

STATE OF	PROJECT	SHEET	TOTAL SHEETS
SOUTH DAKOTA	NH-CR 0013(158)126	F1	F27
Plotting Date:	02/07/2024		

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SECTION F – ESTIMATE OF QUANTITIES

BID ITEM NUMBER	ITEM	QUANTITY	UNIT
120E6200	Water for Granular Material	370.0	MGal
260E1010	Base Course	10,989.1	Ton
260E1080	Base Course, Salvaged, State Furnished	19,846.7	Ton
320E1200	Asphalt Concrete Composite	514.0	Ton
330E0300	SS-1h or CSS-1h Asphalt for Fog Seal	0.2	Ton
330E3000	Sand for Fog Seal	3.8	Ton
380E0070	9" Nonreinforced PCC Pavement	32,405.8	SqYd
380E3020	6" PCC Driveway Pavement	171.4	SqYd
380E3040	8" PCC Driveway Pavement	404.6	SqYd
380E6000	Dowel Bar	19,311	Each
380E6110	Insert Steel Bar in PCC Pavement	307	Each

SURFACING THICKNESS DIMENSIONS

The plans shown spread rates will be applied even though the thickness may vary from that shown in the plans.

At those locations where material must be placed to achieve a required elevation, the depth/quantity may be varied to achieve the required elevation.

EXISTING PCC PAVEMENT

The existing concrete in Section 1 is 7" Plain Jointed PCC Pavement. The existing transverse joints are perpendicular and are spaced at 15 feet. The aggregate in the existing Plain Jointed PCC Pavement is Crushed Quartzite Ledge Rock. The longitudinal joints have 30" long #4 Deformed Tie Bars at a spacing of 48".

The existing concrete in Section 2 is 4" Fiber Reinforced PCC Pavement on top of 4.25" of wire mesh concrete. The existing transverse joints are perpendicular and are spaced at 12.5 feet within the fiber reinforced section and 20 ft within the wire mesh concrete. The aggregate in the existing PCC Pavement is Crushed Quartzite Ledge Rock.

The existing concrete in Section 3 is 6.5" Plain Jointed PCC Pavement. The aggregate in the existing Plain Jointed PCC Pavement is Crushed Quartzite Ledge Rock. The existing transverse joints have a spacing of 20' with 1 1/2" x 18" Plain Dowel Bars at a spacing of 12". The longitudinal joints have 20" long #4 Deformed Tie Bars at a spacing of 30".

RECYCLED CONCRETE AGGREGATE (RCA)

Portland cement concrete pavement (RCA) removed from the mainline within the project limits may be crushed and reused as granular material provided it meets the requirements for Base Course.

All in-place rebar will be separated and removed from the RCA.

There is an estimated 12,545 tons of PCC Pavement on this project that can be crushed and reused. This guantity is based on a unit weight of 118 lbs. per cubic foot for the recycled concrete aggregate.

The Contractor will dispose of the material (including existing rebar) not utilized on the project at a site approved by the Engineer.

Payment for the recycled concrete aggregate will be at the contract unit price per ton for Base Course.

REINFORCEMENT FABRIC (MSE) For SOFT AND UNSTABLE AREAS

If, in the opinion of the Engineer, the subgrade will not stabilize by being reworked and recompacted, Reinforcement Fabric (MSE) and granular material may be used. Contact the Geotechnical Engineering Activity (605-773-3725) for assistance should the use of Reinforcement Fabric (MSE) and additional granular material become necessary. See Section B for installation and specifications of Reinforcement Fabric (MSE).

An additional 1,481 tons. (80' x 500' x 1') of Base Course and 17.8 MGal of Water for Granular Material is included in the materials guantities for Section F. This quantity can be adjusted or eliminated by CCO, depending on field conditions.

BASE COURSE, SALVAGED, STATE FURNISHED

Base Course, Salvaged, State Furnished estimated at 19,846.7 tons of granular material will be obtained from the stockpile site located in the SE 1/4 of Section 36, Township 111 North, Range 62 West of the Black Hills Meridian, Beadle County, South Dakota at the East Huron Maintenance Yard. There is approximately 76,000 tons of material in the stockpile.

No gradation testing will be required for the Base Course, Salvaged, State Furnished material.

The Base Course, Salvaged, State Furnished is royalty free to the Contractor.

All other requirements for Base Course, Salvaged will apply.

CURB AND GUTTER

The curb and gutter cannot be placed monolithically with mainline pavement if the mainline lane is wider than 12 feet. If the mainline lane is wider than 12 feet, it must be placed with a separate operation according to Standard Plate 380.21.

ASPHALT CONCRETE COMPOSITE

Asphalt Concrete Composite will include MC-70 Asphalt for Prime placed at the rate of 0.30 gallons per square yard. The Asphalt for Prime will be applied to the Base Course, Salvaged, State Furnished or Base Course for the full width of the bottom layer of Asphalt Concrete Composite plus one foot additional on the outside shoulder.

Asphalt for tack SS-1h or CSS-1h will be applied prior to each lift of Asphalt Concrete Composite. Asphalt for tack will be applied at a rate of 0.09 gallons per square yard on existing pavement or milled asphalt concrete surfaces and at a rate of 0.06 gallons per square yard on primed base course or new asphalt concrete pavement. The Asphalt for tack will be applied for the full width of the bottom layer of Asphalt Concrete Composite plus one-half foot additional on the outside shoulder. Asphalt for Flush Seal will be applied at a rate of 0.05 gallons per square yard. Sand for Flush Seal will be applied at a rate of 8.00 lbs. per square yard.

FOG SEAL

The Contractor will apply fog seal to the entire parking lot at Sta. 25+61 Rt. Total quantity of Fog Seal is equal to 0.2 tons. The Contractor will plan the fog seal operation to allow adequate cure time for the fog seal before traffic is allowed on the surface.

The Contractor will apply sand to the entire parking lot receiving the fog seal. Sand applied will be broomed off the surface of the parking lot once the fog seal has sufficiently cured as determined by the Engineer. Sand for Fog Seal will conform to Section 879.1.B. Sand for Fog Seal will be applied at a rate of 8.0 lbs. per square yard. Total quantity of Sand for Fog Seal is equal to 3.8 tons.

Prior to hauling. Sand for Fog Seal will be screened to minimize segregation. eliminate oversize, and effectively breakup or discard material bonded into chunks. All costs for supplying, hauling, placing, and brooming the blotting sand will be incidental to the contract unit price per ton for "Sand for Fog Seal".

PREPARATION FOR PARKING LOT & DRIVEWAY PAVEMENTS

The foundation will be excavated, shaped, and compacted to a firm, uniform bearing surface. Unsuitable foundation material will be removed and replaced as directed by the Engineer. The foundation will be thoroughly moistened immediately prior to placing the PCC Pavement. Moisture will be applied without forming pools of water.

Granular material will be placed to the depth specified and satisfactorily compacted. Payment for any excavation will be incidental to the contract unit price of the surfacing material.

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Prior to the application of the fog seal, the Contractor will be required to broom the application area. A CSS-1h or SS-1h emulsion will be used for the fog seal application. A water-to-emulsion rate of 1:1 should be used for the Fog Seal application. SS-1h or CSS-1h Asphalt for Fog Seal will be applied at a rate of 0.05 gallon per square yard.

9" NONREINFORCED PCC PAVEMENT

The aggregate may require screening as determined by the Engineer.

The concrete mix will conform to the special provision for Contractor Furnished Mix Design for PCC Pavement.

In lieu of an automatic subgrader operating from a preset line, a motor grader or other suitable equipment may be used to trim the gravel cushion to final grade prior to placement of concrete. There will be no direct payment for trimming of the gravel cushion for PCC payement. The trimming will be considered incidental to the related items required for PCC Pavement.

A construction joint will be sawed whenever new concrete pavement is placed adjacent to existing concrete pavement.

The surface of the mainline paving will be a heavy carpet drag. All other areas will be textured as directed by the Engineer. The surface of the mainline paving will receive a heavy carpet drag to within 2 or 3 feet of the face of the curb.

Unless specified otherwise in the PCC Pavement Joint Layout Sheets or elsewhere in the plans, the typical joint spacing for 9" Nonreinforced PCC Pavement will be 14'. Joint spacing in the PCC Shoulder Pavement will match adjacent mainline pavement.

See Standard Plate 380.01 and 380.04 for placement of Dowel Bars. The transverse construction joints will be handled in accordance with Standard Plate 380.15.

The transverse contraction joints will be perpendicular to the centerline. In multilane areas the transverse contraction joints will be perpendicular to the centerline and be in a straight line across the entire width of the pavement. In special situations the Engineer may pre-approve transverse contraction joints that do not meet these requirements. All nonconforming transverse contraction joints will be removed at the Contractor's expense. Any method of placement that cannot produce these requirements will not be allowed.

The location of joints, as shown and designated on the PCC Pavement Joint Layout(s) are only approximate locations to be used as a guide and to afford bidders a basis for estimating the construction cost of the joints. The final locations of the joints are to be designated by the Engineer during construction.

The mainline pavement Sta. 9+32.27 to Sta. 58+84.3 will be tested for smoothness with a Contractor furnished and operated 25-foot California style profilograph in accordance with the Special Provision for PI PCC Pavement Smoothness with 0.2 Blanking Band. The Engineer will designate areas not requiring the testing listed above.

CURING OF CONCRETE

Portland Cement Concrete Pavement, Concrete Curb & Gutter, Concrete Gutter, and Concrete Fillet will be cured with Linseed Oil Base Emulsion Compound. All costs for Curing of Concrete will be incidental to the contract unit price per various Portland Cement Concrete bid items.

ALKALI SILICA REACTIVITY

Fine aggregate will conform to Section 800.2 D Alkali Silica Reactivity (ASR) Requirements.

Below is a list of known fine aggregate sources and the average corresponding 14-day expansion values (as of 8-30-2023):

Source	Location	
Bachman	Winner, SD	0.335*
Bitterman	Delmont, SD	0.316*
Concrete Materials	Corson, SD	0.146
Concrete Materials - Vellek Pit	Yankton, SD	0.411**
Croell	Hot Springs, SD	0.089
Croell	Wasta, SD	0.212
Emme Sand & Gravel	Oneil, NE	0.212
Fisher S&G – Blair Pit	W of Vale, SD	0.217
Fisher S&G - Mickelson Pit	E of Nisland, SD	0.129
Fisher S&G - Vallery Pit	Nisland, SD	0.110
Fisher S&G	Rapid City, SD	0.092
Fisher S&G	Spearfish, SD	0.053
Fisher S&G	Wasta, SD	0.159
Fuchs	Pickstown, SD	0.275*
Henning – Tilstra Pit	Ash Creek, MN	0.199
Higman	Hudson, SD	0.187
Jensen	Herried, SD	0.276*
L.G. Everist	Akron, IA	0.257*
L.G. Everist	Brookings, SD	0.297*
L.G. Everist – Ode Pit	E Sioux Falls, SD	0.215
L.G. Everist – Nelson Pit	NE Sioux Falls, SD	0.156
L.G. Everist	Hawarden, IA	0.176
L.G. Everist	Summit, SD	0.184
Mark's S&G – Moerke Pit	Underwood, MN	0.165
Morris – Birdsall	Blunt, SD	0.229
Morris - Leesman	Blunt, SD	0.231
Morris - Richards Pit	Onida, SD	0.188
Morris - Shawn's Pit	E of Sturgis, SD	0.186
Northern Concrete Agg.	Rauville, SD	0.113
Northern Concrete Agg.	Luverne, MN	0.133
Opperman - Gunvordahl Pit	Burke, SD	0.363*
Opperman - Cahoy Pit	Herrick, SD	0.307*
Opperman - Jones Pit	Burke, SD	0.321*
Opperman - Randall Pit	Pickstown, SD	0.230
Pete Lien & Sons	Creston, SD	0.158
Pete Lien & Sons	Oral, SD	0.157
Pete Lien & Sons	Wasta, SD	0.226
Simon Materials - Beltline Pit	Scottsbluff, NE	0.277*
Thorpe Pit	Britton, SD	0.098
Wagner Building Supplies	Pickstown (Wagner), SD	0.251*
Winter Brothers- Whitehead Pit	Brookings, SD	0.197

- * These sources will require Type II cement with a fly ash content of 25% in the concrete mix.
- ** These sources will not be used.

The Department will use the running average of the last three or fewer known expansion test results for determining acceptability of the source. These expansion results are reported in the preceding table. Additional testing, when requested by the Contractor, will be performed by the Department at the Contractor's expense.

The values listed in the table are intended for use in bidding. If a previously tested pit by SDDOT with a test value less than 0.250 is discovered after letting to be 0.250 or greater, then the Department will accept financial responsibility if higher costs are incurred due to higher percent of fly ash requirement.

TABLE OF PCC PAVEMENT

		N	9" Nonreinforced PCC Pavement
Station		Station	SqYd
	SD37		
9+32.27	to	25+90.9	11,241.6
25+90.9	to	26+73.9	532.8
26+73.9	to	29+74.1	2,068.0
29+74.1	to	31+02.3	712.2
31+02.3	to	34+06.4	2,094.2
34+06.4	799.4		
35+50.8	2,094.2		
38+54.9	to	39+83.4	714.1
39+83.4	to	43+48.1	2,512.4
43+48.1	to	44+89.5	976.9
44+89.5	to	47+05.5	1,697.9
47+32.6	to	58+84.3	8,891.6
Misce	ellaneous	Areas	
Intersecting S	treets - 19	each	1,844.5
		TOTAL	32,405.8

TABLE OF DOWEL BARS

	LOCATION	N	1-1/4" Dowel Bars
Station		Station	Each
	SD37		
9+32.27	to	25+65.7	6,490
25+65.7	to	26+73.9	401
26+73.9	to	29+74.1	937
29+74.1	to	31+02.3	408
31+02.7	to	34+06.3	924
34+06.3	to	442	
35+51.1	to	924	
38+54.7	to	408	
39+83.8	to	1,173	
43+48.1	to	666	
44+89.5	to	803	
47+32.6	to	748	
49+09.8	to	49+80.9	390
49+80.9	to	53+97.5	1650
53+97.5	to	54+82.8	510
54+82.8	to	57+66.7	1109
57+66.7	546		
Misc	ellaneous		
Intersecting St	treets - 19	Each	782
		TOTAL	19,311

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8" and 6" PCC DRIVEWAY PAVEMENT

The concrete for the 8" and 6" PCC Driveway Pavement will comply with the requirements of the specifications for Class M6 Concrete, unless otherwise specified in the Plans. The mix design can meet either Class M6 Concrete specifications or conform to the Contractor Furnished Mix Design for PCC Pavements Special Provision.

The surface of the 8" and 6" PCC Driveway Pavement will have a maximum 10% slope and the tie-ins will match the existing and/or new adjoining PCC Approach Pavement.

Contraction joints in the 8" and 6" PCC Driveway Pavement will be $1\frac{1}{2}$ inches deep if formed in the fresh concrete using a suitable grooving tool. If a saw is used to cut the contraction joints, then the depth of the joint will be at least $\frac{1}{4}$ the thickness of the approach pavement.

All costs for furnishing and placing the 8" and 6" PCC Driveway Pavement and constructing the expansion and contraction joints including labor, equipment, and materials (including the earthen backfill) will be incidental to the contract unit price per square yard for 8" and 6" PCC Driveway Pavement.

Payment for any excavation required for placing the 8" and 6" PCC Driveway Pavement and granular material will be incidental to the contract unit price of the surfacing material.

All costs for furnishing and placing the granular material will be incidental to the contract unit price per ton for Base Course, Salvaged, State Furnished.

TABLE OF PCC PAVEMENT

LO	CATION		6" PCC Driveway Pavement	8" PCC Driveway Pavement
Station	Station Station			SqYd
Driveways / Parking Lots - 12 each			171.4	
Driveways / Parking Lots - 6 each				404.6
		TOTAL	171.4	404.6

STEEL BAR INSERTION

The Contractor will insert the Steel Bars (1 $\frac{1}{4}$ " x 18" Epoxy Coated Plain Round Steel Bars or No. 5 x 24" Epoxy Coated Deformed Tie Bars) into drilled holes in the existing concrete pavement. Anchoring of the steel bars in the drilled holes will conform to the Specifications. The steel bars will be cut to the specified length by sawing or shearing and will be free from burring or other deformations.

1 ¹⁄₄" Epoxy Coated Plain Round Steel Bars will be inserted on 12-inch centers in the transverse joint. The first steel bar will be placed a minimum of 3 inches and a maximum of 6 inches from the outside edge of the slab.

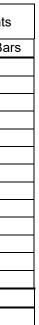
No. 5 Epoxy Coated Deformed Steel Bars will be inserted on 30-inch centers in the longitudinal joint and will be placed a minimum of 15 inches from the existing transverse contraction joint.

TABLE OF STEEL BAR INSERTION

LOCATIO	N	Transverse Joints	Longitudinal Joints	
		1-¼" Dowel Bars	#5 Deformed Tie Ba	
SD37				
Sta. 9+32.27	30.5' Lt. to 30.5' Rt.	61		
Sta. 58+26.7 to Sta. 58+84.3	29.1' Rt. to 31.5' Rt.		23	
Sta. 58+43.7 to Sta. 58+84.3	32.9' Lt. to 34.1' Lt.		16	
Sta. 58+84.3	34.1' Lt. to 31.5' Rt.	65		
Sta. 59+45.12	2.67' Lt. to 10.67' Lt.	8		
Sta. 59+45.12 to Sta. 60+31.12	2.67' Lt. to 2.31' Lt.		34	
Sta. 59+45.12 to Sta. 60+31.12	10.67' Lt. to 10.31' Lt.		34	
Sta. 60+31.12	2.31' Lt. to 10.31' Lt	8		
Market Str. West		48		
3 RD Street NE			10	
	SUBTOTAL	190	117	
	TOTAL	30)7	

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RATES OF MATERIALS

The Estimate of Surfacing Quantities is based on the following quantities of materials per station.

SD37 MAINLINE

Sta. 9+32.27 to Sta. 25+90.9

Base Course

168.79 tons (4" depth)

Water for Granular Material at the rate of 2.03 M Gallons

Base Course, Salvaged, State Furnished 337.59 tons (8" depth)

Water for Granular Material at the rate of 4.05 M Gallons

SD37 MAINLINE

Sta. 26+26.2 to Sta. 43+93.3

Base Course 171.13 tons (4" depth)

Water for Granular Material at the rate of 2.05 M Gallons

Base Course, Salvaged, State Furnished 342.25 tons (8" depth)

Water for Granular Material at the rate of 4.11 M Gallons

SD37 MAINLINE

Sta. 44+46.2 to Sta. 46+49.7 Sta. 48+11.9 to Sta. 56+25.0

Base Course 192.13 tons (4" depth)

Water for Granular Material at the rate of 2.31 M Gallons

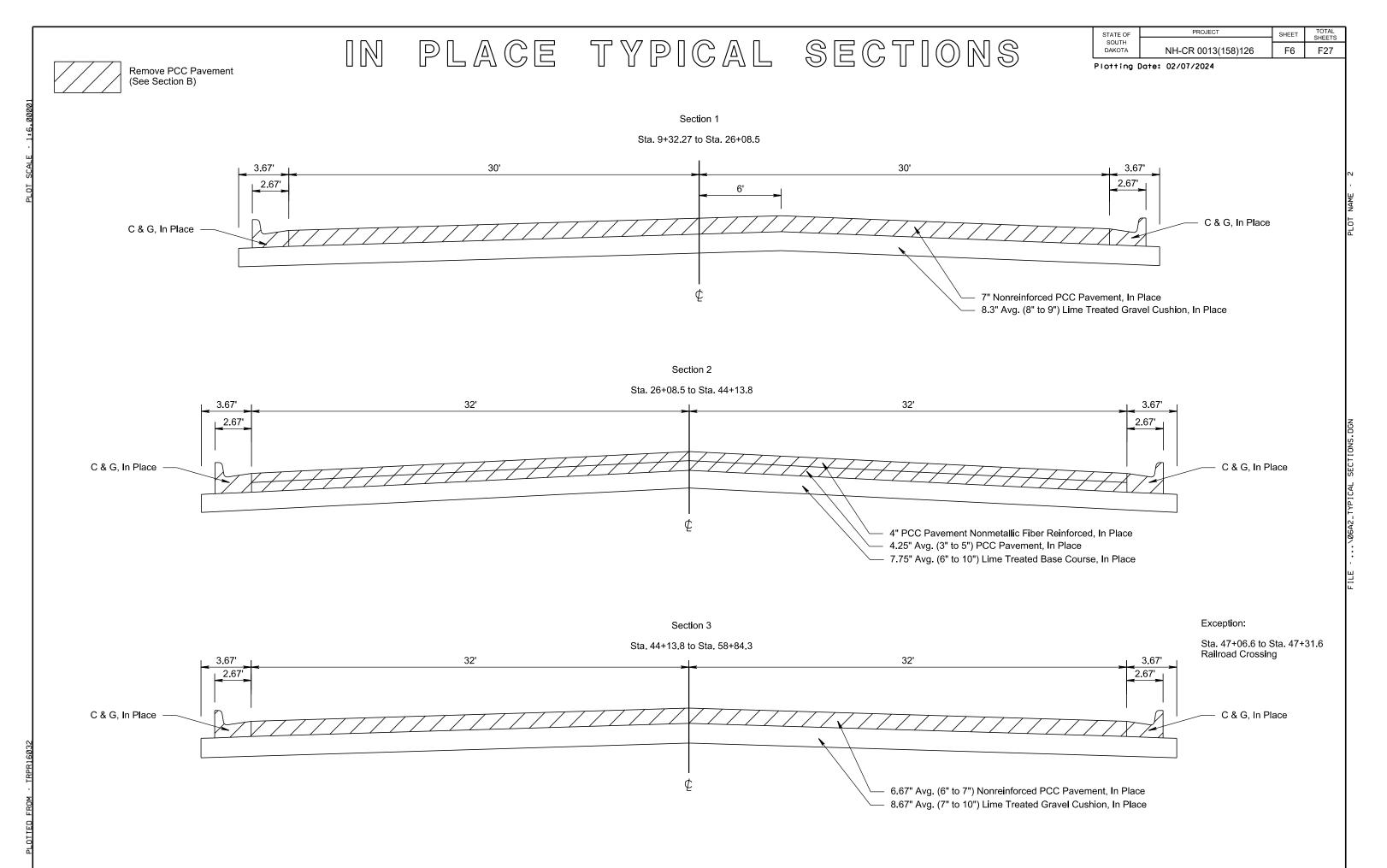
Base Course, Salvaged, State Furnished 384.25 tons (8" depth)

Water for Granular Material at the rate of 4.61 M Gallons

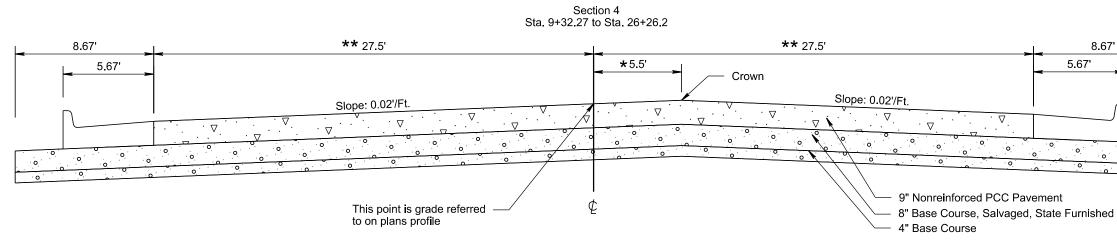
						STATE OF SOUTH	PROJEC	ст	SHEET TOT SHEET
						DAKOTA	NH-CR 001	3(158)126	F5 F
TABLE OF ADDITION	IAL	QUANTITIES				F	Revised: 7F	eb24, RML	
LO	CAT	ION	WATER FOR GRANULAR MATERIAL	4" BASE COURSE	8" BASE COURSE, SALVAGED, STATE FURNISHED	BASE COURSE, SALVAGED, STATE FURNISHED		ALT CONC COMPOSIT	
							1st Lift	2nd Lift	Top Lift
Station	to	Station	(MGal)	(Ton)	(Ton)	(Ton)	(Ton)	(Ton)	(Ton)
			<u> </u>		(00.0				
25+90.90	to	26+26.20	2.1	60.0	120.0				
43+93.30	to	44+46.20	3.3	90.1	180.2				
46+49.70	to	47+06.60 48+11.90	3.9	108.0	215.9				
47+31.60 56+25.00	to to	58+84.30	5.4 16.0	151.5 444.9	303.0 889.8				
50+25.00	10	58+84.30	10.0	444.9	889.8				
MISCELLA	NEC	DUS AREAS							
Intersecting Str 1 e	ach		2.4	68.5	136.9				
Intersecting Str 18		n	35.1	808.3	1616.5	497.9	210.5	210.5	
Driveways / Parking	Lots	-11 each	0.9			75.7			
Driveways / Parking	Lots	-7 each	0.8			65.5	57.9		
Driveways / Parking			1.3			106.2			
Driveways / Parking	Lots	-12 each	0.5			45.0			
						4.0			
47+05.5		47+06.6	0.4			1.6	2.0	2.0	2.0
47+16.6		47+21.0	0.4			37.4	7.9	7.9 1.8	7.9 1.8
47+31.6	to	47+32.6				1.6	1.8	Ι.Ծ	Ι.δ
	I	Subtotals =			3,762.3	830.9	280.1	222.2	11.7
		Totals =	72.1	1,731.3	4,293	3.2	1	514.0	

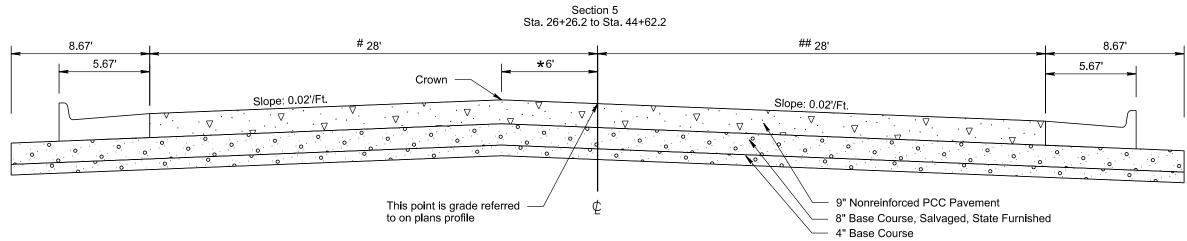
INTERSECTING STREETS AND DRIVEWAYS / PARKING LOTS

Intersecting Streets			Driveways / Parking Lots					
PCCP Only – 1 each	PCCP & AC – 18 each		Granular Materi	al Only – 11 each	AC & Granular Material – 7 each	8" PCCP only – 6 each	6" PCCP O	nly – 12 each
Sta. 44+14 Lt	Sta. 12+97 Lt.	Sta. 39+19 Lt.	Sta. 10+70 Lt.	Sta. 57+49 Rt.	Sta. 9+92 Lt.	Sta. 27+14 Lt.	Sta. 9+92 Rt.	Sta. 16+66 Rt.
	Sta. 12+97 Rt.	Sta. 39+19 Rt.	Sta. 14+76 Lt.		Sta. 25+70 Rt.	Sta. 42+57 Rt.	Sta. 10+42 Rt.	Sta. 21+56 Rt.
	Sta. 17+37 Lt.	Sta. 44+14 Rt.	Sta. 15+38 Lt.		Sta. 28+24 Lt.	Sta. 48+73 Lt.	Sta. 11+01 Rt.	
	Sta. 17+37 Rt.	Sta. 49+37 Lt.	Sta. 16+08 Lt.		Sta. 39+86 Rt.	Sta. 52+22 Lt.	Sta. 11+39 Rt.	
	Sta. 26+09 Lt.	Sta. 54+40 Lt.	Sta. 37+54 Rt.		Sta. 43+18 Lt.	Sta. 56+57 Lt.	Sta. 11+66 Rt.	
	Sta. 26+09 Rt.	Sta. 54+40 Rt.	Sta. 46+16 Rt.		Sta. 46+20 Lt.	Sta. 57+13 Rt.	Sta. 11+95 Lt.	
	Sta. 30+37 Lt.	Sta. 58+10 Lt.	Sta. 46+85 Lt.		Sta. 49+54 Rt.		Sta. 14+06 Rt.	
	Sta. 30+39 Rt.	Sta. 58+10 Rt.	Sta. 47+25 Rt.				Sta. 14+39 Rt.	
	Sta. 34+79 Lt.		Sta. 55+29 Lt.				Sta. 14+82 Rt.	
	Sta. 34+79 Rt.		Sta. 56+75 Rt.				Sta. 15+12 Rt.	



TYPICAL SURFACING SECTIONS





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S	SOUTH DAKOTA		NH-CR 0013(158)126	F7	F27
	Plotting [Date:	02/07/2024		
▽		0 0	Transitions: Sta. 25+90.9 to Sta ** 27.5' to 28' Sta. 25+65.7 to Sta * Crown Pt - 5.5' Rt	. 26+46	.2



Sta. 41+60 to Sta. 42+95 * Crown Pt - 6' to 2.5' # 28' to 24.5' ## 28' to 31.5'

Sta. 42+95 to Sta. 43+93.3 # 24.5' ## 31.5'

Sta. 43+93.3 to Sta. 44+62.2 # 24.5' to 34' ## 31.5' to 31'

Sta. 42+95 to Sta. 43+98 * Crown Pt - 2.5'

Sta. 43+93.3 to Sta. 44+46 * Crown Pt - 2.5' to 2'

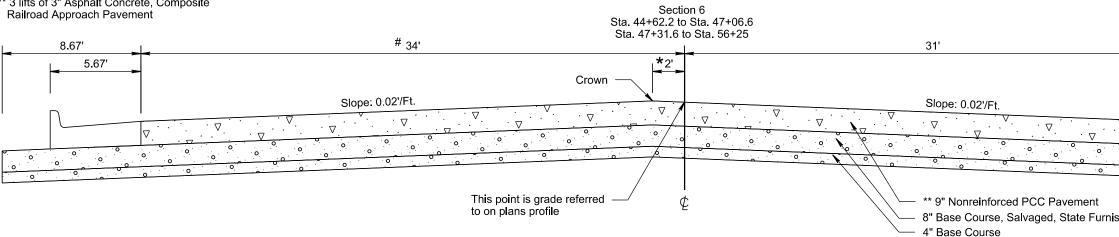
Sta. 44+46 to Sta. 44+62.2 * Crown Pt - 2'

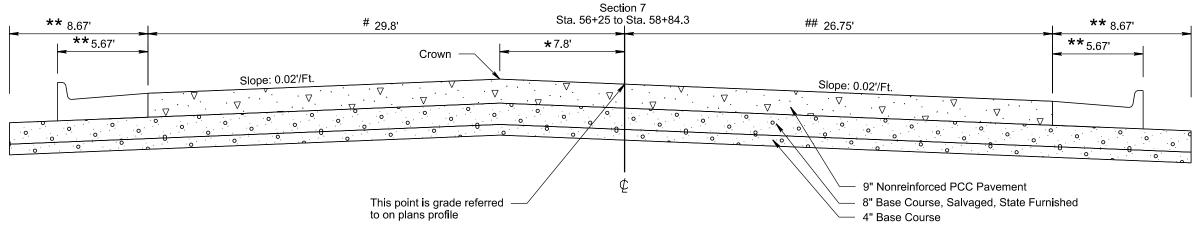
TYPICAL SURFACING SECTION

Surfacing Exception for Railroad Tracks: 47+06.6 to 47+31.6

Transitions:

Sta. 47+05.5 to Sta. 47+06.6 Sta. 47+16.6 to Sta. 47+21.0 Sta. 47+31.6 to Sta. 47+32.6 ** 3 lifts of 3" Asphalt Concrete, Composite Railroad Approach Pavement





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v v v v v v v v v v v v v v v v v v v	Plotting 8.67 5.67'	Date: 02/07	/2024 Transitions: Sta. 46+49.7 tr # 34' to 32' Sta. 46+99.7 tr Sta. 47+31.6 tr # 32' Sta. 47+61.9 tr # 32' to 34' Sta. 52+10 to * Crown Pt - 2' Sta. 53+75 to * Crown Pt - 1;	o Sta. 4 o Sta. 4 o Sta. 4 Sta. 53+ ' to 12' Sta. 56+	7+06.6 7+61.9 8+11.9 ⊦75
					L SECTIONS, DGN

Transitions:

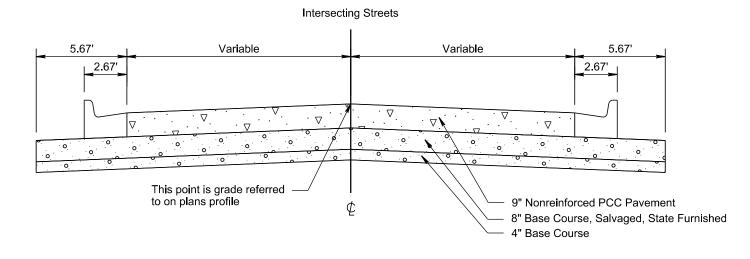
Sta. 56+25 to Sta. 57+50 * Crown Pt - 12' to 7.8' # 34' to 29.8' ## 31' to 26.75'

Sta. 57+93.3 to Sta. 58+26.6 # 29.8' to 29.3' ## 26.75' to 26.1'

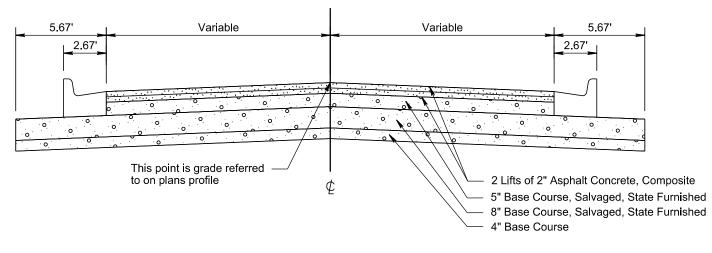
Sta. 58+26.6 to Sta. 58+84.3 ** 0' # 29.3' to 31.1' ## 26.1' to 28.5'

Sta. 58+43.4 to Sta. 58+84.3 * Crown Pt - 7.8' to 9.1'

TYPICAL SURFACING SECTIONS



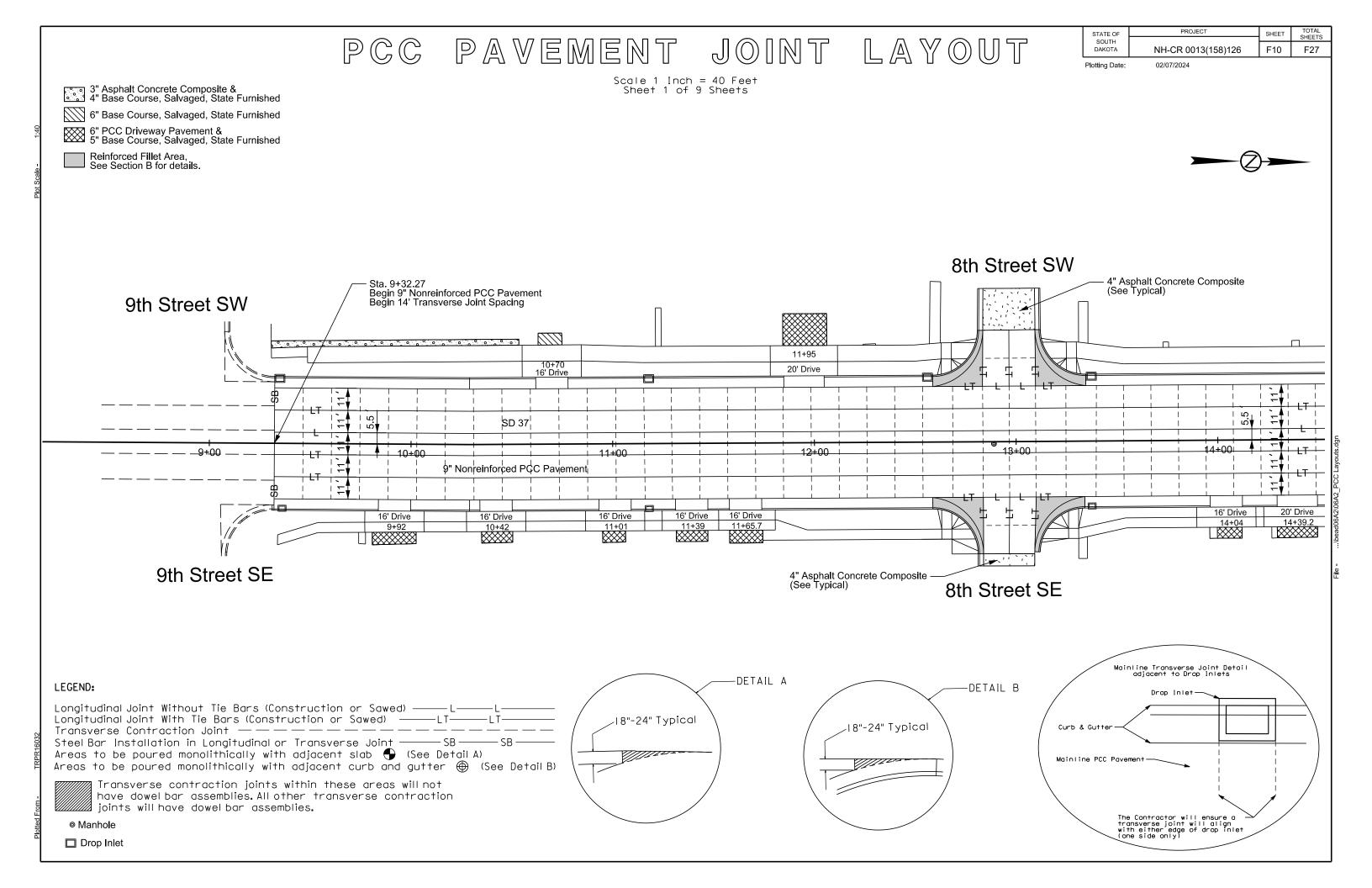
Intersecting Streets

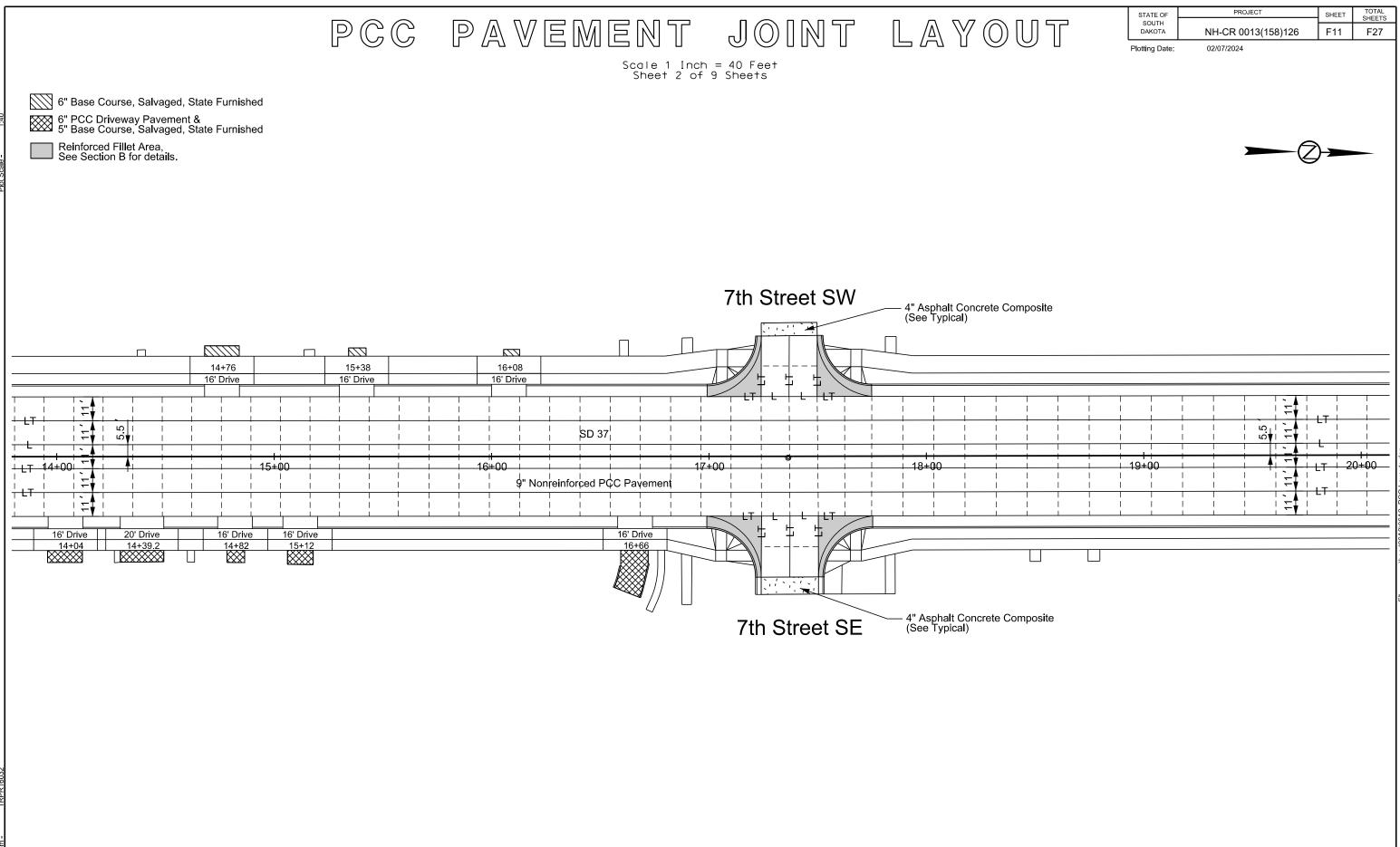


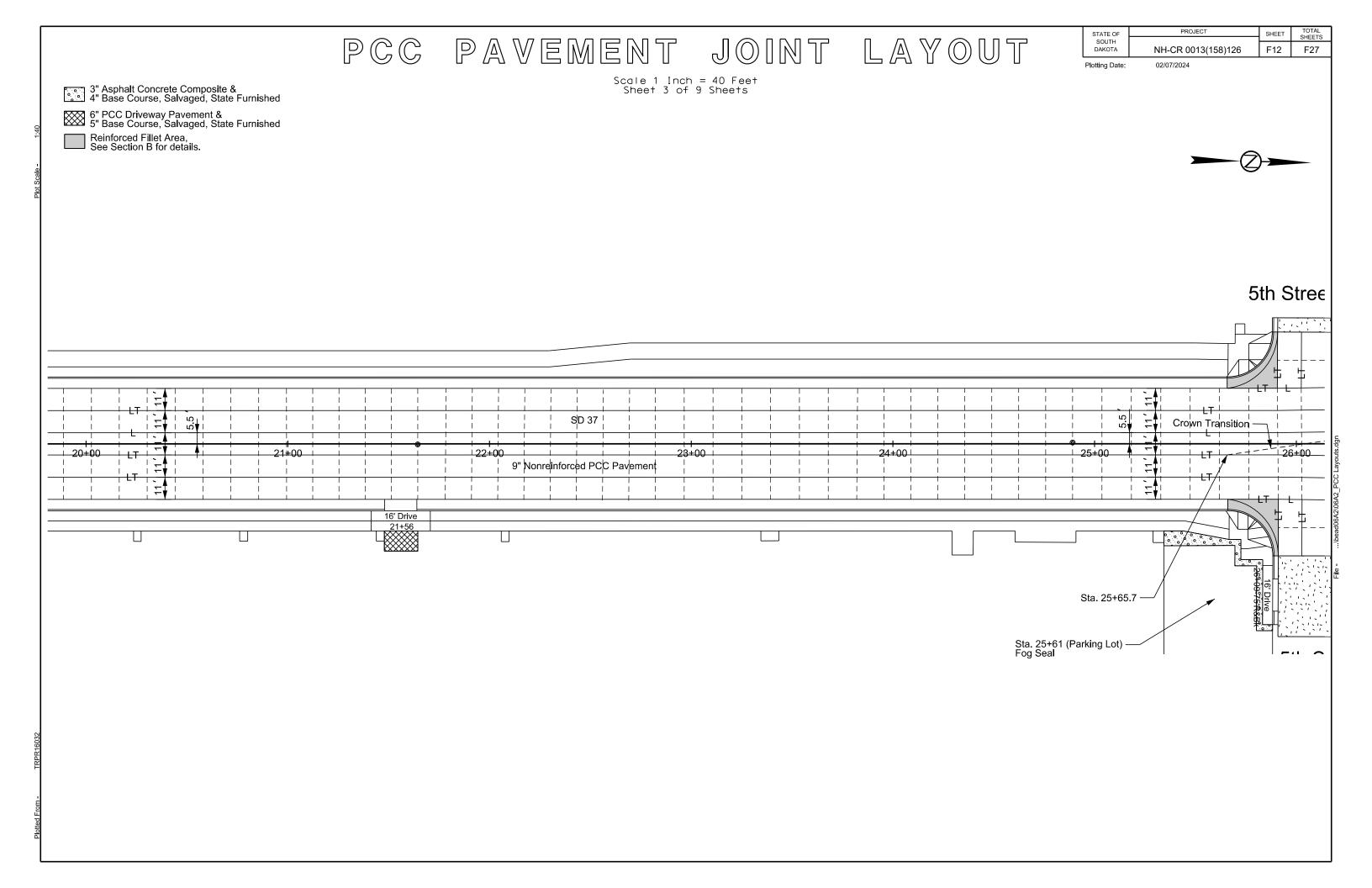
6	5	
\leq	5	
C	シ	

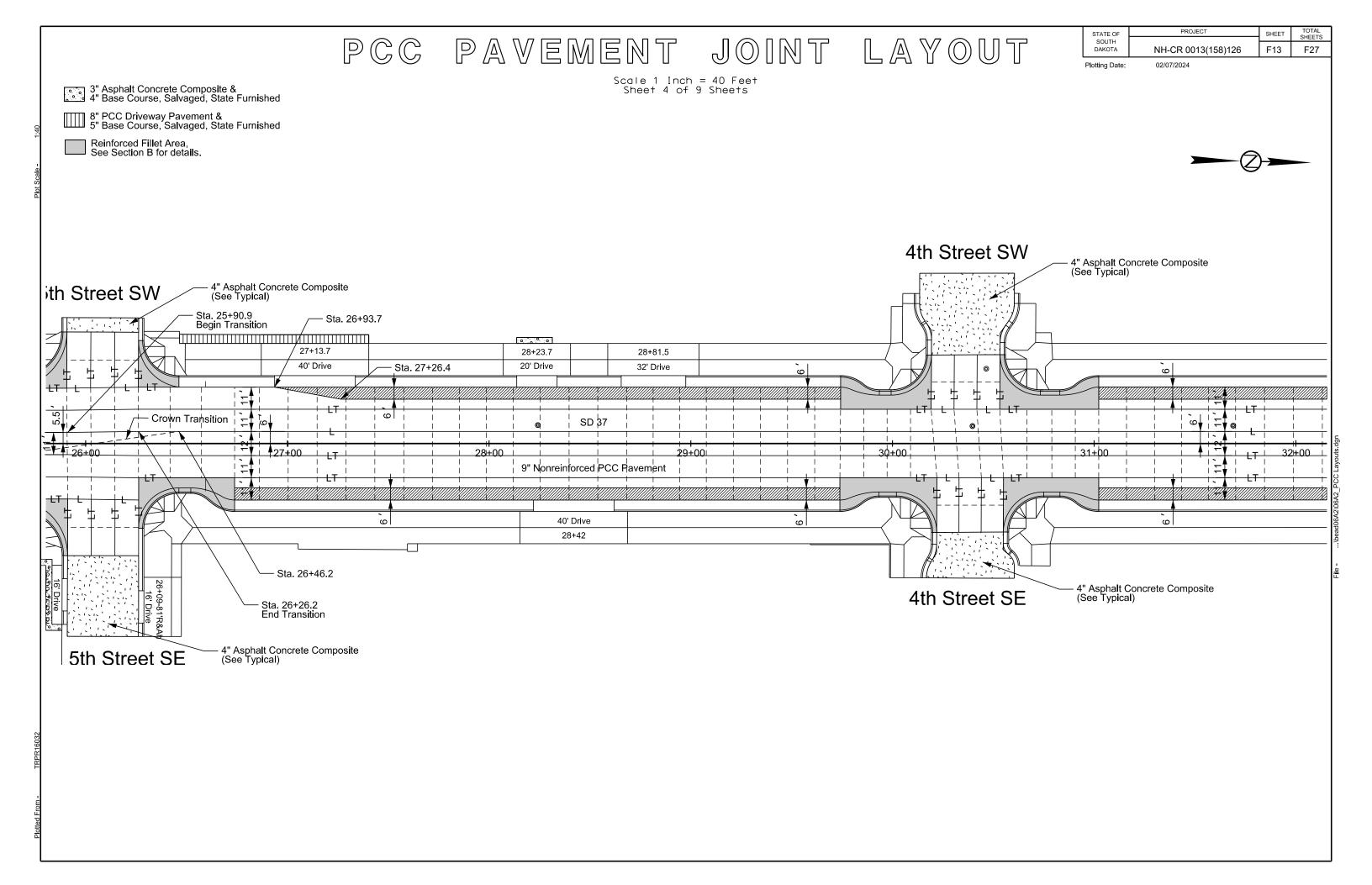
STATE OF	PROJECT	SHEET	TOTAL SHEETS
SOUTH			SHEETS
DAKOTA	NH-CR 0013(158)126	F9	F27
Plotting (Date: 02/07/2024		

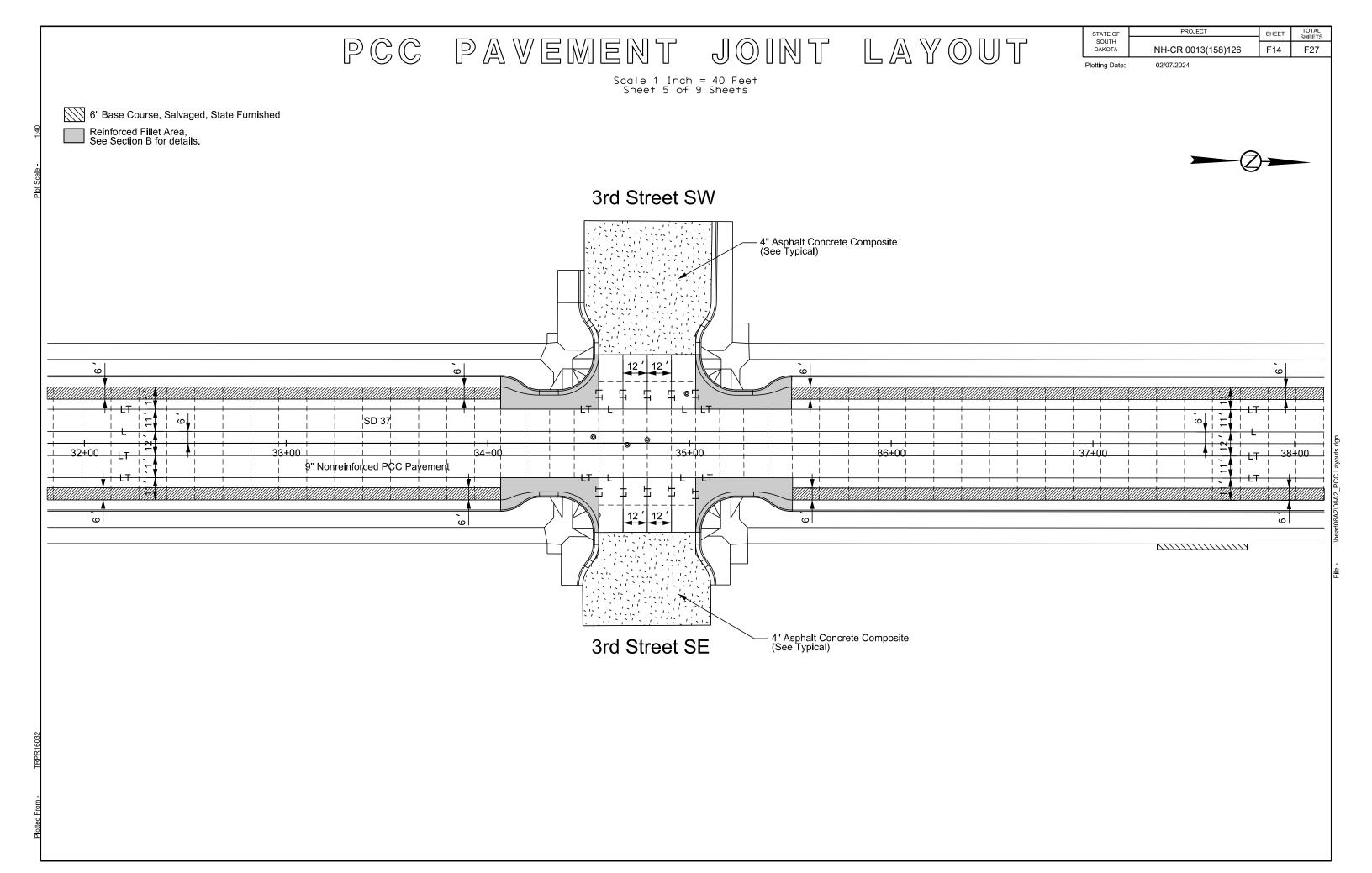
LOT NAME - 5

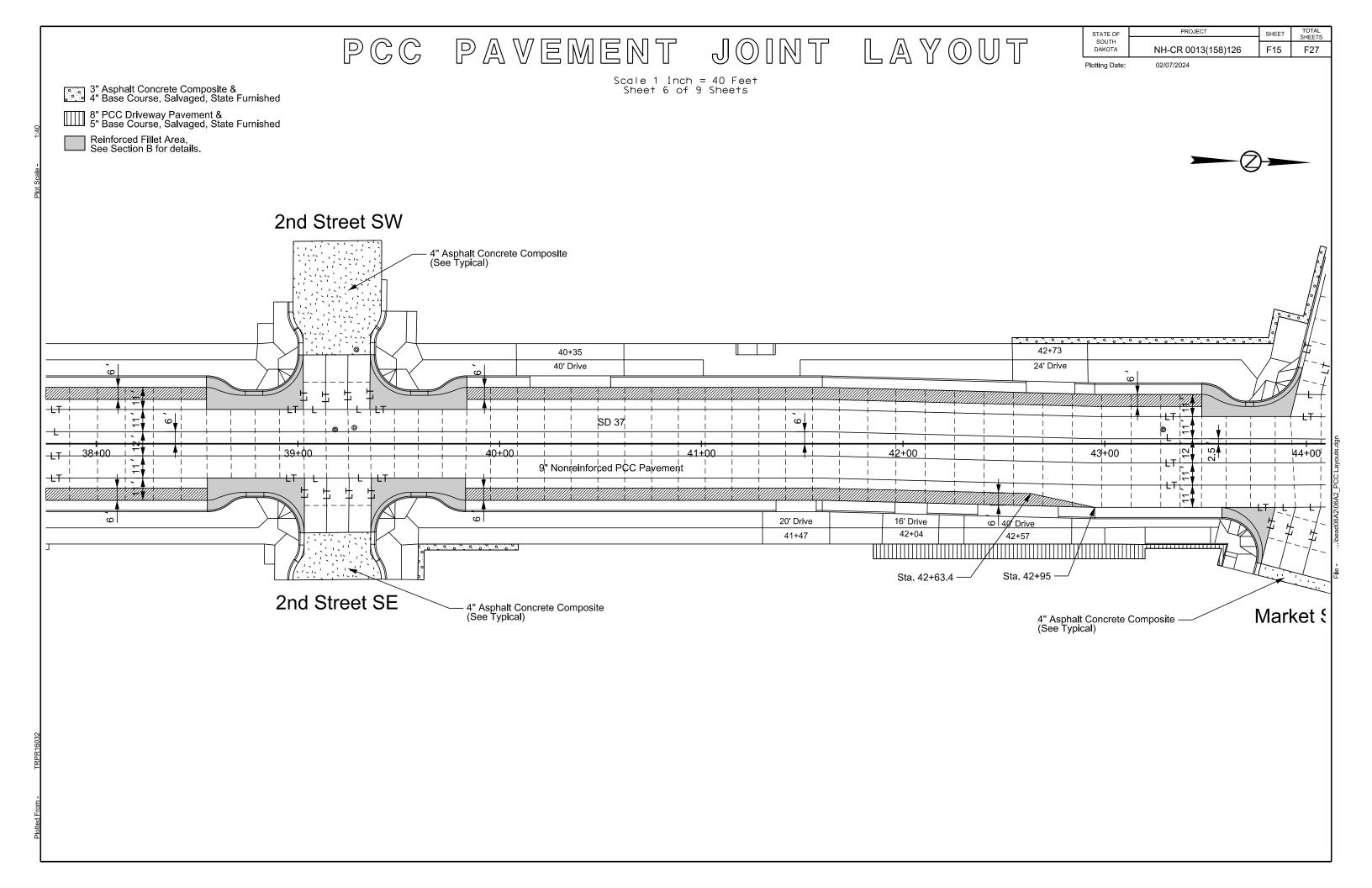


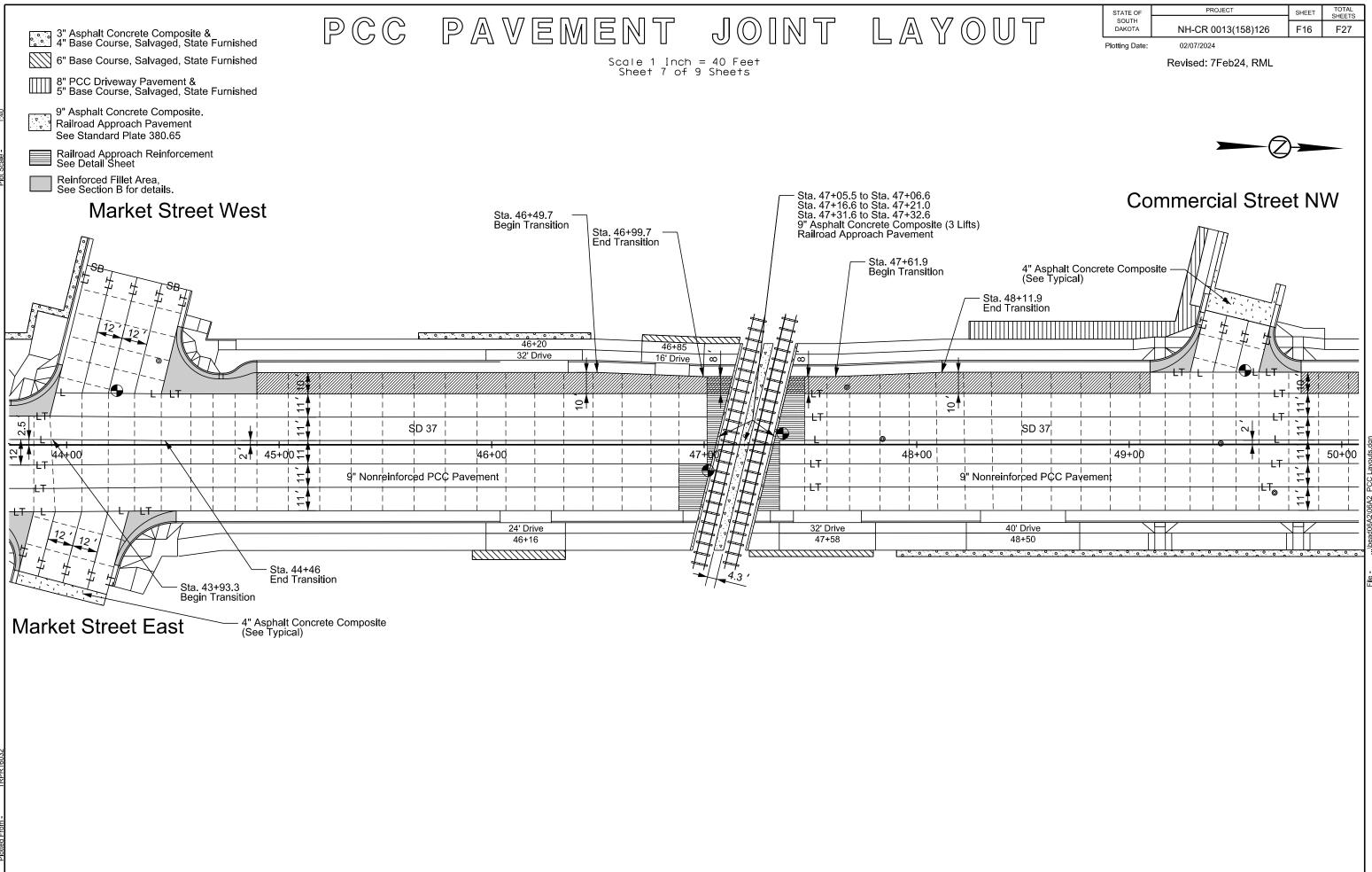


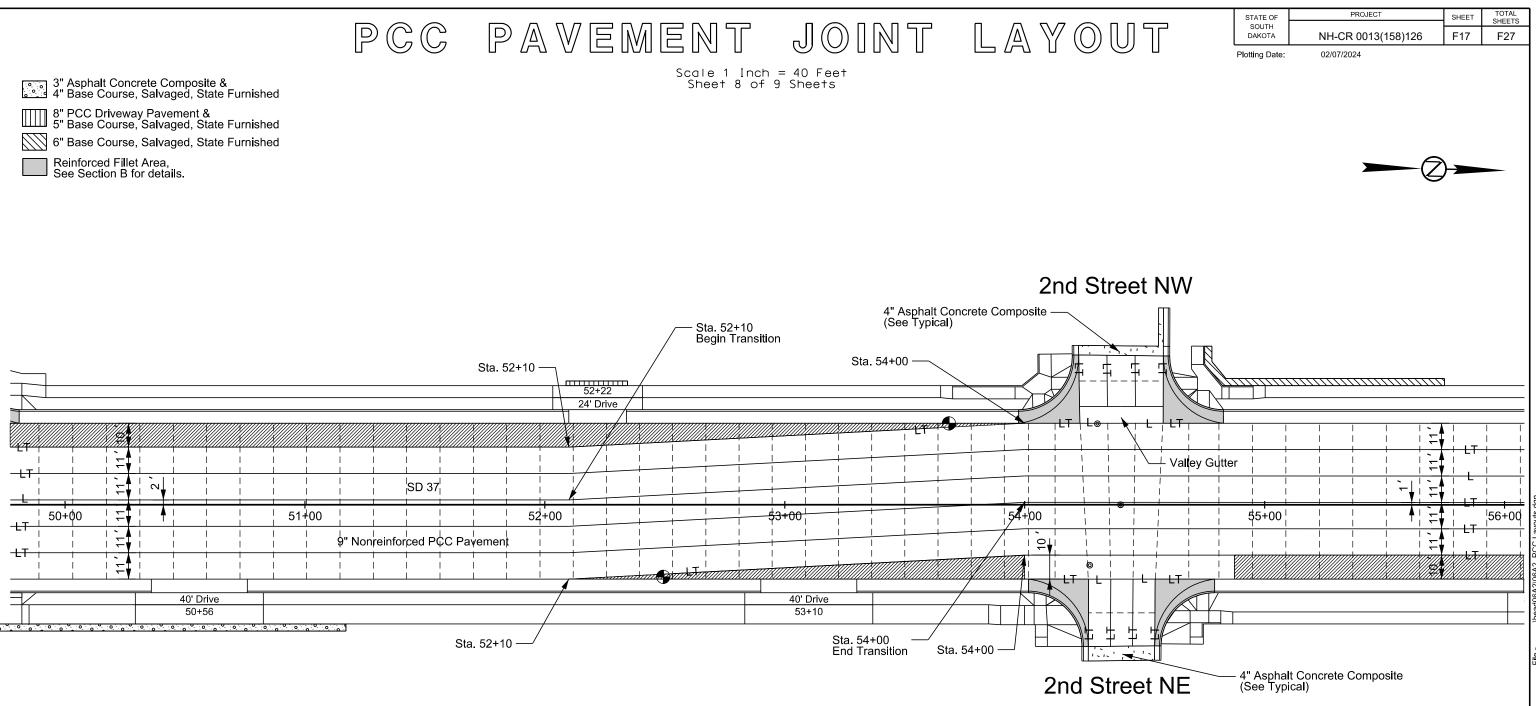


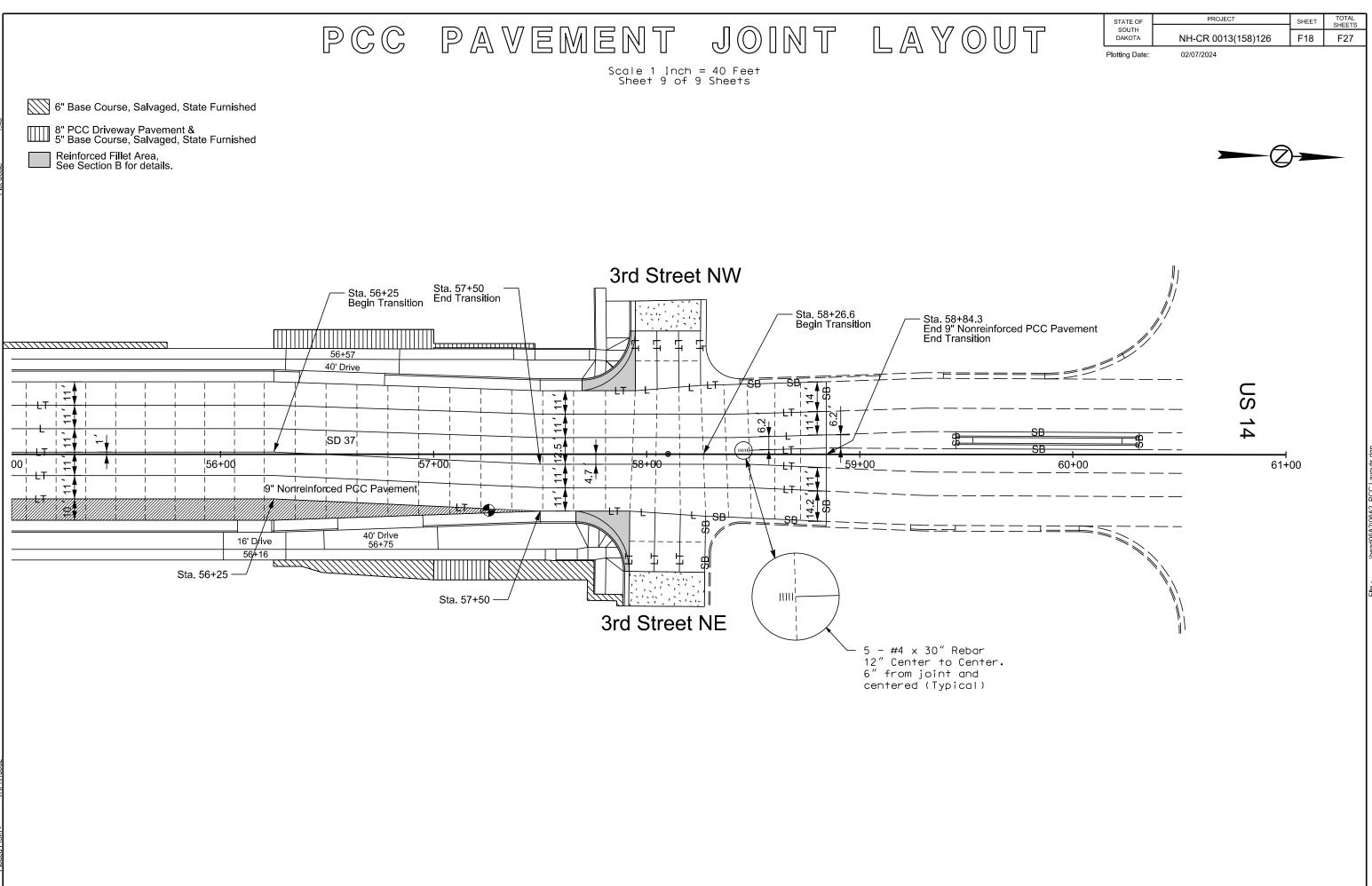


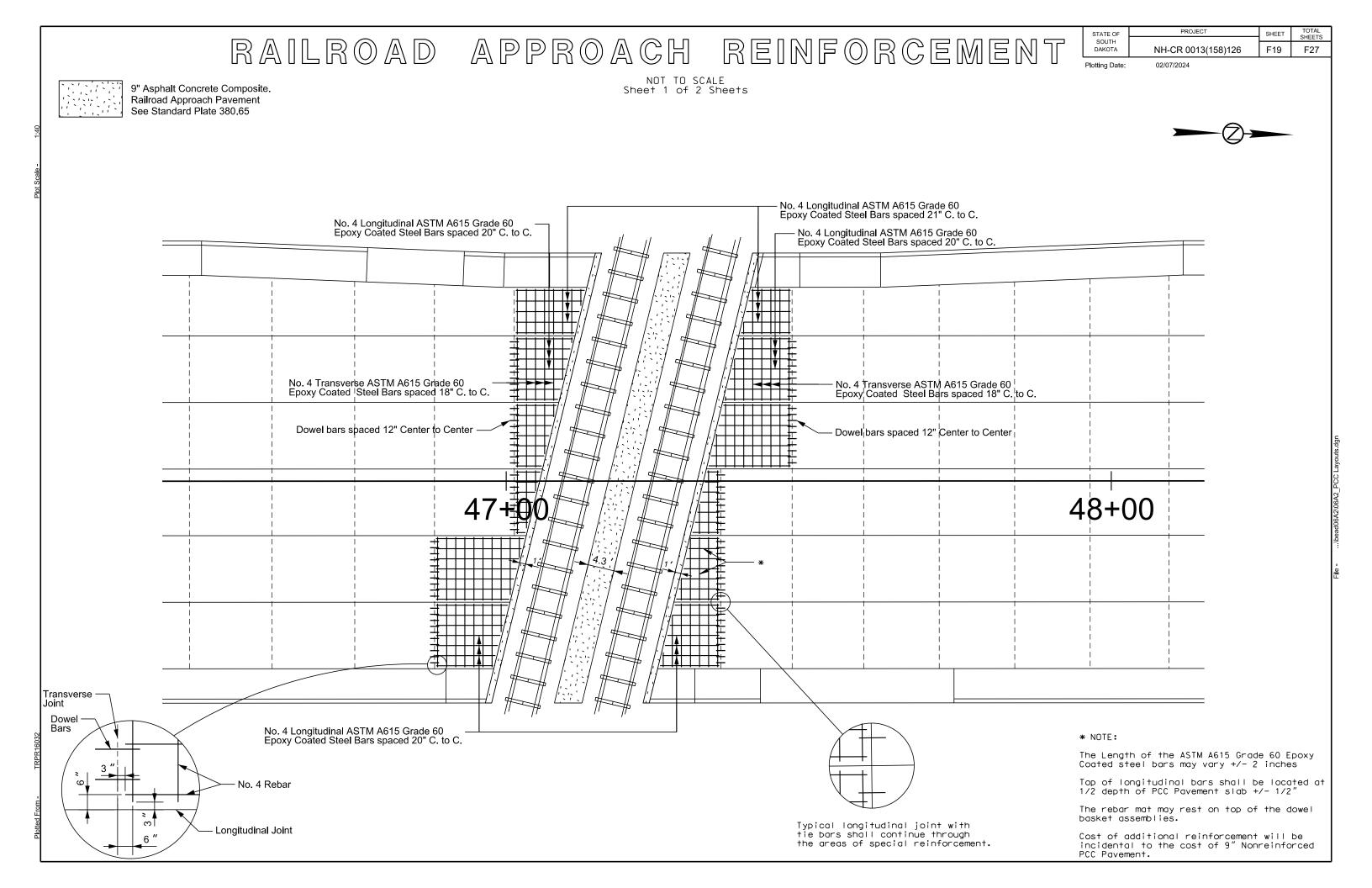


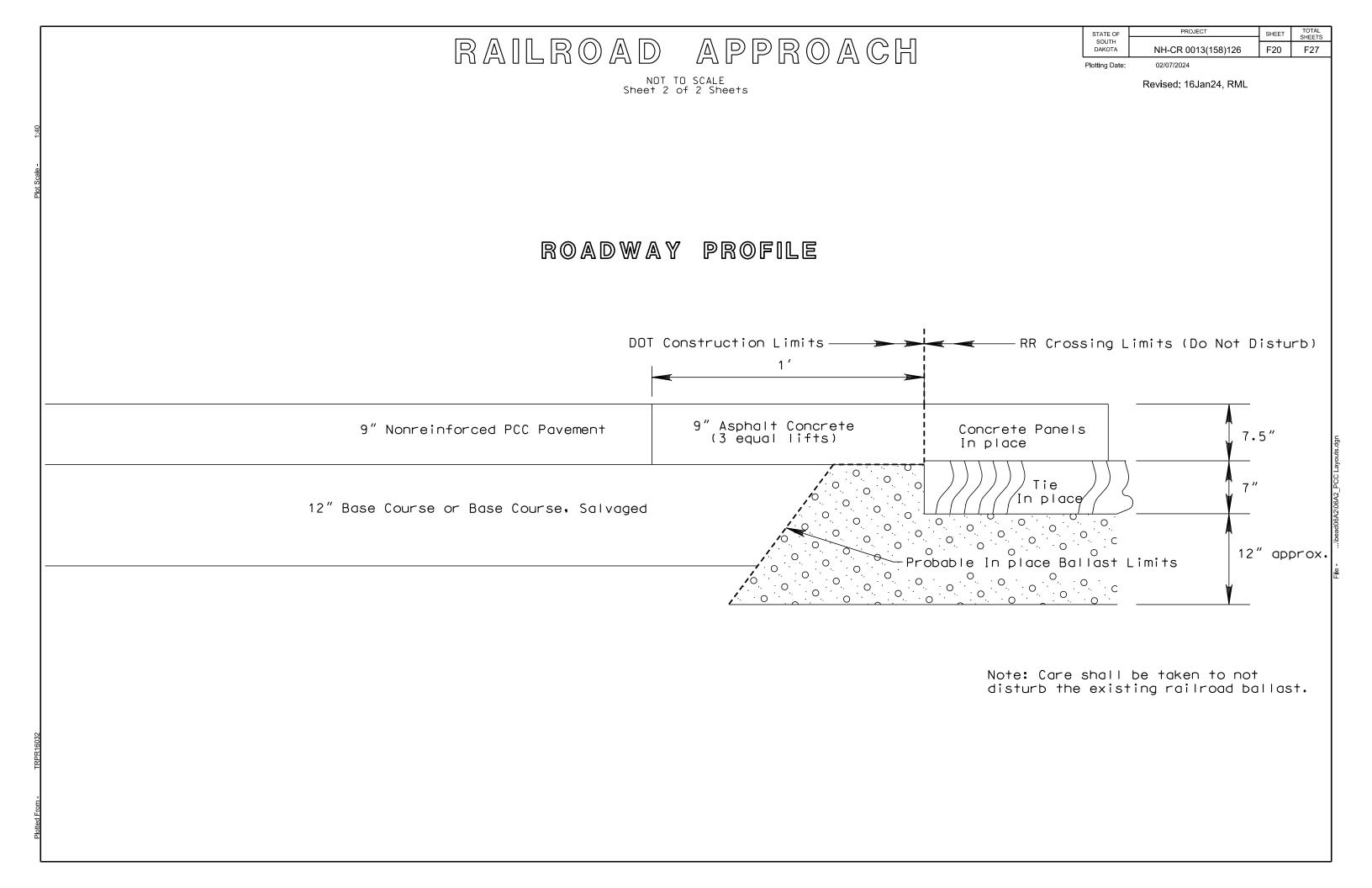


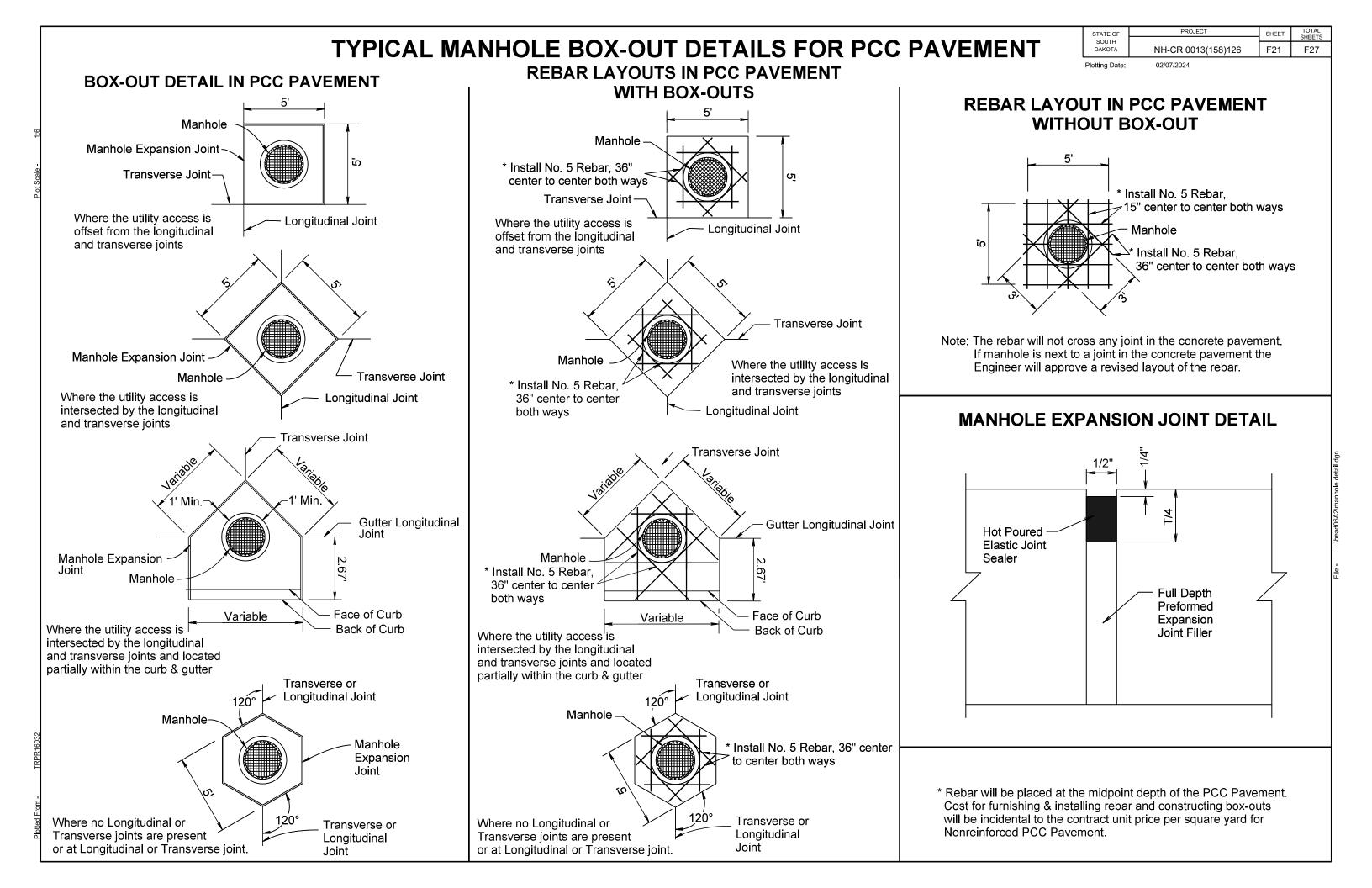


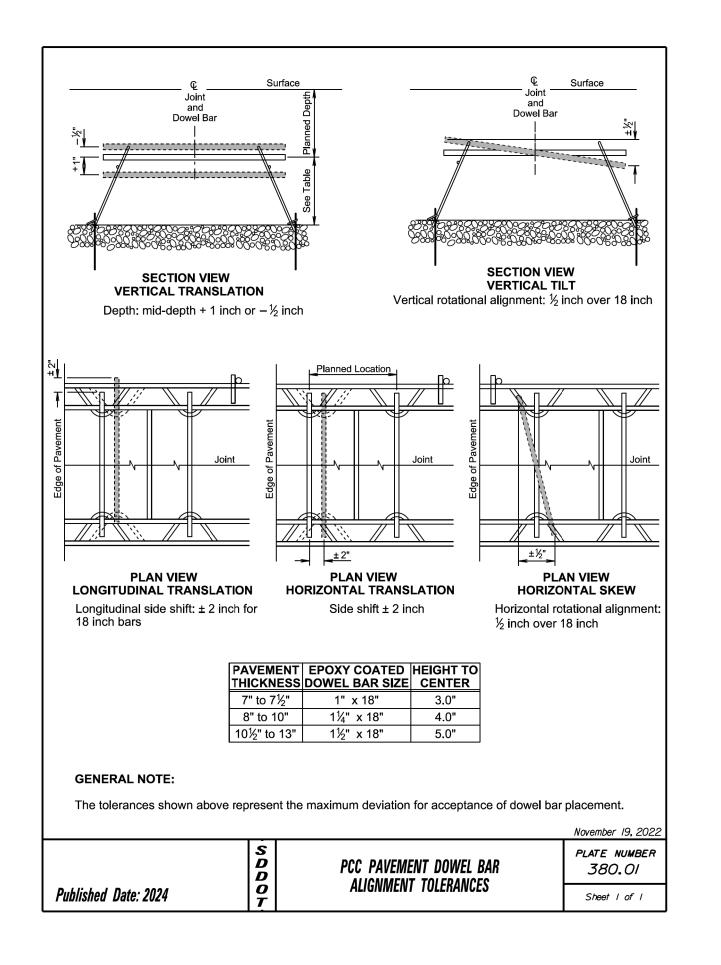


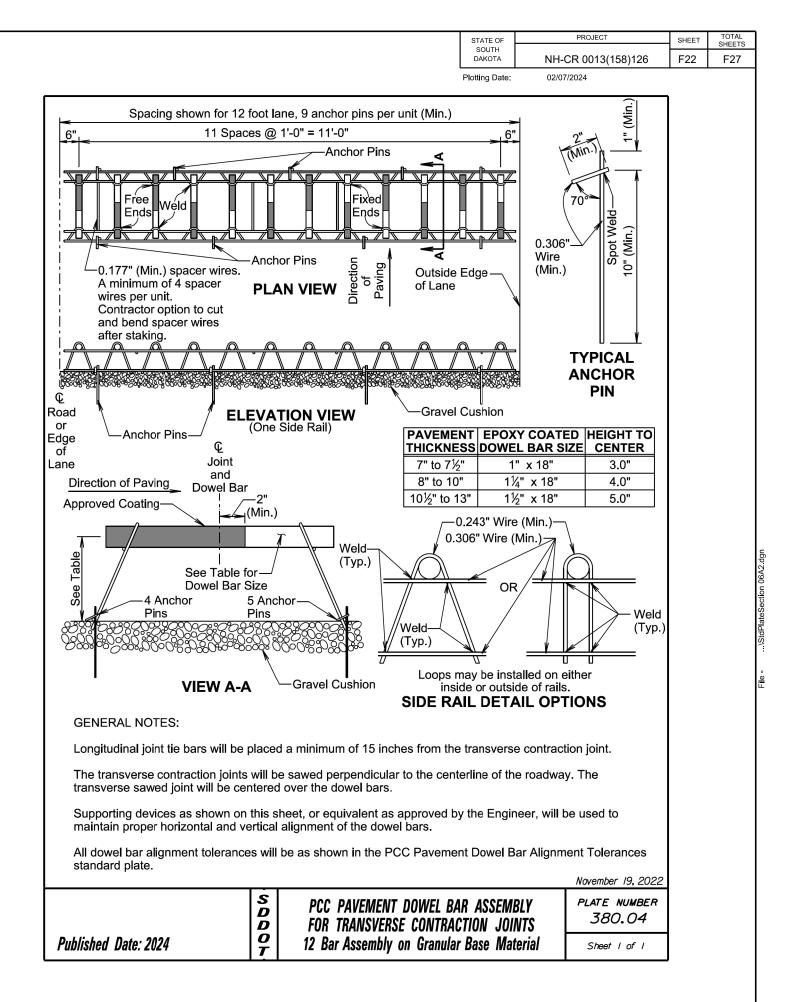


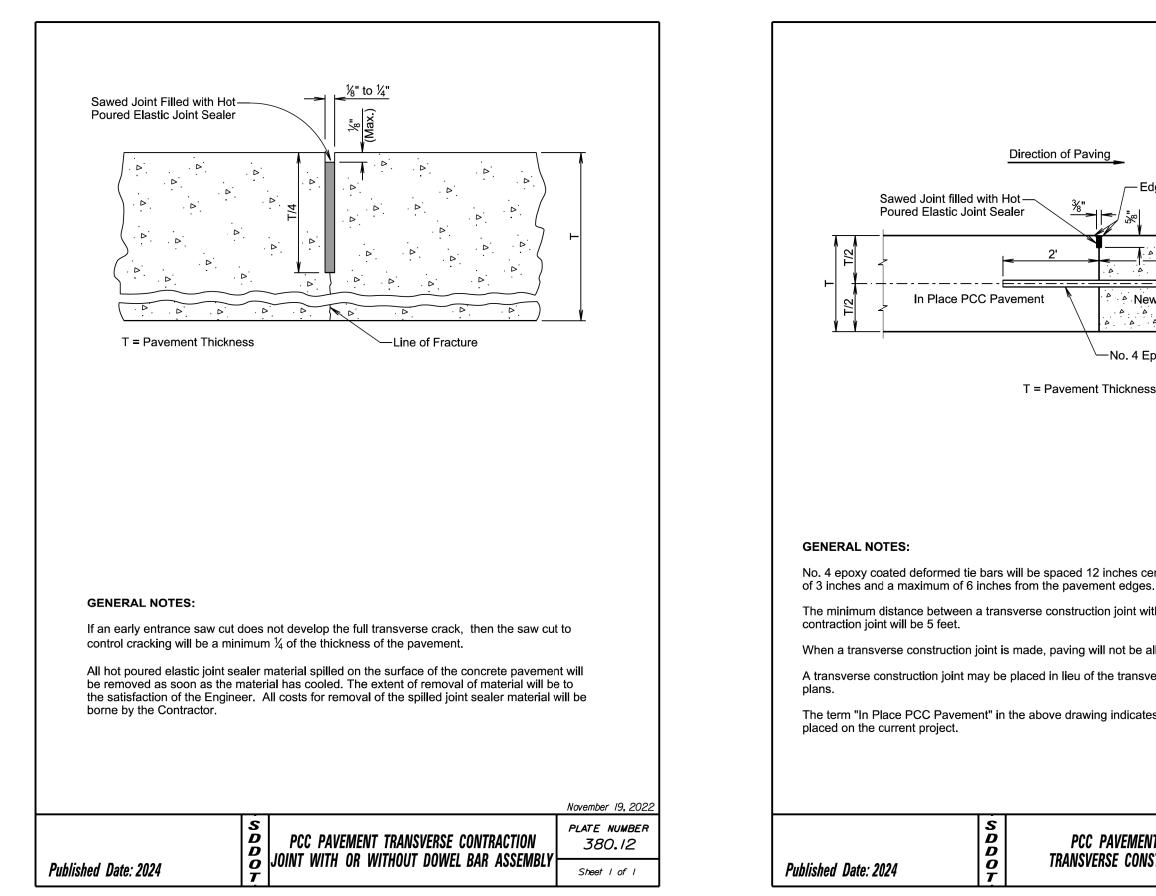


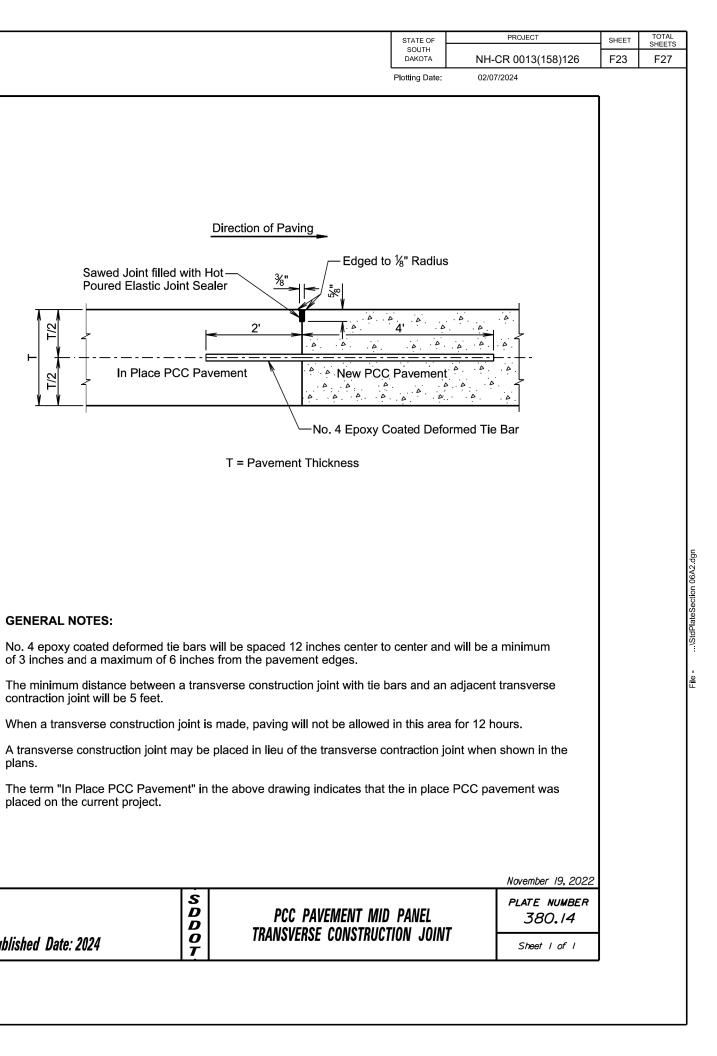


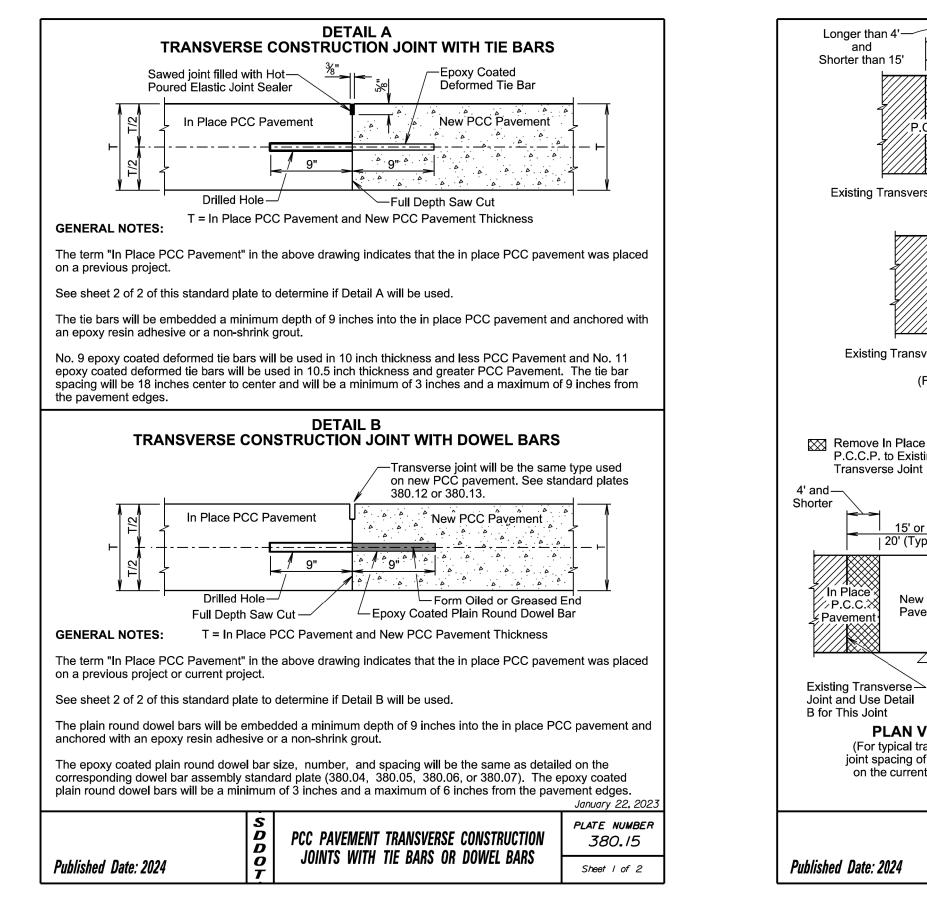


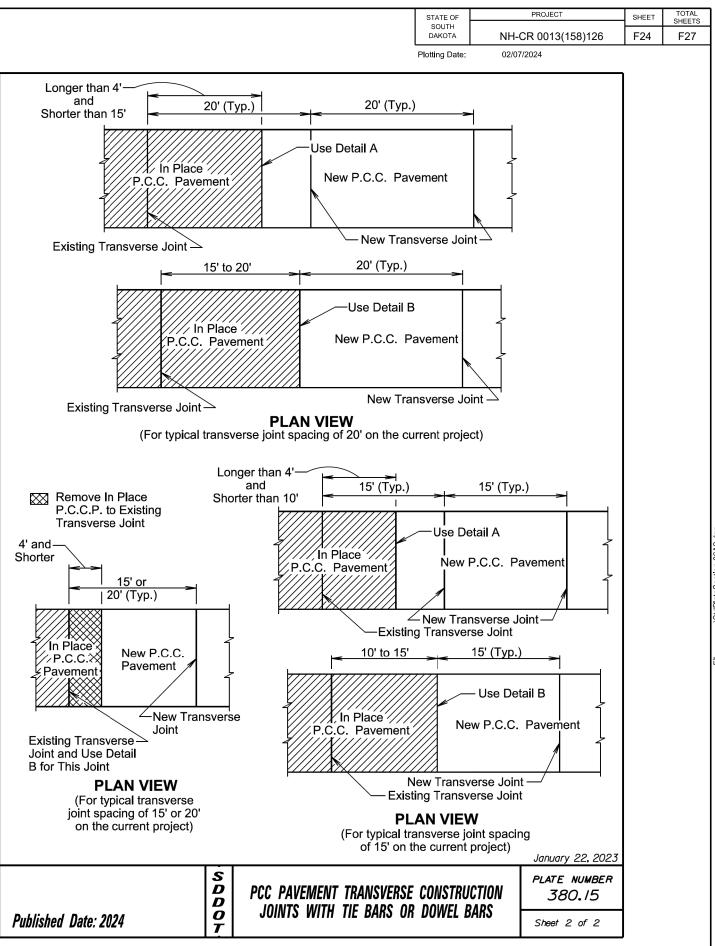




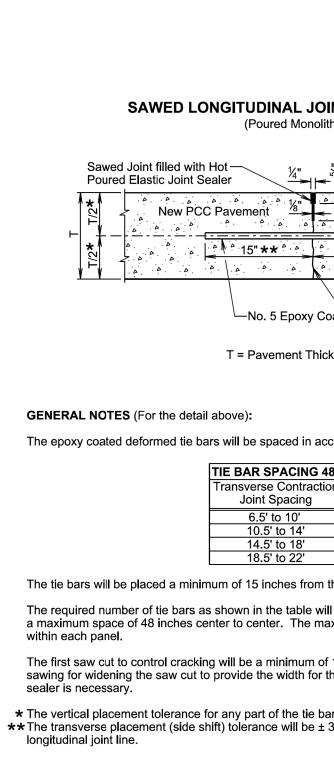






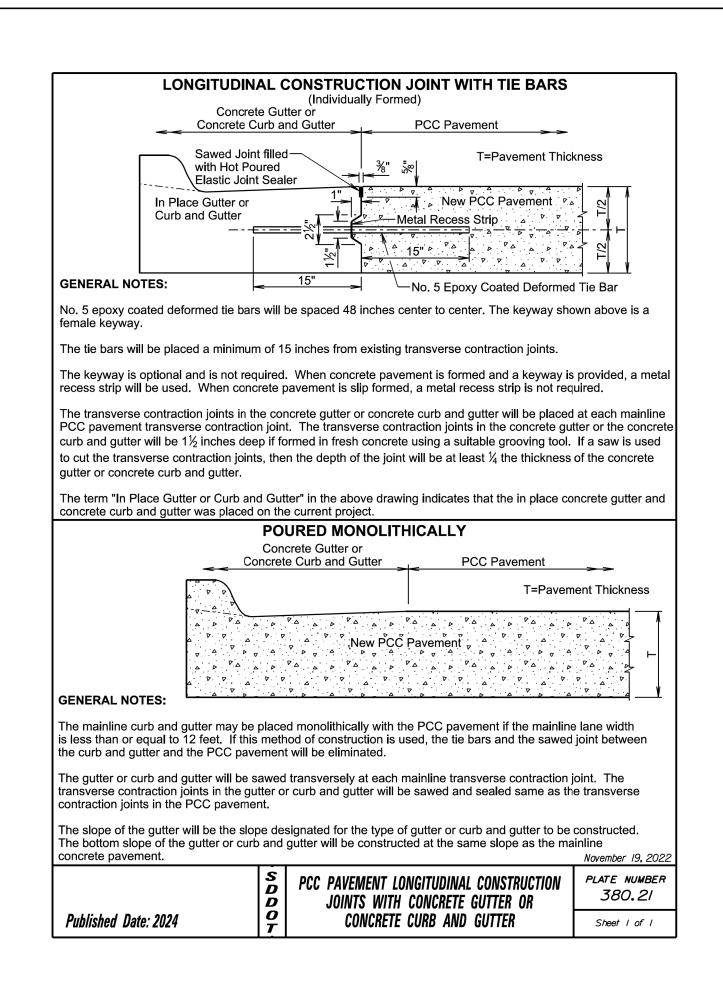


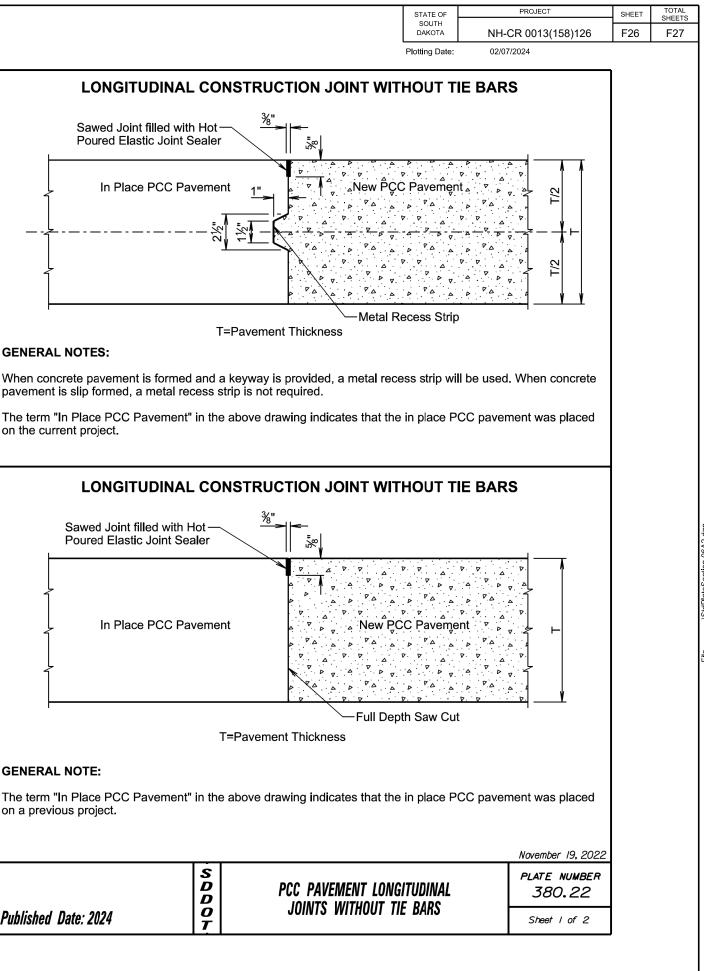
LONGITUDINAL	CONSTRUCTION JOINT WITH TIE BARS	
Sawed Joint filled with F Poured Elastic Joint Se		lickness
	sproject Metal Recess Strip	
	CONSTRUCTION JOINT WITH TIE BARS (Inserted or Formed in Bars)	
Sawed Joint filled with H Poured Elastic Joint Se		
H Solice Literie conin co In place PCC Pave placed on the curr project ★	ement ent 15" ** 15" **	
GENERAL NOTES (For the details abov	re):	ned Tie Bar
The epoxy coated deformed tie bars will	be spaced in accordance with the following tables:	
	TIE BAR SPACING 30" MAXIM	
TIE BAR SPACING 48" MA	Transverse Contraction Number Joint Spacing Tie B	
Transverse Contraction Nu	mber of 5' to 7' 2	
	e Bars 7.5' to 9.5' 3 10' to 12' 4	
6.5' to 10' 10.5' to 14'	2 3 12.5' to 14.5' 5	
14.5' to 18'	15' to 17' 6	
18.5' to 22'	4 17.5' to 19.5' 7 5 20' to 22' 8	
The tie bars will be placed a minimum	of 15 inches from transverse contraction joints.]
spaced tie bars will be spaced a maxin	own in the table will be uniformly spaced within each par num of 48 inches center to center for a female keyway a r to center for a vertical face and male keyway. The ma ch panel.	and will be
The keyway illustrated in the above de	tails depict a female keyway.	
The keyway is optional and is not requ metal recess strip will be used. When	ired. When concrete pavement is formed and a keyway concrete pavement is slip formed, a metal recess strip i	/ is provided, a s not required.
 ★ The vertical placement tolerance for ar ★★The transverse placement (side shift) t longitudinal joint line. 	by part of the tie bar will be \pm T/6. olerance will be \pm 3 inches when measured perpendicu	lar to the November 19, 202
S D D	PCC PAVEMENT LONGITUDINAL JOINTS WITH TIE BARS	plate number 380.20
Published Date: 2024	JUNNIS WIIN HE DANS	Sheet I of 2



	S	
		PCC PA JOIN
Published Date: 2024		JUII

STATE OF SOUTH DAKOTA	PROJECT	SHEET	TOTAL
DAKOTA			SHEETS
	NH-CR 0013(158)126	F25	F27
E BARS	02/07/2024		
contraction j	oints.		
spacing will ss of the pa of the hot po	apply to tie bars vement. Additional ured elastic joint erpendicular to the	<u>,</u>	
	E BARS	E BARS	E BARS



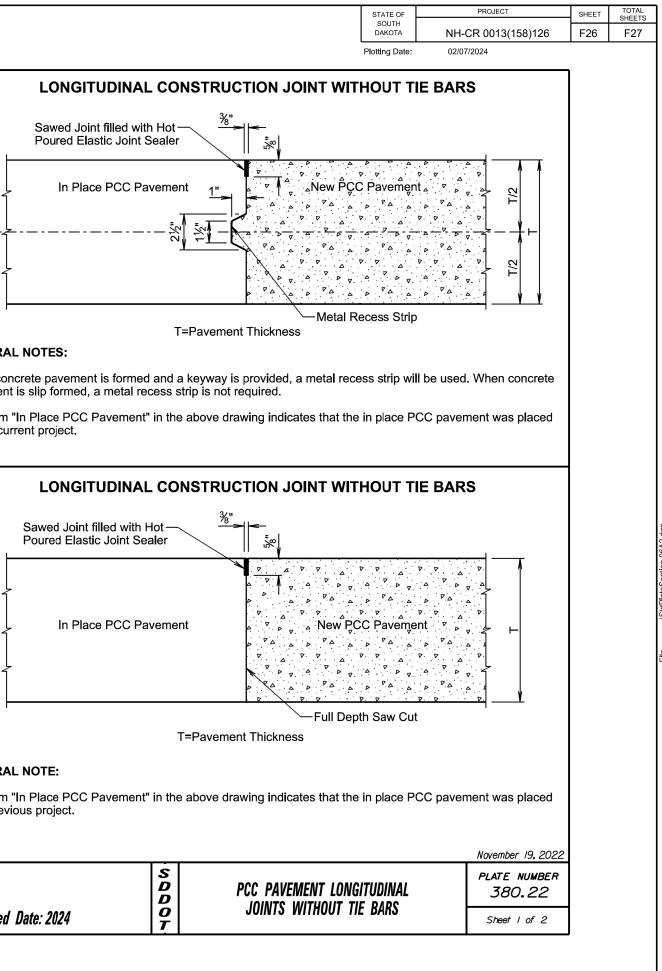




GENERAL NOTES:

pavement is slip formed, a metal recess strip is not required.

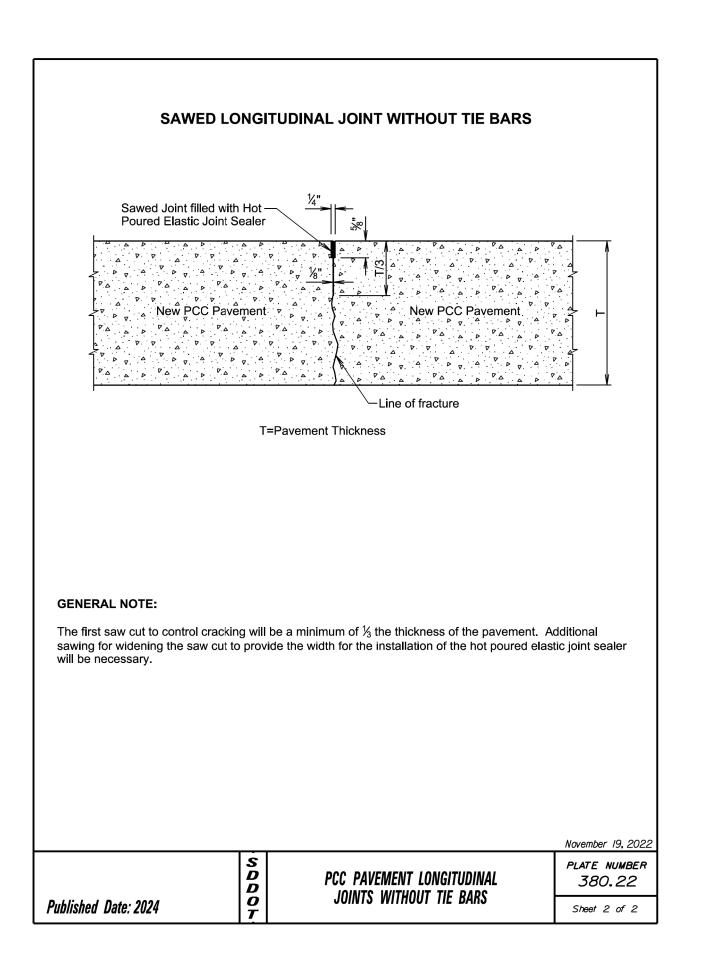
on the current project.

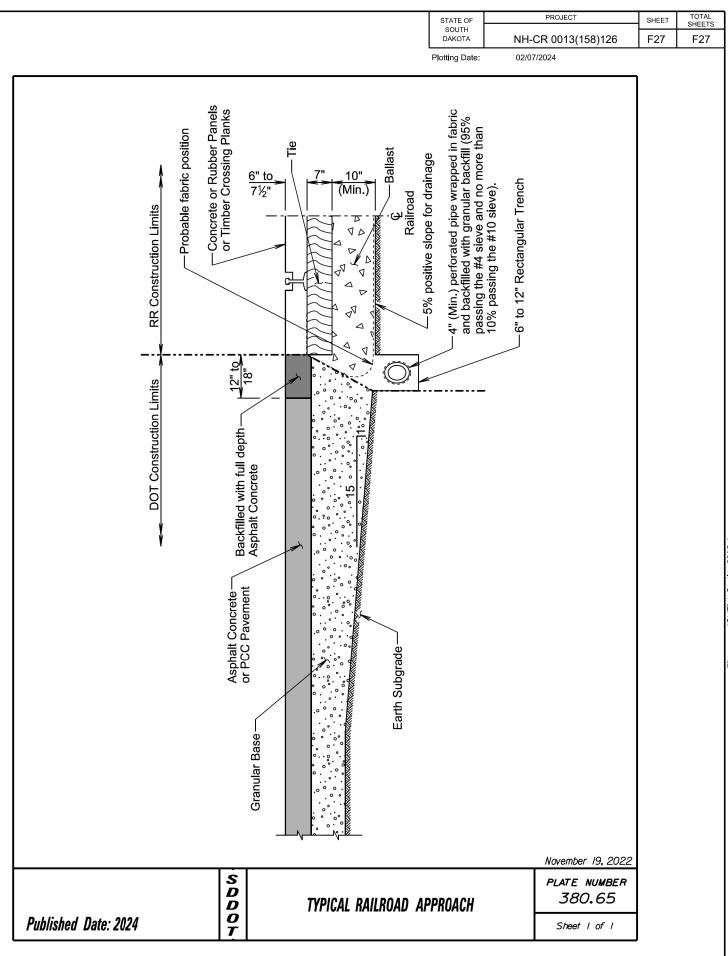


GENERAL NOTE:

on a previous project.

Published Date: 2024	S D D 0 T	PCC PA Joints





\StdPlateSection 06A2