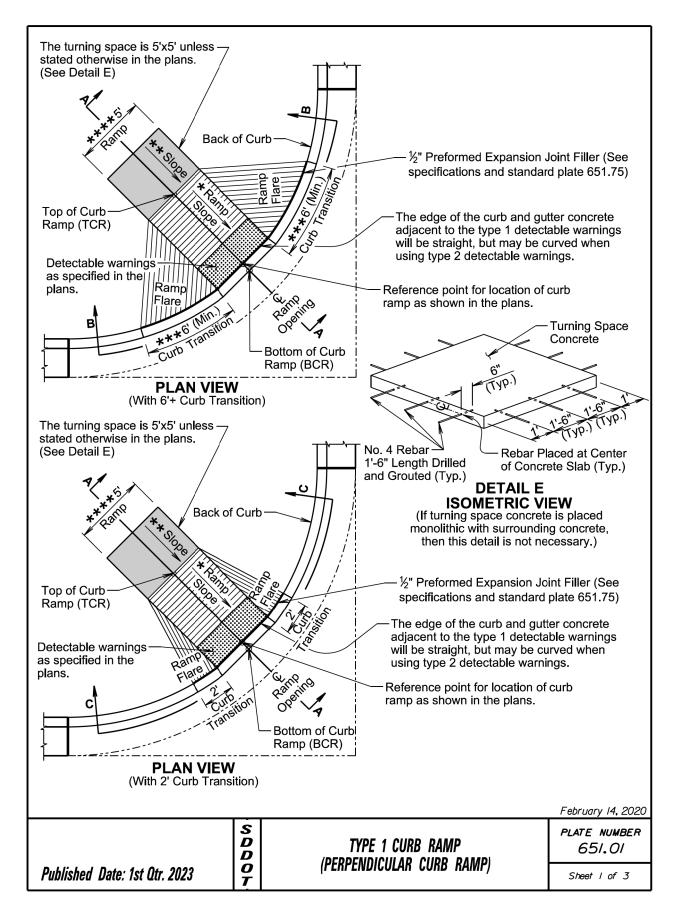
When concrete curb and gutter is not placed monolithically with the mainline PCC pavement or when the adjacent mainline surfacing is not PCC concrete, the transverse contraction joints in the concrete curb and gutter will be 1½ inches deep if formed in the fresh concrete using a suitable grooving tool. If a saw is used to cut the contraction joints, then the depth of the joint will be at least 1/4 the thickness of the concrete and the joint will be sealed in accordance with the details shown above.

December 23, 2019

S D PLATE NUMBER JOINTS IN CONCRETE CURB AND GUTTER D **O T** Published Date: 1st Qtr. 2023

Sheet 2 of 2

650.90



 STATE OF SOUTH DAKOTA
 PROJECT
 SHEET
 TOTAL SHEETS

 B52
 B63

Plotting Date:

Date: 04/04/2023

Curb ramp slopes are designed at 7.5% unless stated otherwise in the plans. The curb ramp may have a maximum slope of 8.3% and will not exceed 15' in length unless stated otherwise in the plans.

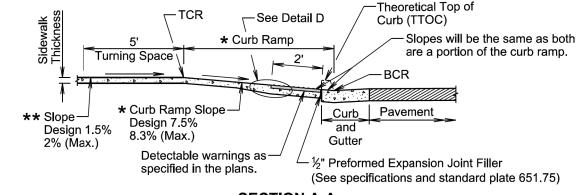
The curb ramp length may be computed based on the intersection of a continuous 1.5% theoretical slope from theoretical top of curb (TTOC) with the curb ramp using a continuous 7.5% curb ramp slope. The elevation of point TCR will always be higher than the elevation of point TTOC unless specified otherwise in the plans. The curb ramp length dimension as shown in the plans will be adjusted as necessary to meet all slope and length requirements based on field geometrics.

The cross slope of the ramp will not be steeper than 2%. Plans are designed using a 1.5% slope unless stated otherwise in the plans.

- ** The slope in the turning space will not be steeper than 2% in any direction of pedestrian travel. Plans are designed using a 1.5% slope unless stated otherwise in the plans.
- *** The curb transition will be a minimum of 6' long, a maximum of 10' long, and the curb transition slope will not be steeper than 10% unless stated otherwise in the plans. The curb transition length will be adjusted as necessary to meet slope and length requirements based on field geometrics.

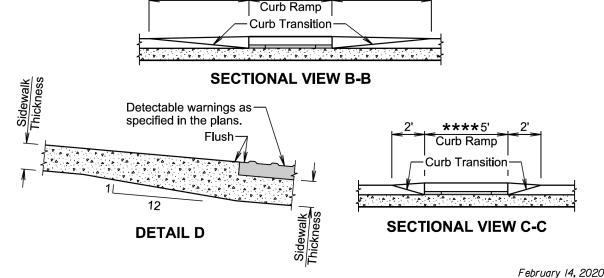
**** The ramp width is 5' unless stated otherwise in the plans.

***6' (Min.)



SECTION A-A

****5'



Published Date: 1st Qtr. 2023

TYPE 1 CURB RAMP (PERPENDICULAR CURB RAMP)

***6' (Min.)

PLATE NUMBER 651.01

Sheet 2 of 3

GENERAL NOTES:

For illustrative purpose only, type 1 detectable warnings are shown in the drawings.

For illustrative purpose only, PCC fillet sections are shown in the drawings. The curb ramp depicted on this standard plate may be used with a PCC fillet section or curb and gutter.

For illustrative purpose only, the curb ramp location is shown at the center of a PCC fillet section. The curb ramp will be placed at the location stated in the plans.

Sidewalk will not be placed adjacent to the curb ramp flares when a 2-foot curb transition is used unless shown otherwise in the plans.

★ Care will be taken to ensure a uniform grade on the curb ramp, free of sags and short grade changes.

Surface texture of the curb ramp will be obtained by coarse brooming transverse to the slope of the curb ramp.

The normal gutter line profile will be maintained through the area of the ramp opening.

Joints will be sawed or tooled into the concrete adjacent to the detectable warnings to alleviate possible corner cracking.

Care will be taken to ensure that the surface of the detectable warnings are clean and maintains a uniform color.

The detectable warnings will be cut as necessary to fit the plan specified limits of the detectable warnings. Cost for cutting the detectable warnings will be incidental to the corresponding detectable warning contract

There will be no separate payment for curb ramps. The curb ramp will be measured and paid for at the contract unit price per square foot for the corresponding concrete sidewalk contract item. The square foot area of the detectable warnings will be included in the measured and paid for quantity of sidewalk.

If rebar is placed in the turning space as depicted in detail E, the cost of the materials, labor, and equipment to furnish and install the rebar will be incidental to the contract unit price per square foot for the corresponding concrete sidewalk contract item.

The curb transitions and ramp opening will be measured and paid for at the contract unit price per foot for the corresponding curb and gutter contract item when curb and gutter is used. The curb transitions and ramp opening will be measured and paid for at the contract unit price per square yard for the corresponding PCC fillet section contract item when a PCC fillet section is used.

The type 1 detectable warnings will be measured to the nearest square foot. All costs for furnishing and installing the type 1 detectable warnings including labor, equipment, materials, and incidentals will be paid for at the contract unit price per square foot for "Type 1 Detectable Warnings".

The type 2 detectable warnings will be measured to the nearest square foot. All costs for furnishing and installing the type 2 detectable warnings including labor, equipment, and materials, including adhesive, necessary sealant or grout, and necessary grinding will be paid for at the contract unit price per square foot for "Type 2 Detectable Warnings".

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0

February 14, 2020

PLATE NUMBER TYPE 1 CURB RAMP 651.01

Published Date: 1st Qtr. 2023

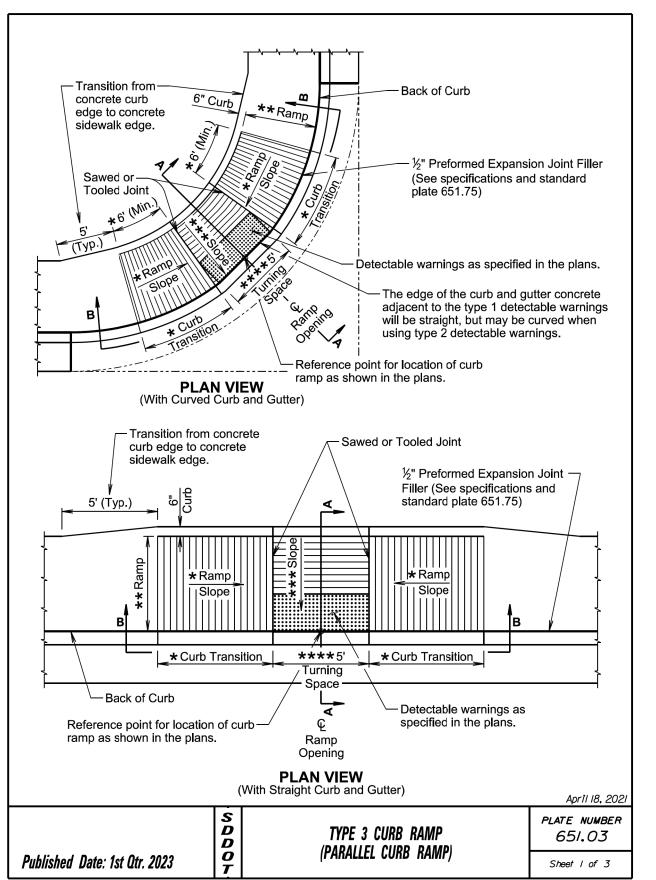
(PERPENDICULAR CURB RAMP)

Sheet 3 of 3

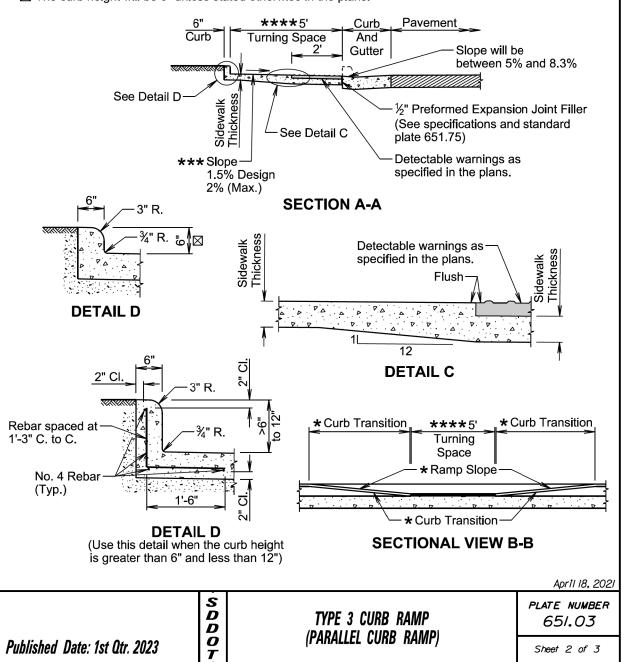
STATE OF	PROJECT	SHEET	TOTAL SHEETS	
SOUTH DAKOTA	IM-B 1902(67)0	B53	B63	

Plotting Date:

04/04/2023



- ★ The curb transition slope will match the curb ramp slope. Curb ramp slopes are designed at 7.5% unless stated otherwise in the plans. The curb ramp may have a maximum slope of 8.3% at any location of the curb ramp and will not exceed 15' in length unless stated otherwise in the plans. The curb transitions and curb ramp lengths will be adjusted as necessary to meet all slope and length requirements based on field
- ** The cross slope of the ramp will not be steeper than 2% and the ramp width is 5' unless stated otherwise in the plans. Plans are designed using a 1.5% cross slope for the ramp unless stated otherwise in the plans.
- *** The slope in the turning space will not be steeper than 2% in any direction of pedestrian travel. Plans are designed using a 1.5% slope unless stated otherwise in the plans.
- **** The turning space is 5'x5' unless stated otherwise in the plans.
 - ☑ The curb height will be 6" unless stated otherwise in the plans.



STATE OF	PROJECT	SHEET	TOTAL SHEETS
SOUTH	IM P 1003/67/0		
DAKOTA	IM-B 1902(67)0	B54	B63

Plotting Date:

04/04/2023

GENERAL NOTES:

For illustrative purpose only, type 1 detectable warnings are shown in the drawings.

For illustrative purpose only, a PCC fillet section is shown in one of the drawings. The curb ramp depicted on this standard plate may be used with a PCC fillet section or with curb and gutter.

The curb ramp will be placed at the location stated in the plans.

Sidewalk adjacent to the curb ramp will be as shown in the plans.

Care will be taken to ensure a uniform grade on the curb ramp, free of sags and short grade changes.

Surface texture of the curb ramp will be obtained by coarse brooming transverse to the slope of the curb ramp.

The normal gutter line profile will be maintained through the area of the ramp opening.

Joints will be sawed or tooled into the concrete adjacent to the detectable warnings to alleviate possible corner cracking (see plan view for joint location).

Care will be taken to ensure that the surface of the detectable warnings are clean and maintains a uniform color.

The detectable warnings will be cut as necessary to fit the plan specified limits of the detectable warnings. Cost for cutting the detectable warnings will be incidental to the corresponding detectable warning contract item.

When curb height is greater than 6" and less than 12", reinforcing steel is required in accordance with the detail on sheet 2 of 3. The reinforcing steel will conform to ASTM A615, Grade 60. Cost for furnishing and installing the reinforcing steel will be incidental to the contract unit price per square foot for the corresponding concrete sidewalk contract item.

There will be no separate payment for curb ramps. The curb ramp will be measured and paid for at the contract unit price per square foot for the corresponding concrete sidewalk contract item. The square foot area of the detectable warnings and the curb along the short radius will be included in the measured and paid for quantity of sidewalk.

The curb transitions and ramp opening will be measured and paid for at the contract unit price per foot for the corresponding curb and gutter contract item when curb and gutter is used. The curb transitions and ramp opening will be measured and paid for at the contract unit price per square yard for the corresponding PCC fillet section contract item when a PCC fillet section is used.

The type 1 detectable warnings will be measured to the nearest square foot. All costs for furnishing and installing the type 1 detectable warnings including labor, equipment, materials, and incidentals will be paid for at the contract unit price per square foot for "Type 1 Detectable Warnings".

The type 2 detectable warnings will be measured to the nearest square foot. All costs for furnishing and installing the type 2 detectable warnings including labor, equipment, and materials, including adhesive, necessary sealant or grout, and necessary grinding will be paid for at the contract unit price per square foot for "Type 2 Detectable Warnings".

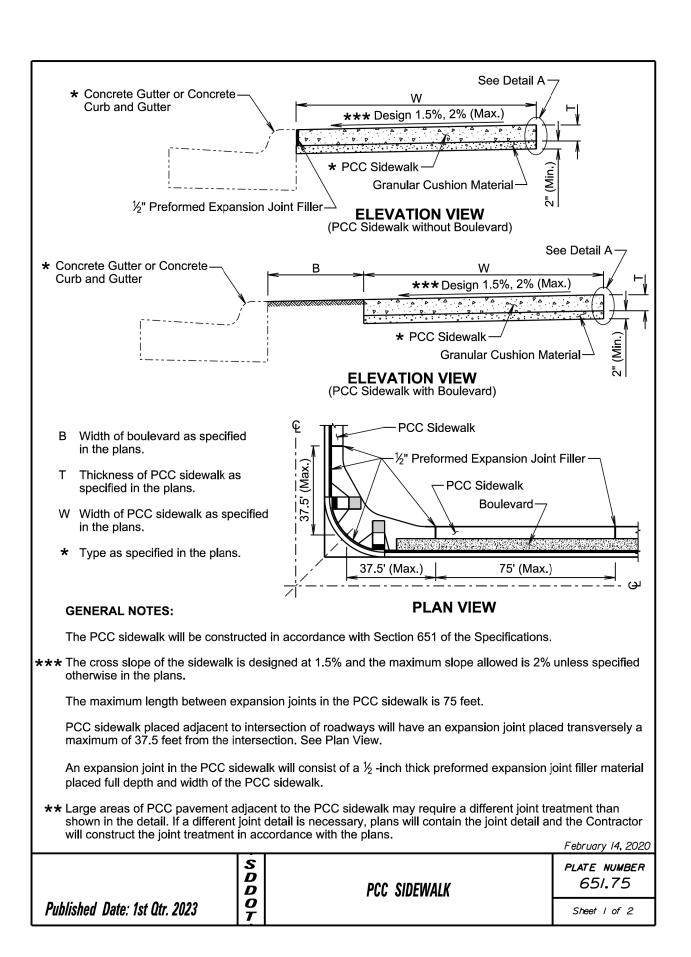
April 18, 2021

D \bar{D} 0 Published Date: 1st Qtr. 2023

TYPE 3 CURB RAMP (PARALLEL CURB RAMP) PLATE NUMBER *651.03*

Sheet 3 of 3

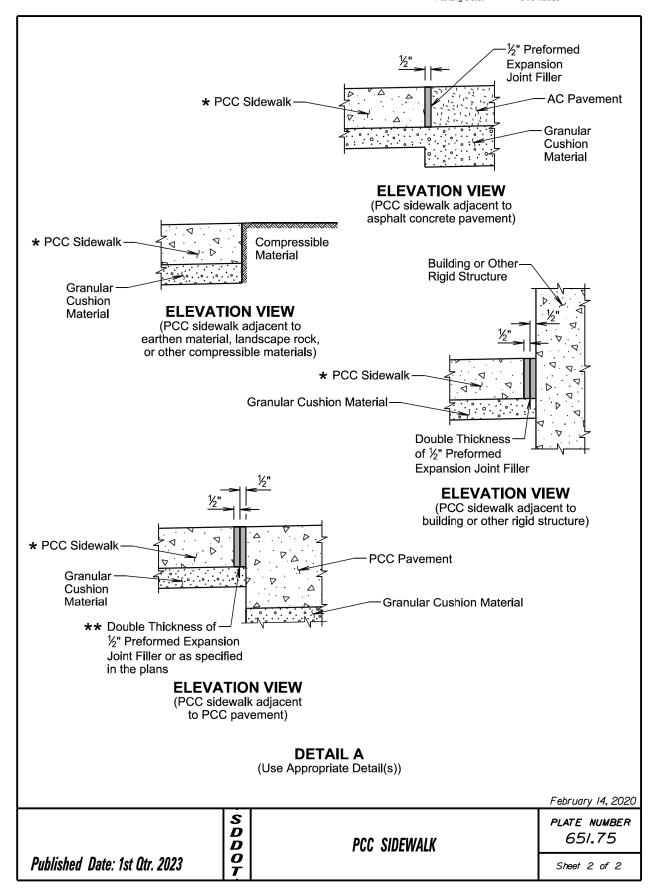




STATE OF PROJECT SHEET TOTAL SHEETS IM-B 1902(67)0 B55 B63 DAKOTA

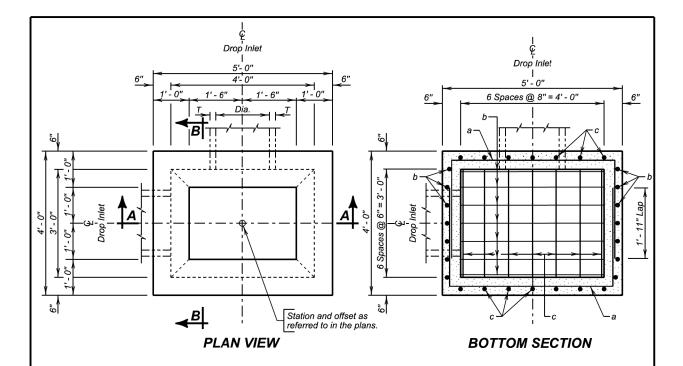
Plotting Date:

04/04/2023



Plotting Date:

04/04/2023



ESTIMATED QUANTITIES						
ITEM	UNIT	CONSTANT QUANTITY	VARIABLE QUANTITY			
★ Class M6 Concrete	Cu. Yd.	0.72	0.30H			
Reinforcing Steel	Lb.	130.93	36.54H			
Frame and Grate Assembly	Each	1				

DROP INLETS FOR 12" TO 36" DIAMETER PIPE

SPECIFICATIONS

Design Specifications: AASHTO LRFD Bridge Design Specifications, 2012 Edition.

Construction Specifications: South Dakota Standard Specifications for Roads and Bridges, Current Edition and required Provisions, Supplemental Specifications, and Special Provisions as included in the Proposal.

GENERAL NOTES:

Design Live Load: HL-93. No construction loading in excess of legal load

Reinforcing steel shall conform to ASTM A615 grade 60. The d bars shall be lapped 12 inches with the b and c bars. Cut and bend reinforcing steel as required to place pipe(s) through the drop inlet wall.

Drop inlet may be precast. If precast drop inlet details differ from this standard plate, submit a checked design done by a SD registered P.E. and shop plans to the Office of Bridge Design for approval.

X Reduce total quantities of concrete by the amount of concrete displaced by the pipe(s). The total quantity of concrete shall be computed to the nearest hundredth of a cubic yard. The total quantity of reinforcing steel shall be computed to the nearest pound.

Drop inlet shown may be modified by the addition or omission of connecting pipes as noted elsewhere in the plans. All pipes entering drop inlet must fit between the inside face of walls and shall not enter through the corners.

Maximum R.C.P. diameter shall not exceed 24 inches (24 inches for R. C. arch) on the 3-foot wide side and shall not exceed 36 inches (30 inches for R.C. arch)

on the 4-root wide side of the drop inlet.	
The dimension of H is in feet. Maximum H is 10 feet.	December

S D D 0 Published Date: 1st Qtr. 2023

3'X 4'TYPE B REINFORCED CONCRETE DROP INLET PLATE NUMBER 670.02

16, 2015

DISPLACEMENT

REDUCTIONS

Diameter

(Inches)

15

18

24

30

36

30

Wall Class M6 T Concrete

(Inches) (Cu. Yd.)

2 0.03

2 1/4 0.04

21/2 0.05

3 0.09

3 1/2 0.14

4 0.20

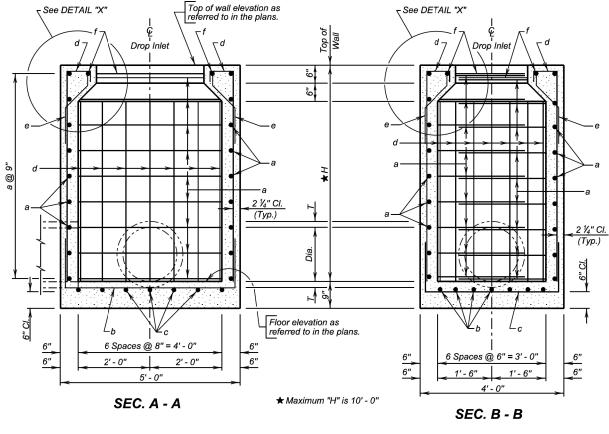
4 0.14

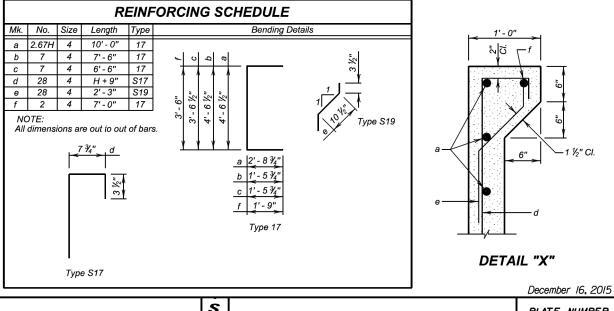
18 2½ 0.05 24 3 ½ 0.09

> Published Date: 1st Qtr. 2023 Sheet I of 2

D D 0

3'X 4'TYPE B REINFORCED CONCRETE DROP INLET PLATE NUMBER 670.02 Sheet 2 of 2



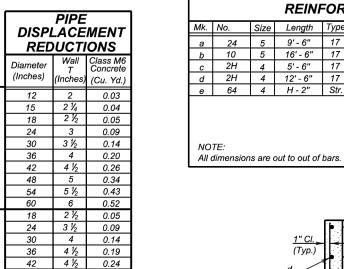


12' - 0" 11'- 0" Station and offset as referred to in plans Drop Inlet Base PLAN Drop Inlet Base e - 23 Spaces @ 6" = 11' - 6" PLAN (Bottom Steel) SPECIFICATIONS: (Pipe Not Shown) Design Specifications: AASHTO LRFD Bridge Design Specifications 2012 Edition. 2. Construction Specifications: South Dakota Standard Specifications for Roads and Bridges, Current Edition and required Provisions, Supplemental Specifications, and Special Provisions as included in the Proposal. **GENERAL NOTES:** 1. Design Live Load: HL-93 loading. No construction loading in excess of legal load was Base is intended for use with a Precast Concrete Type S Drop Inlet Lid, Standard Plate 670.40.
 Base may be precast. If precast base used, and details differ from that shown, the precast base must be on the current approved list. The current approved list is available through proper channels from the SDDOT Office of Bridge Design. To qualify for addition to the approved list, submit a checked design, by South Dakota Registered Professional Engineers and shop plans to the Office of Bridge Design for approval. Design shall be in accordance with the current edition of the AASHTO LRFD Bridge Design ₹ 4. Reduce total quantities of concrete by the amount of concrete displaced by the pipe. The total quantity of concrete shall be computed to the nearest hundredth of a cubic yard. The total quantity of reinforcing steel shall be computed to the nearest pound. 5. Inlets shown may be modified by the addition or omission of connecting pipes as shown on the layouts. Connecting pipes shall not enter the inlet through the corners. 6. Maximum R.C.P. diameter shall not exceed 36 inches (30 inches for R.C. Arch) on the 4-foot 7. Reinforcing steel shall conform to ASTM A615 Grade 60. Cut and bend reinforcing steel as required to place pipe(s) through the inlet wall. 8. Use 1 inch clear cover on all reinforcing steel unless otherwise noted. 9. The dimension of H is in feet. Maximum H is 8 feet. June 26, 2015 PLATE NUMBER D *670.32* D 4'X 11'TYPE S DROP INLET BASE 0

PROJECT TOTAL SHEETS SHEET STATE OF IM-B 1902(67)0 B57 DAKOTA B63

Plotting Date:

04/04/2023



48

54

60

72

ITEM

X Class M6 Concrete

Sheet I of 2

84

5

5 1/2

6

8

0.32

0.39

0.49

0.70

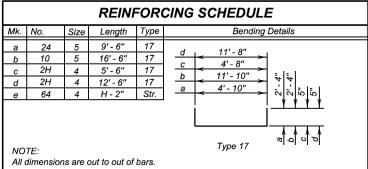
0.93

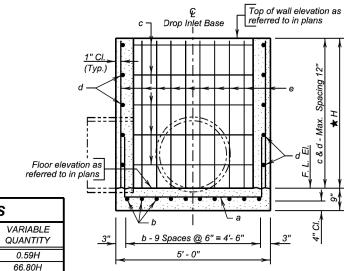
ESTIMATED QUANTITIES

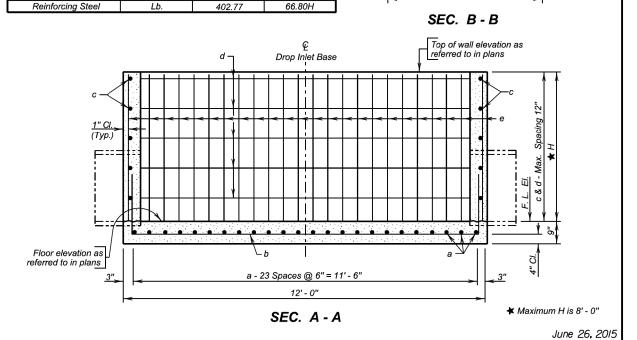
CONSTANT

OLIANTITY

1.67







D \bar{D} 0 Published Date: 1st Qtr. 2023

PLATE NUMBER *670.32* 4'X 11'TYPE S DROP INLET BASE

Sheet 2 of 2

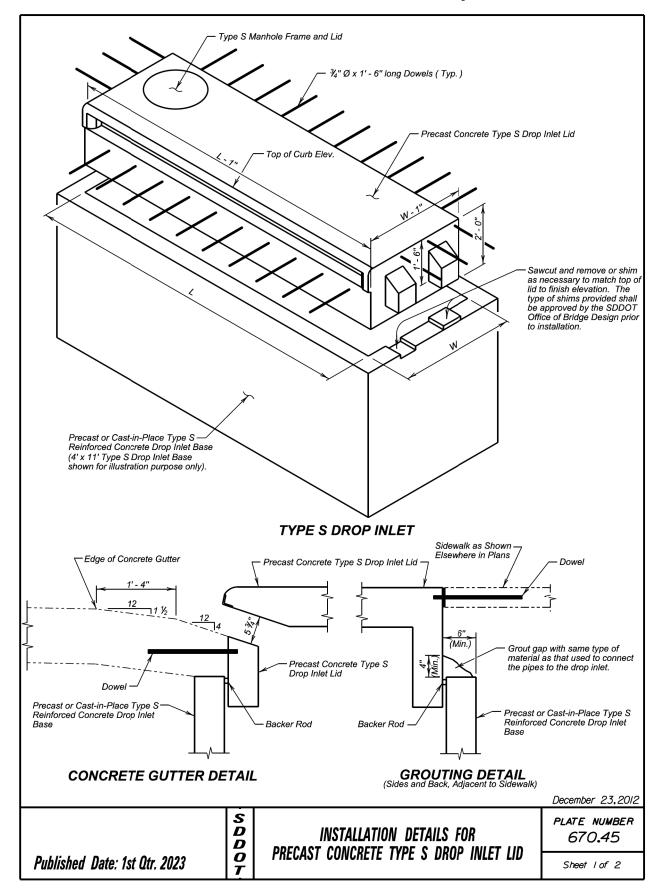
Published Date: 1st Qtr. 2023

Pul

Type S Manhole Frame and Lid ¾" Ø Inserts (Typ.) __7___7__7 ,¹ 1 ½" Overhang **PLAN** 3" Radius Transition Block −¾" Ø Inserts (Typ.) ¾" Ø Inserts ~ 9 spaces @ 12" = 9' - 0" 9' - 11" **ELEVATION** 4' - 0 1/8" .6" ~¾" Ø Inserts $\stackrel{\triangle}{\rightarrow}$ 2 X 2 X $\stackrel{6}{\cancel{1}}$ 10' - 11" with $\stackrel{6}{\cancel{1}}$ $\stackrel{6}{\cancel{1}}$ $\stackrel{7}{\cancel{1}}$ $\stackrel{7}{\cancel{1}$ Spacing (Typ.) -¾" Ø Inserts (Typ.) **GENERAL NOTES:** 1. The Precast Concrete Type S Drop Inlet Lid and the shims shall be on the current approved list available through proper channels from the SDDOT Office of Bridge Design. To qualify for addition to the approved list, submit a checked design, done by South Dakota Registered Professional Engineers, and shop plans to the Office of Bridge Design for approval. Design shall be in accordance with the current edition of the AASHTO LRFD Bridge Design Specifications. 3' - 11" SIDE VIEW (With Sidewalk Inserts) 2. Design Live Load shall be HL - 93. Concrete mix shall be as per fabricators design, however, minimum compressive strength shall not be less than 4500 psi. Type II Cement is required. 4. The Type S Manhole Frame and Lid shall conform to AASHTO M105, Class 30. 5. Structural Steel shall conform to ASTM A36. The ¾ inch diameter Headed Type A Steel Studs shall conform to Section 7 of the current edition of AWS D1. 1 Structural 23 1/4" The ¾ inch diameter Concrete Inserts shall be galvanized or made of a corrosion resistant material. Provide ¾ inch diameter x 1¹ - 6" long dowels conforming to ASTM A615, Gr. 60 threaded to fit inserts with each lid. TYPICAL SECTION THROUGH TYPE S MANHOLE 7. All costs associated with furnishing and installing the Precast Concrete Type S Drop Inlet Lid including the type S manhole frame and lid, shims, inserts, and dowels shall be included FRAME AND LID (Weight 140 Lbs.) in the contract unit price per each for " 4' x 11' Precast Concrete Type S Drop Inlet Lid ". December 23,2012 PLATE NUMBER D 4'X 11' PRECAST CONCRETE *670.40* D TYPE S DROP INLET LID 0 Published Date: 1st Qtr. 2023 Sheet I of I

| STATE OF | SOUTH | DAKOTA | IM-B 1902(67)0 | B58 | B63

Plotting Date: 04/04/2023



PROJECT TOTAL SHEETS STATE OF SHEET IM-B 1902(67)0 B59 B63 DAKOTA

Top of Curb El.

Top of Grate El.

Top of Wall El. (Without Collar)

Top of WallEl. (With Collar)

⊥Floor El.

Type B Drop Inlet

Plotting Date: 04/04/2023

—Curb & Gutter Outline

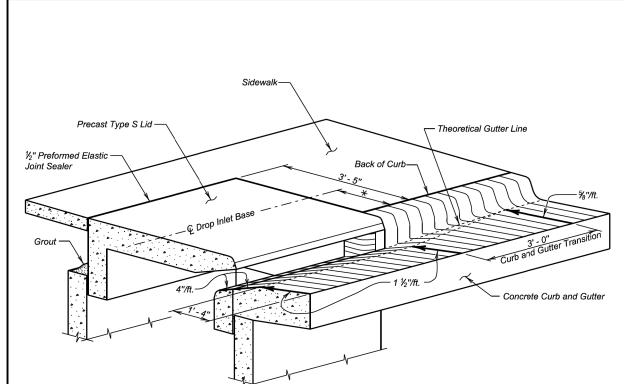
1.67

1.0'

-Precast Drop Inlet Collar

Drop Inlet

(See Standard Plate 670.99)



GENERAL NOTES:

- Dowels shall be used to anchor the precast concrete Type S drop inlet lid to the concrete gutter. See Standard Plate 670.38 or 670.40
 as applicable. If there is sidewalk adjacent dowels shall be used to anchor the precast concrete Type S drop inlet lid to the sidewalk.
 If there is sidewalk adjacent to the drop inlet, the precast lid shall match the finish elevations and cross slopes of the sidewalk.
- 2. The sidewalk shall be steel reinforced when the sidewalk adjoins the precast lid. Refer to Standard Plate 651.70 for reinforced concrete sidewalk details.

Published Date: 1st Qtr. 2023

D D O

GENERAL NOTE:

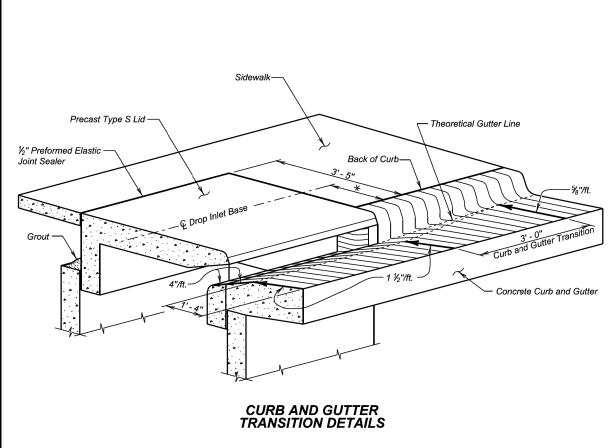
Top of grate elevation shall be 0.04' below theoretical elevation of gutter.

INSTALLATION OF TYPE B DROP INLET

PLATE NUMBER 670.75

Sheet I of I

June 26, 2011

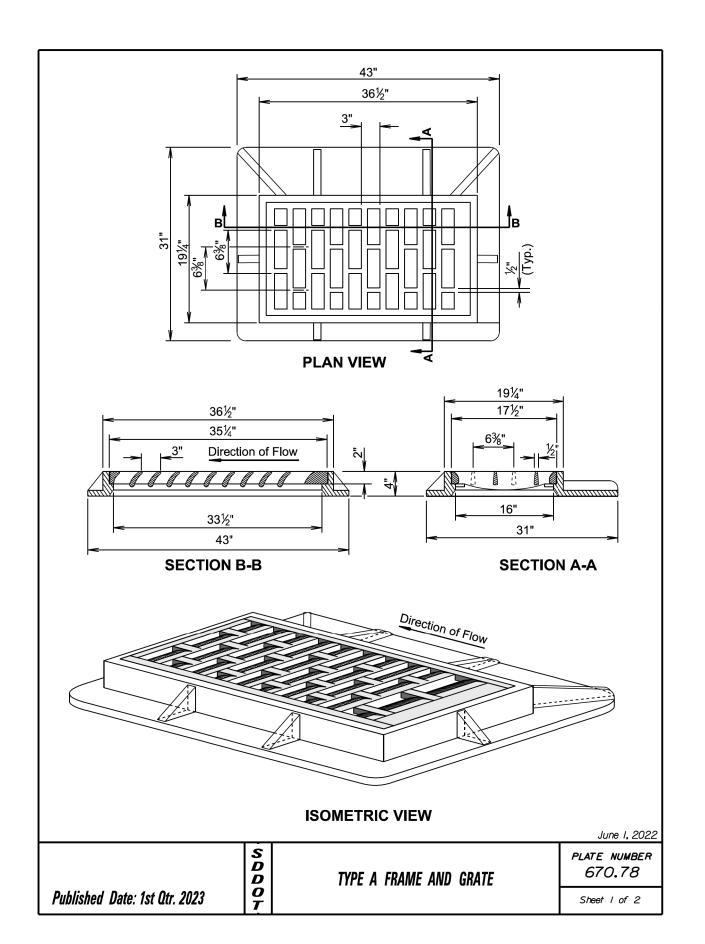


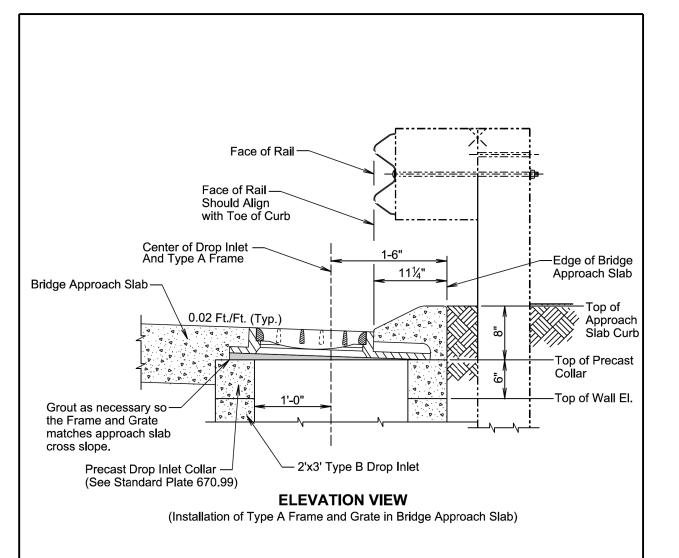
Drop Inlet X Distance Base Unit Size 4' x 6' 1' - 5 1/2" 4' x 11' 7' x 11' 2' - 11 ½" December 23,2012 PLATE NUMBER D D O INSTALLATION DETAILS FOR *670.45* PRECAST CONCRETE TYPE S DROP INLET LID Published Date: 1st Qtr. 2023 Sheet 2 of 2

PROJECT STATE OF SHEET TOTAL SHEETS IM-B 1902(67)0 B60 B63 DAKOTA

Plotting Date:

04/04/2023





GENERAL NOTES:

The product dimensions may vary from those shown on the standard plate depending on the manufacturer. Grate size and configuration will be similar to the standard plate for hydraulic capacity and bicycle safety. Any variation in dimensions will be approved by the Engineer and the type A frame and grate will be from a manufacturer on the approved products list.

Design load for the grate will meet the requirements of AASHTO HL-93.

The type A frame and grate will be installed on a 2'x3' type B drop inlet.

The direction of flow is shown for illustrative purpose only. The grate will be installed to intercept the direction of flow.

June 1, 2022

S D D O Published Date: 1st Qtr. 2023

TYPE A FRAME AND GRATE

670.78

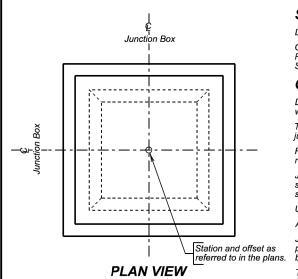
PLATE NUMBER

Sheet 2 of 2

PROJECT SHEET TOTAL SHEETS STATE OF IM-B 1902(67)0 B61 B63 DAKOTA

Plotting Date:

04/04/2023



SPECIFICATIONS

Design Specifications: AASHTO LRFD Bridge Design Specifications, 2012 Edition.

Construction Specifications: South Dakota Standard Specifications for Roads and Bridges, Current Edition and required Provisions, Supplemental Specifications, and Special Provisions as included in the Proposal.

GENERAL NOTES

Design Live Load: HL-93. No construction loading in excess of legal load

The design of the junction box is based on a maximum fill over the junction box of 5 feet and minimum fill over the junction box of 2 feet.

Reinforcing steel shall conform to ASTM A615 Grade 60. Cut and bend reinforcing steel as required to place pipe(s) through junction box wall.

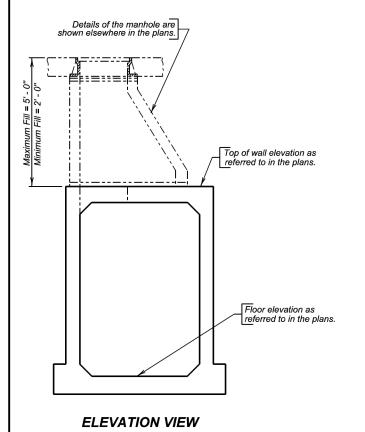
Junction box may be precast. If precast junction box details differ from this standard plate, submit a checked design done by a SD registered P.E. and shop plans to the Office of Bridge Design for approval.

Use 1 inch clear cover on all reinforcing steel unless otherwise noted.

All exposed edges shall be chamfered ¾ inch.

Junction box shown may be modified by the addition or omission of connecting pipes as noted elsewhere in the plans. All pipes entering junction box must fit between the inside face of walls and shall not enter through the corners.

The cost of furnishing and installing the manhole steps shall be incidental to the contract unit price per pound for "Reinforcing Steel".



	PIPE DISPLACEMENT REDUCTIONS				
	Diameter (Inches)	Wall T (Inches)	Class M6 Concrete (Cu. Yd.)		
R.C.P.	12	2	0.03		
	15	2 1/4	0.04		
	18	2 ½	0.06		
	24	3	0.11		
	30	3 1/2	0.16		
	36	4	0.23		
	42	4 1/2	0.31		
	48	5	0.40		
	54	5 ½	0.50		

ESTIMATED QUANTITIES					
ITEM	X Class M6 Concrete	Reinforcing Steel			
UNIT	Cu. Yd.	Lb.			
H = 4' - 0"	4.37	821			
H = 4' - 6"	4.61	846			
H = 5' - 0"	4.85	908			
H = 5' - 6"	5.10	933			
H = 6' - 0"	5.34	958			
H = 6' - 6"	5.58	1020			
H = 7' - 0"	5.82	1045			
H = 7' - 6"	6.06	1071			
H = 8' - 0"	6.30	1132			

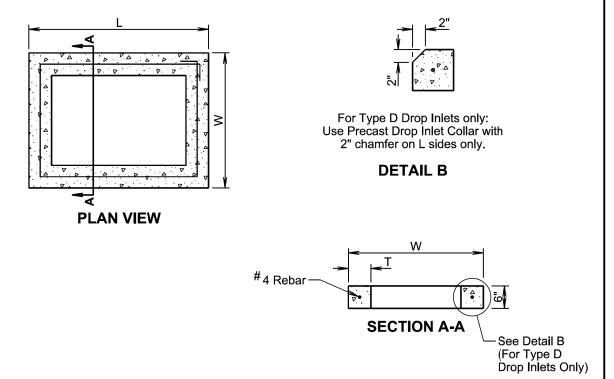
★ Reduce total quantities of concrete by the amount of concrete displaced by the pipe(s). Quantity shown includes reduction for a 24-inch diameter manhole opening. The total quantity of concrete shall be computed to the nearest hundredth of a cubic yard.

May 9, 2020

S D D 0 Published Date: 1st Qtr. 2023

5'X 5' JUNCTION BOX PLATE NUMBER 671.01

Sheet I of 3



INFORMATIONAL QUANTITIES					
FRAME AND GRATE TYPE	L (Ft-in)	W (Ft-in)	T (in)	CLASS M6 CONCRETE (CuYd)	REINFORCING STEEL (Lb)
TYPE A, B, and E	4'-0"	3'-0"	6	0.11	9
TYPE C	5'-0"	4'-0"	6	0.15	11
TYPE D	4'-0"	2'-6"	6	0.10	8

GENERAL NOTES:

All reinforcing steel will conform to ASTM A615, Grade 60.

The $\frac{1}{2}$ " diameter bar will lap 6"± and will be centered in the concrete.

The cost of furnishing and installing Precast Drop Inlet Collars, including labor, materials, and incidentals will be incidental to the contract unit price per Each for "Precast Drop Inlet Collar".

June 1, 2022

PLATE NUMBER D D 670.99 PRECAST DROP INLET COLLAR 0 Published Date: 1st Qtr. 2023 Sheet I of I

Mk. No. Size Length Type

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REINFORCING SCHEDULE

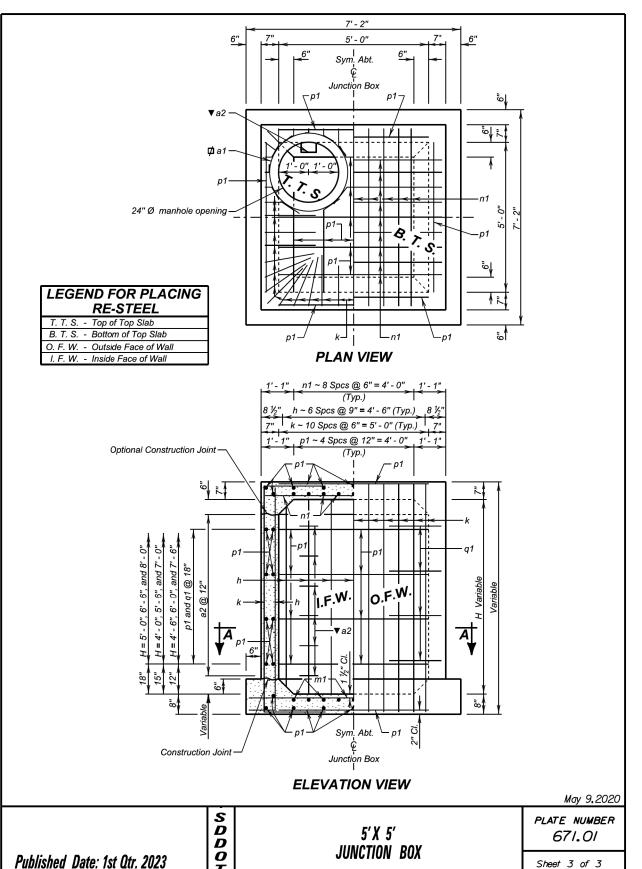
Mk. No. Size Length Type

Sheet 2 of 3

Bending Details

PROJECT TOTAL SHEETS SHEET STATE OF IM-B 1902(67)0 B62 DAKOTA B63

Plotting Date: 04/04/2023



Plotting Date:

04/04/2023

f	
A	PLAN VIEW (Lid)
PLAN VIEW (Frame)	
Machine Finished Seat	ISOMETRIC VIEW
24" (Min.) 34" (Min.) SECTION A-A	TYPE HEIGHT (Inches) A7 7 A8 8
GENERAL NOTES:	A9 9 A10 10
The product dimensions may vary from those shown on the s	tandard plate depending on the manufacturer

The product dimensions may vary from those shown on the standard plate depending on the manufacturer. Any variation in dimensions will be approved by the Engineer and the type A manhole frame and lid will be from a manufacturer on the approved products lists.

Design load for the grate will meet the requirements of AASHTO HL-93.

SDDOT

Geometric pattern on top of lid other than that shown will be approved by the Engineer.

June 1, 2022

Published Date: 1st Qtr. 2023

TYPE A MANHOLE FRAME AND LID

PLATE NUMBER 671.10

Sheet I of I

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