

#### **SECTION B ESTIMATE OF QUANTITIES**

BID ITEM NUMBER	ITEM	QUANTITY	UNIT
009E0010	Mobilization	Lump Sum	LS
009E3230	Grade Staking	0.334	Mile
009E3250	Miscellaneous Staking	0.300	Mile
009E3301	Engineer Directed Surveying/Staking	174.0	Hour
110E0300	Remove Concrete Curb and/or Gutter	919	Ft
110E0400	Remove Drop Inlet	7	Each
110E0460	Remove Manhole	2	Each
110E0480	Remove Manhole Frame and Lid	2	Each
110E0520	Remove Sewer Pipe	298	Ft
110E0530	Remove Storm Sewer Pipe	750	Ft
110E1010	Remove Asphalt Concrete Pavement	4,185.0	SqYd
110E1100	Remove Concrete Pavement	36.4	SqYd
110E1130	Remove Concrete Driveway Pavement	162.0	SqYd
110E1140	Remove Concrete Sidewalk	219.0	SqYd
110E5800	Salvage Fence	202	Ft
120E0010	Unclassified Excavation	2,542	CuYd
120E2000	Undercutting	1,104	CuYd
120E6100	Water for Embankment	15.0	MGal
250E0020	Incidental Work, Grading	Lump Sum	LS
380E4010	6" PCC Fillet Section	155.9	SqYd
450E7666	66" Steel Pipe, Furnish	180	Ft
451E1004	4" PVC Sewer Pipe	146	Ft
451E5166	Bore and Jack 66" Pipe	180	Ft
451E6080	Adjust Water Valve Box	7	Each
451E6102	Water Main Lowering	5	Each
451E6105	Connect To Existing Water Main	10	Each
451E7005	Adjust Sewer Service	1	Each
451E7010	Reconnect Sewer Service	3	Each
451E7016	Connect to Existing Sewer Main	1	Each
451E7500	Locate Utilities	2	Each
462E0250	Cellular Grout	130.6	CuYd
600E0200	Type II Field Laboratory	1	Each
620E3000	Install Fence	71	Ft
650E0060	Type B66 Concrete Curb and Gutter	1,126	Ft
650E4660	Type P6 Concrete Gutter	192	Ft
650E6260	6" Concrete Valley Gutter	13.0	SqYd
670E1010	2' x 3' Type B Drop Inlet	12	Each
670E1030	5.5' x 3' Type B Drop Inlet	2	Each
670E1200	Type B Frame and Grate	16	Each
671E0550	Special Manhole	2	Each
671E1072	72" Manhole	4	Each
671E1096	96" Manhole	5	Each
671E1120	120" Manhole	1	Each
671E6007	Type A7 Manhole Frame and Lid	11	Each
671E7000	Reset Manhole Frame and Lid	2	Each
671E7010	Adjust Manhole	2	Each
998E0100	Railroad Protective Insurance	Lump Sum	LS

<sup>\* -</sup> Denotes Non-Participating

#### SECTION B ESTIMATE OF QUANTITIES CONTINUED

#### Section B - Grading - PCN 09V7 Storm Sewer Material - Alternate A - RCP

BID ITEM NUMBER	ITEM	QUANTITY	UNIT
450E0112	15" RCP Class 2, Furnish	226	Ft
450E0120	15" RCP, Install	226	Ft
450E0122	18" RCP Class 2, Furnish	26	Ft
450E0130	18" RCP, Install	26	Ft
450E0182	36" RCP Class 2, Furnish	46	Ft
450E0190	36" RCP, Install	46	Ft
450E0202	48" RCP Class 2, Furnish	448	Ft
450E0210	48" RCP, Install	448	Ft
450E0222	60" RCP Class 2, Furnish	682	Ft
450E0230	60" RCP, Install	682	Ft
450E2044	60" RCP Flared End, Furnish	1	Each
450E2045	60" RCP Flared End, Install	1	Each

#### Section B - Grading - PCN 09V7 Storm Sewer Material - Alternate B - HDPE

BID ITEM NUMBER	ITEM	QUANTITY	UNIT
450E5239	60" CMP Flared End, Furnish	1	Each
450E5240	60" CMP Flared End, Install	1	Each
450E7009	15" High Density Polyethylene Pipe, Furnish	226	Ft
450E7010	15" High Density Polyethylene Pipe, Install	226	Ft
450E7019	18" High Density Polyethylene Pipe, Furnish	26	Ft
450E7020	18" High Density Polyethylene Pipe, Install	26	Ft
450E7049	36" High Density Polyethylene Pipe, Furnish	46	Ft
450E7050	36" High Density Polyethylene Pipe, Install	46	Ft
450E7069	48" High Density Polyethylene Pipe, Furnish	158	Ft
450E7070	48" High Density Polyethylene Pipe, Install	158	Ft
450E7089	60" High Density Polyethylene Pipe, Furnish	974	Ft
450E7090	60" High Density Polyethylene Pipe, Install	974	Ft

#### **GRADING OPERATIONS**

Water for Embankment is estimated at the rate of 10 gallons of water per cubic yard of Embankment minus Waste.

TABLE OF WATER FOR EMBANKMENT				
		Area		Water
Location	Туре	(SF)	Depth	(Mgal)
Surfacing Grading	Undercut	40753.5	12"	15.0

#### **TYPE II FIELD LABORATORY**

The lab will be equipped with an internet connection such as DSL, cable modem, or other approved service. The internet connection will be provided with a multi-port wireless router. The internet connection will be a minimum speed of 5 Mbps unless limited by job location and approved by the DOT. Prior to installing the wireless router, the Contractor will submit the wireless router's technical data to the Area Office to check for compatibility with the state's computer equipment. The internet connection is intended for state personnel usage only. The Contractor's personnel are prohibited from using the internet connection unless pre-approved by the Project Engineer. These items will be incidental to the contract unit price per each for "Type II Field Laboratory".

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The Contractor will be aware that the existing utilities shown in the plans were surveyed prior to the design of this project and might have been relocated or replaced by a new utility facility prior to construction of this project, might be relocated or replaced by a new utility facility during the construction of this project, or might not require adjustment and may remain in its current location. The Contractor will contact each utility owner and confirm the status of all existing and new utility facilities. The utility contact information is provided elsewhere in the plans or bidding documents.

Montana-Dakota Utilities – Tanner Lewis – 701-456.7119 West River Cooperative Telephone Company – Tyrell Ellingson – 605-490-1601

Prior to excavation in or adjacent to BNSF (Burlington Northern Santa Fe) Railway ROW and in conjunction with contacting the SD One-Call and ND One-Call, the Contractor will call the BNSF Utility Locate number 1-800-533-2891.

#### **LOCATE UTILITIES**

The work consists of the Contractor excavating material to locate and verify depth of an existing utility line (private or public) when directed by the Engineer to determine if changes are necessary to project scope or quantity of contract bid items. Payment will be at the contract unit "Locate Utilities" per each for all labor, equipment and materials needed. A quantity of  $\underline{2}$  estimated locations have been included in these plans.

#### **TABLE OF LOCATE AND VERIFY UTILTIES**

		Type		Quantity
Station to	Station		L/R	(Each)
81+90.47	81+97.51	Fiber	L/R	1
200+45.00	205+45.00	Railroad	L/R	1
			Total:	2

**SHRINKAGE FACTOR:** Embankment +10%

See Table of Excavation Quantities at the end of the notes for Additional Details of Unclassified Excavation.

#### TABLE OF UNCLASSIFIED EXCAVATION

Excavation	1216
Undercut	1104
Topsoil	222
Total Unclassified Excavation	2542



# PROCEDURES FOR DETERMINING UNCLASSIFIED EXCAVATION QUANTITY

When plan quantities are used for payment, the Unclassified Excavation quantity will be used for final payment and the plans quantity of Topsoil listed in the Table of Unclassified Excavation will be adjusted according to field measurements.

The following paragraphs are general earthwork information and information in regard to computing the Unclassified Excavation quantity when final cross sections are taken in the field:

The Topsoil quantity in the Table of Unclassified Excavation is an estimate. When finaling a project, the total quantity of field measured Topsoil will be used in place of the estimated Topsoil quantity. The quantity of Topsoil from the cuts will be paid for twice as Unclassified Excavation, as it will be in both the Excavation and Topsoil quantities. This will be full compensation for Excavation, which includes necessary undercutting to provide space for placement of topsoil.

The Excavation quantities from individual balances and the Table of Unclassified Excavation have been reduced by the volume of in place surfacing that will be removed and/or salvaged.

The volume of in place Asphalt Surfacing removed will NOT be paid for as Unclassified Excavation.

Undercutting will be paid separately as noted below.

#### UNDERCUTTING

In all cut sections, the earthen subgrade will be undercut 1 feet below the earthen subgrade surface. The undercut will extend from behind the curb to behind the opposite curb as needed for paving operations. Shallow embankment sections, fills less than 1 feet in height measured at the finished subgrade shoulders, will be undercut to ensure a minimum 1-foot height of earth embankment for the entire width of the roadbed. The undercut material or other suitable material, as directed by the Engineer, will then be replaced and recompacted to the density specified for the section being constructed.

Intersecting streets will be undercut to the same depth as the Mainline roadway out to the limits of asphalt concrete / PCC pavement and curb & gutter placement on the intersecting street. Quantities are included in the "Table of Undercutting".

The Table shown is for bidding purposes, payment will be made in accordance with the contract unit price per cubic yard for "Undercutting".

#### **TABLE OF UNDERCUTTING QUANTITIES URBAN**

Station to	Station	Description	Quantity (CuYd)
80+49.85	84+41.85	1 <sup>st</sup> Street	609.77
100+20.21	102+97.70	1 <sup>st</sup> Avenue E	288.00
43+35.48	44+57.57	2 <sup>nd</sup> Street	180.42
43+50 R		Int. Street	25.50
		Total:	1103.69

#### **TABLE OF PCC PAVEMENT REMOVAL**

			Quantity
Station to	Station	L/R	(SqYd)
80+00.00	84+51.80	L _	36.4
		Total	36.4

#### **6" CONCRETE VALLEY GUTTER (4' WIDE)**

Payment for "6" Concrete Valley Gutter" will be based on plans quantity. If additions or reductions to the area of PCC valley gutters are ordered by the Engineer, payment will be made in accordance with the contract unit price per square yard for "6" Concrete Valley Gutter".

#### **TABLE OF 6" CONCRETE VALLEY GUTTER (4' WIDE)**

				Quantity
Station	to	Station	L/R	(SqYd)
83+73.7		84+01.9	R	13
			Total:	13

#### PIPE CULVERT UNDERCUT

Pipe culvert undercut may be required for this project. The Engineer will determine which pipe will be undercut in accordance with Section 421 of the Specifications.

If pipe culvert undercut is required, the table below contains the rate for one-foot depth of pipe culvert undercut per foot of pipe length. When calculating pipe culvert undercut, the length of pipe ends should be included in the overall pipe length. The table below contains the rate for one-foot depth of pipe culvert undercut per foot of pipe length and should be used as an aid in determining the actual amount of undercut to be performed during construction. The table is derived from the drawing below and conforms to the Specifications. When calculating pipe culvert undercut, the length of pipe ends should be included in the overall pipe length.



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Pipe culvert undercut will be incidental to the corresponding pipe install bid
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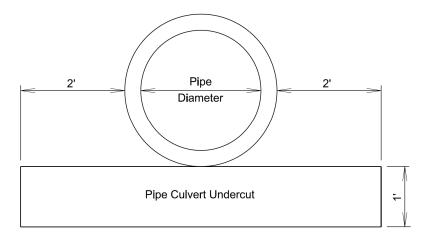
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	Pipe	Round Pipe	Arch Pipe
	Diameter	Undercut Rate	Undercut Rate
		for 1' Depth	for 1' Depth
_	(ln)	(CuYd/Ft)	(CuYd/Ft)
	24	0.2407	0.2577
	30	0.2623	0.2847
	36	0.2840	0.3110
	42	0.3056	0.3337
	48	0.3272	0.3596
	54	0.3488	0.3827
	60	0.3704	0.4105
	66	0.3920	
	72	0.4136	0.4630
	78	0.4352	
	84	0.4568	0.5123
	90	0.4784	

PIPE CULVERT UNDERCUT CONTINUED



#### **SALVAGED ITEMS**

The Contractor will coordinate with City personnel to determine the materials deemed salvageable. Care will be taken not to damage the structural properties of the items during dismantling and transporting. All broken concrete and materials not salvaged will be disposed of in accordance with the Specifications. All costs for salvaging and transporting the items will be incidental to the contract lump sum price for "Incidental Work, Grading". In preparation of bid, the Contractor will make a visual inspection of the project to verify the extent of the work and material involved.

#### SALVAGED ITEMS CONTINUED

Items to be considered for salvage include but are not limited to:

- Manhole Castings
- Drop Inlet Castings
- Sign Supports

#### **SALVAGE FENCE**

Fence removal from Sta. 82+30.9 to 83+55.4 is a combination of Wire Mesh and Wooden 4"x4" Posts. Fence removal from Station 205+34.2 to 206+38.7 is barbed 4-wire with metal posts. The Contractor will coordinate with the adjacent landowner to determine an appropriate location to stockpile the salvaged material.

#### **REMOVAL OF GRAVEL SURFACING**

See note for "GRAVEL SURFACING" in Section F.

#### TABLE OF ASPHALT CONCRETE PAVEMENT REMOVAL

Station to	Station	L/R	Quantity (SqYd)
43+35.44	44+57.64	L	417
43+35.44	44+57.64	R	430
80+00.00	84+51.80	L	964
80+00.00	84+51.80	R	1035
100+33.20	102+88.70	L	656
100+33.20	102+88.70	R _	683
		Total:	4185

#### **TABLE OF REMOVE SEWER PIPE**

					Quantity
	Station to	L/R	Station	L/R	(Ft)
_	83+84.5	L	83+32	L	85
	100+02.50	L	102+12.80	L	213
				Total:	298

#### TABLE OF CONCRETE CURB AND/OR GUTTER REMOVAL

Station to	Station	L/R	Quantity (Ft)
43+27.50	43+60.40	R	50
44+06.57	44+10.63	R	45
80+34.04	80+63.04	L	32
80+92.85	81+95.08	L	103
82+70.03	83+52.97	L	83
82+30.92	83+46.05	R	116
84+14.33	84+23.43	R	17
100+23.93	101+53.20	L	162
101+73.95	102+57.10	L	84
100+23.00	101+46.80	R	136
101+70.50	102+42.70	R	72
102+66.20	102+85.20	R _	19
		Total	919

#### TABLE OF CONCRETE DRIVEWAY PAVEMENT REMOVAL

Station to	Station	L/R	Quantity (SqYd)
81+95.15	82+69.98	L	71
80+53.30	81+13.40	R	70
101+46.80	102+65.50	R	21
		Total:	162

#### TABLE OF SIDEWALK REMOVAL

			Quantity
Station to	Station	L/R	(SqYd)
43+27.49	43+60.09	R	18
100+28.10	102+88.72	L _	201
		Total:	219

#### TABLE OF FENCE SALVAGE & REMOVAL

			Quantity
Station to	Station	L/R	(Ft)
82+30.9	83+55.4	R	131
*205+34.2	206+38.7	R	71
		Total:	202

<sup>\*</sup>Noted in plan sheets to reinstall

#### TABLE OF MANHOLE WITH FRAME AND LID REMOVAL

		Quantity
Station	L/R	(Each)
83+74.90	L	1
100+09.90	L	1
	–	
	Total:	2

#### TABLE OF DROP INLET REMOVAL

All costs for removal of the frame and grate assembly will be incidental to the contract unit price per each for "Remove Drop Inlet".

Station	L/R	Quantity (Each)
43+61.35	R	1
44+08.59	R	1
83+49.10	L	1
83+51.00	R	1
84+32.10	R	1
100+29.70	R	1
100+23.30	L _	1
	Total:	7



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Ota Cara	L /D	06.45	L /D	Quantity
Station to	L/R	Station	L/R	(Ft)
43+72.60	L	43+35.07	L	39
43+73.43	R	43+62.06	R	11
43+74.11	L	43+74.24	R	54
43+75.22	R	44+07.79	R	33
43+75.54	L	44+63.24	L	89
81+21.20	L	81+27.20	L	6
82+43.70	L	82+49.70	L	6
83+31.90	L	83+75.70	L	45
83+49.10	L	83+49.40	L	11
83+49.40	L	83+51.10	R	35
83+76.40	L	83+74.90	L	10
83+78.90	L	83+74.90	L	8
100+09.90	L	103+33.80	L	324
100+25.20	L	100+23.30	L	14
100+25.20	L	100+29.70	R	32
102+73.66	R	102+85.02	R	33

TABLE OF REMOVE STORM SEWER PIPE

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#### CELLULAR GROUT FOR BORE AND JACK

The Contractor will submit a proposed grouting procedure to the Engineer at least two weeks prior to beginning this work.

Total:

Bulkheads will be constructed at each end of the 48" RCP and 66" Steel casing railroad crossing. Each bulkhead will be constructed to withstand the pressure of the grouting operation. The bulkhead will extend from the end of the existing pipe inward a minimum depth of 18 inches and will be free from leaks.

Pressure grouting will be done to ensure all the voids are filled including all breaks or holes in and around the existing pipe.

The grout will be a cellular grout (grout with pre-generated foam) with a minimum 28-day compressive strength of 100 pounds per square inch. If water is not present within the pipe a low-density grout with a minimum of 30 pounds per cubic foot wet density may be used. When it is not possible to dewater the existing pipe, a high-density grout with a minimum of 70 pounds per cubic foot will be used which may include approved sand. The foaming agent used will meet the requirements of ASTM C869 when tested in accordance with ASTM C796.

Both of the cellular grout mix designs will be submitted to the Engineer for approval prior to use. The mix design submittal will include the base cement slurry mix per cubic yard, expansion factor from the foaming agent, and the cellular grout wet density (pounds per cubic foot).

The Contractor will install a bypass valve adjacent to the location where the pressure grouting hose is attached for obtaining samples to be checked for wet density. The wet density of the cellular grout will be checked by the Contractor to verify the proper minimum wet density before the cellular grout filling operations begin and at a minimum once every two hours during production. The Engineer will document the results of the density checks.

#### CELLULAR GROUT FOR BORE AND JACK CONTINUED

Cellular grout will be wasted until the cellular grout meets the minimum wet density required; however, if 0.5 cubic yards or more of base cement slurry is wasted trying to meet density requirements, then that quantity will not be included for payment.

If grout holes are utilized, cylindrical wooden plugs or other approved plugs will be inserted to plug holes until the grout has set. After the plugs are removed the holes will be filled with concrete.

The quantity of cellular grout was estimated based on volume of the existing pipe and voids outside the existing pipe.

The quantity of base cement slurry ordered will be approved by the Engineer. The quantity of base cement slurry needed will be calculated to the nearest tenth of a cubic yard using the approved mix design, expansion factor of the foaming agent, and estimated amount of cellular grout. The quantity for payment to the nearest tenth of a cubic yard of "Cellular Grout" is a calculated quantity based on the amount of base cement slurry used on the project to the nearest tenth of a cubic yard, expansion factor of the foaming agent, and approved mix design.

All costs for furnishing and installing the cellular grout including bulkhead construction, inlet bevel construction, and incidentals necessary to satisfactorily complete the work will be included in the contract unit price for related items.

#### TABLE OF CELLULAR GROUT BORE AND JACK

Station	to	Station	L/R	Quantity (CuYd)	
201+02.10*		202+82.10	L/R	40.9	_
201+02.10*	*	202+82.10	L/R	89.7	
To	otal:			130.6	

The quantity at each location includes an additional 5% to account for void and waste volume outside the existing pipe.

\*First Grouting Quantity is to fill 2" Void outside of 66" Steel Casing

#### **CONCRETE PIPE CONNECTIONS**

Pipe connections to existing pipes, manholes, junction boxes, and drop inlets will be done by breaking a hole into the existing structure and inserting the pipe. A concrete collar will then be poured around the pipe in the area of the connection.

When it is not possible to use a normal pipe joint (male-female ends), connections to existing pipe will be made by placing a 2' wide by 6" thick M6 concrete collar around the outside of the connection. The concrete collar will be reinforced with 6x6 W2.9 x W2.9 wire mesh.

All costs for constructing the concrete collars including materials and labor will be incidental to the contract unit price per foot for the corresponding pipe contract item.

#### PIPE COVER

The earthen subgrade cover for some pipe installations is less than one foot. The Contractor will take the necessary precautions to ensure the structural properties of the pipes are not damaged after installation and prior to the placement of final surfacing. Any additional costs for preventing damage to these pipes will be incidental to the contract unit price per foot for the corresponding pipe installation contract item.

#### STORM SEWER

Reinforced concrete pipe may be bell and spigot. The pipe sections will be adjoined such that the ends are fully entered and the inner surfaces are reasonably flush and even.

Lift holes in the reinforced concrete pipe will be plugged with grout.

Watertight joints are required for reinforced concrete pipe, drop inlets, manholes, and junction boxes where storm sewers run parallel to and within 10 feet horizontally from existing or proposed water mains.

Watertight joints are required where reinforced concrete pipes, drop inlets, manholes, or junction boxes cross water mains and are separated a distance of 18 inches or less, above or below, the water main.

If watertight joints are required, then the watertight joints will extend for a distance of 10 feet beyond the water main. This measurement will be from the sealed concrete joint to the outer most surface of the water main.

Watertight joint seals will conform to the following requirements:

- 1. Reinforced Concrete Pipe (Circular): Gasketed pipe will conform to the requirements of ASTM C443 and the gasket will be in conformance with Section 990 of the Specifications. Non-gasketed concrete pipe will be sealed with a mastic joint seal conforming to the requirements of ASTM C990 and encased with a minimum 2-foot wide by 6-inch thick M6 concrete collar reinforced with 6x6 W2.9 x W2.9 wire mesh.
- 2. Reinforced Concrete Pipe (Arch): Gasketed pipe will conform to the requirements of ASTM C443 and the gasket will be in conformance with Section 990 of the Specifications. Non-gasketed concrete pipe joints will be sealed with a hydrophilic flexible water stop seal and wrapped with a 1-foot wide strip of fabric above the cradle. The fabric will conform to the requirements of Section 831 of the Specifications for Type A Drainage Fabric. The hydrophilic flexible water stop will be from the list below.
- 3. <u>Drop Inlets, Manholes, and Junction Boxes</u>: Joints will be sealed with one of the following methods:
  - A. A flexible strip seal placed in the joints conforming to the requirements of ASTM C990 and the perimeter encased with a minimum 2-foot wide by 6-inch thick M6 concrete collar reinforced with 6x6 W2.9 x W2.9 wire mesh.
  - B. A hydrophilic flexible water stop seal placed in the joints and a 1-foot wide strip of fabric wrapped around the perimeter of the pipe. The fabric will conform to the requirements of Section 831 of the Specifications for Type A Drainage Fabric. The hydrophilic flexible water stop will be from the list below.

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 A self-adhesive external joint seal wrap. The seal wrap will be from the list below.

#### Approved List of Self-adhesive Joint Wrap

Product Manufacturer

Mar Mac Seal Wrap Mar Mac Construction Products

McBee, SC 843-335-5909 www.marmac.com

ConWrap CS-212 Concrete Sealants, Inc.

Tipp City, OH 800-332-7325

http://www.conseal.com

#### Approved List of Hydrophilic Flexible Water Stop Seal:

<u>Product</u> <u>Manufacturer</u>

Waterstop RX Cetco

Hoffman Estates, IL 800-527-9948 www.cetco.com

Conseal CS-231 Concrete Sealants, Inc.

Tipp City, OH 800-332-7325

http://www.conseal.com

Gaskets and seals (mastic, waterstop, and seal wraps) will be installed in accordance with the Manufacturer's recommendations.

The cost for furnishing and installing all gaskets, mastic joint seal, water stop seal, seal wrap, concrete collars, and for plugging the lift holes will be incidental to the contract unit price per foot for the corresponding pipe contract item.



<sup>\*\*</sup> Second Grouting Quantity is to fill void between Steel casing and 48" RCP.

#### **BORE AND JACK RCP PIPE**

The Contractor will install 48" RCP from Sta. 201+01 to 202+86 by boring a casing pipe as specified in the "Bore and Jack Steel Pipe" note, then jacking the 48" RCP pipe through said steel pipe. The pipe will be installed by boring and jacking methods as specified herein unless an alternate plan is submitted in writing and approved by the Engineer and Railroad.

The Contractor will submit to the Engineer for approval, a design analysis for the RCP for jacking.

The pipe wall thickness will be determined by the pipe manufacturer.

The variation in laying length of two opposite sides of each pipe section will not be more than 3/8-inch.

The Contractor will submit a pipe type such that the pipe joint will contain a steel band around the exterior of the pipe and have a mastic seal.

Each section of pipe will have 2 threaded pipe inserts cast through the pipe wall located 2 feet from each end and 180 degrees apart. Refer to the RCP for Jacking Threaded Insert Detail.

It is preferred that lifting holes through the pipe wall are not provided. However, if lifting holes are provided in the pipe then the holes will be plugged using a wire mesh and grout or other manufactured plugs as approved by the Engineer.

Any single end crack in the joint will be cause for rejection of an individual section of pipe.

Acceptance testing of pipe at the plant will meet the requirements of the three-edge load bearing test to produce a 0.01-inch crack in at least three sections of pipe.

The manufacturer will supply 4 concrete cylinders for compressive strength testing.

The manufacturer will submit three initial copies of shop drawings to the Engineer for review a minimum of fifteen days prior to manufacturing the pipe. One reviewed copy will be sent back to the manufacturer who will then make any necessary changes and then send six final copies to the Engineer for approval and distribution. The shop drawings will contain the following information:

- 1. Detailed dimensions of pipe and joint, including diameters, wall thickness, laying length, and joint tongue and groove.
- 2. Concrete compressive strength, concrete mix design, admixtures, if any, and curing process.
- 3. Gasket or mastic type, specifications, and dimensions.
- 4. Reinforcing steel type, specification and grade, placement tolerance, diameter and area of circumferential, longitudinal and special reinforcement.

#### BORE AND JACK RCP PIPE CONTINUED

5. Location and dimensions of joint area where jacking force will be applied and maximum allowable jacking force in pounds per square inch on this area.

The jacking pit will be constructed of sufficient size to accommodate equipment and workmen and no closer than 30 feet to the centerline of the Railroad Tracks. The pit walls will be sloped or shored to comply with all applicable State and Federal regulations. The Contractor will be responsible for the design of the pit floor and jacking thrust restraint wall to carry the cyclic loads and thrust applied by the Contractor's operation. Water will not be allowed to accumulate in the jacking pit. All components of the jacking pit will be removed after installation of the pipe unless otherwise allowed by the Engineer.

The pipe will be pushed into position from a jacking pit with hydraulic jacks while simultaneously excavating at the forward end of the pipe. Each pipe section will be jacked from the jacking pit as the excavation at the boring head progresses so that the excavation is supported by the boring head or the pipe at all points.

Jacking thrust will be applied to the pipe by means of a yoke or frame designed to distribute the thrust uniformly around the pipe joint. The thrust will be applied to the pipe joint only in the location and only to the maximum force recommended by the pipe manufacturer. The pipe will be jacked into place without visible damage to the pipe or joint. A ¾-inch plywood cushion will be used between the yoke and each pipe joint.

Upon completion of the pipe jacking operation, the annular space between the pipe and the bored excavation and all voids around the outside face of the pipe will be filled with grout. The grout will be pumped through the threaded pipe inserts at sufficient pressure to force the grout into the annular space and voids as directed by the Engineer. The grout will be a Portland cement and sand mix or other material approved by the Engineer.

Payment for furnishing the jacking pipe will be incidental to the contract unit price per foot for "48" Class II RCP, Furnish".

All costs involved with boring and jacking the pipe including labor, equipment, and materials, including pressure grouting, and all costs related to constructing and backfilling the jacking pit will be incidental to the contract unit price per foot for "48" RCP. Install".

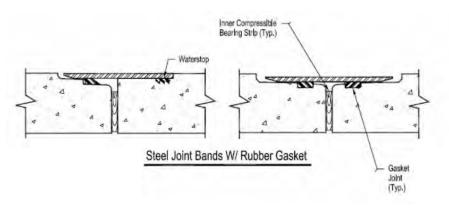


#### FOR BIDDING PURPOSES ONLY DAKOTA

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**BORE AND JACK RCP PIPE CONTINUED** 

# 1-1/2" Schedule 40 PVC Pipe V A V P



#### **BORE AND JACK STEEL PIPE**

The Contractor will install a 66" Smooth Steel Pipe at station 201+01 to 202+86 by boring and jacking the pipe through the existing railroad embankment. The pipe will be installed by boring and jacking methods as specified herein unless an alternate plan is submitted in writing and approved by the Railroad and Engineer.

Smooth steel pipe for boring and jacking will meet or exceed the requirements of ASTM A1097 Grade B, with a minimum yield strength of 35,000 psi. Hydrostatic testing will not be required for this application. The pipe will be required to have a minimum wall thickness of 0.875 inches.

The exterior of the steel pipe will be coated with a fusion bonded epoxy coating and an abrasion resistant overcoat or a two-component coal tar epoxy. The coal tar will meet the requirements of Sherwin-Williams Targuard, Tnemec Hi-Build Tneme-Tar, or an approved equal. Applications of the coatings will be in conformance with the manufacturer's recommendations.

#### BORE AND JACK STEEL PIPE CONTINUED

The Contractor will be responsible for providing cathodic protection if other measures do not meet BNSF's cathodic protection requirements, as noted in BNSF's "UTILITY ACCOMMODATION POLICY".

The pipe joints will be welded by a certified welder in accordance with Section 410.3 D of the Specifications. After the welding has been completed, the exposed area will be coated with 3M Scotchkote Liquid Epoxy 328 or a two-component coal tar epoxy meeting the requirements of Sherwin-Williams Targuard, Tnemec Hi-Build Tneme-Tar, or an approved equal.

The jacking pit will be constructed of sufficient size to accommodate equipment and workmen. The pit walls will be sloped or shored to comply with all applicable State and Federal regulations. The Contractor will be responsible for the design of the pit floor and jacking thrust restraint wall to carry the cyclic loads and thrust applied by the Contractor's operation. Water will not be allowed to accumulate in the jacking pit. All components of the jacking pit will be removed after installation of the pipe unless otherwise allowed by the Engineer.

The pipe will be pushed into position from a jacking pit with hydraulic jacks while simultaneously excavating at the forward end of the pipe. Each pipe section will be jacked from the jacking pit as the excavation at the boring head progresses so that the excavation is supported by the boring head or the pipe at all points.

Jacking thrust will be applied to the pipe by means of a yoke or frame designed to distribute the thrust uniformly around the pipe joint. The thrust will be applied to the pipe joint only in the location and only to the maximum force recommended by the pipe manufacturer. The pipe will be jacked into place without visible damage to the pipe or joint.

The boring head excavation will be circular with a maximum diameter equal to the outside diameter of the jacking pipe plus 1 inch. The Contractor will take whatever corrective action is necessary to prevent running, flowing, or squeezing ground conditions at the cutting face from causing large voids or significant loss of soil that may cause surface settlement.

The Contractor will control the alignment and grade of the pipe installation to meet the following tolerances:

- 1. Maximum horizontal deviation from plan shown alignment will be less than 0.15% of pipe length from the downstream end of pipe to the point of measurement.
- 2. Maximum vertical deviation from plan shown alignment will be less than 0.075% of pipe length from the downstream end of pipe to the point of measurement.

All material excavated by the boring head for the pipe installation will be disposed of by the Contractor. The excavated material from the boring pit will be used as backfill for the pit and compacted into place to the satisfaction of the Engineer.

Payment for furnishing the pipe will be included in the contract unit price per foot for "66" Steel Pipe, Furnish".

All costs involved with boring and jacking the pipe including labor, equipment, welding, materials, disposal of waste material, constructing and backfilling the jacking pit, and excavating and backfilling the embankment will be included in the contract unit price per foot for the "Bore and Jack 66" Pipe".

#### BORE AND JACK STEEL PIPE CONTINUED

Due to the nature of the soils at the bore and jack location, the boring head excavation may not be advanced beyond the leading end of the casing.

The Contractor is advised that additional precautionary measures may be necessary to prevent caving and/or inflow of materials during boring operations. Any settlement, displacement, or other damage to the railroad and/or embankment will be repaired by the Contractor, as directed by the Engineer, at no additional cost to the Owner.

#### RAILROAD COORDINATION

The Contractor is required to give BNSF at least 30 days advanced notice, in writing, before any work is started on the site. To avoid hazards, BNSF may have representatives present. If deemed necessary, for the purpose of inspection and any issuance of appropriate instructions for railway operations, the Contractor will coordinate all necessary work with the local roadmaster and any other persons deemed pertinent by BNSF.

Flagging, insurance, site access and equipment staging agreements not covered under the BNSF permit obtained by the City of Lemmon (21W-11198), will be provided as specified by BNSF requirements.

All costs to reimburse BNSF for flagging services provided will be the responsibility of the Contractor and will be incidental to the associated contract unit prices.

The Contractor will provide a copy of all documents required by BNSF to complete the project work, including but not limited to: Railroad protective insurance, certifications allowing entrance to BNSF property (safety, training, etc..), Contractor Right-of-Entry Agreements, if required, to the Engineer.

#### **RAILROAD TRACK MONITORING PLAN**

A survey crew will continually monitor the elevation and alignment of the railroad track above the proposed Bore and Jack during the entire procedure. If track movement or loss of ballast exceeds 1/4 inch during the jacking or boring operations, all work must stop, and the Railroad must be notified. The Railroad may take any actions necessary to ensure safe passage of trains. The Contractor must immediately submit a corrective plan of action to the Railroad and Engineer for review and approval. The Railroad must review and approve the proposed repair procedure before a notice to proceed. The finished repair must be inspected by the Railroad before the track can be placed back into service.

An estimated <u>144</u> hours for Contractor survey monitoring are included in the project quantities. All costs will be included in the contract bid item "Engineer Directed Surveying/Staking".

#### **SALVAGING OF FRAMES, GRATES, AND LIDS**

The Contractor will take precautions to salvage existing manhole or drop inlet frames, grates and lids.

Frames, grates, and lids that are salvaged by the Contractor will be stockpiled at the City of Lemmon City Shop at 211 1st Ave W, Lemmon, SD.

Payment for salvaging components were not included in the plans and will be incidental to the contract unit price "Incidental Work, Grading".

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Where drop inlets are constructed within areas of curb and gutter, the Contractor will construct weep holes of at least 3 inches in diameter in the drop inlet walls. The weep holes will be constructed at the same elevation as the adjacent top of the earthen subgrade and will be maintained clean and open at all times until the permanent surfacing is placed. The drop inlets will be covered throughout construction operations as necessary with an Engineer approved cover to provide safe travel for motorists and to prevent materials from entering the storm sewer system. After the permanent surfacing has been placed, the Contractor will seal the weep holes with grout and remove all debris from the drop inlet. All costs involved with the coverings, weep holes, and removing debris from the drop inlets will be incidental to the contract unit prices for the components of the drop inlets.

All costs to Furnishing and Install Pre-Cast Reinforced Concrete Drop Inlets will be paid per each to the contract unit price "2'x3' Type B Drop Inlet" as indicated in the plans.

If additions or reductions to the number of drop inlets are ordered by the Engineer, payment for the components required to construct the drop inlets will be made at the contract unit prices as indicated above.

#### **MANHOLES**

All costs to Furnishing and Install Pre-Cast Reinforced Concrete Round Manholes per the Detail Plate will be paid per each to the contract unit price for the associated size of Manhole as indicated in the plans.

The callouts included in these plans are for bidding purposes based on the pipe size, junction locations, and anticipated construction schedule requirements. The successful bidder (Contractor) is allowed to submit an alternative to the proposed structural type, and size, if they so choose with the approval of the Engineer. Any alternative structural type and size (such as cast in-place, square or rectangular manholes, etc..) must be approved by the Engineer prior to production or procurement of materials. The Contractor should not anticipate any changes will be allowed.

If cast in-place structures are allowed, the Contractor will be responsible for all structural design and submittals for review if sizes are not included in SDDOT Standard Plates.

If additions or reductions to the number of manholes are ordered by the Engineer, payment for the components required to construct the manholes will be made at the contract unit prices as indicated above.



#### **ADJUSTMENT OF MANHOLES**

The Contractor will adjust manholes to the extent necessary on this project. Adjusting the manholes may consist of removing the upper course of brick or removing the concrete walls, replacing the removed materials with brick or Class M6 concrete, placing adjusting rings if necessary, and resetting the manhole frame and lid. The elevation of the lid will be set at the same elevation of the adjacent new pavement or surrounding ground. All manhole frames, lids, and rings that are cracked or broken due to carelessness of the Contractor will be replaced with new manhole frames, lids, and rings that conform with the Specifications at the Contractor's expense. Manholes will be adjusted to the satisfaction of the Engineer. All costs involved in adjusting the manholes will be incidental to the contract unit price per each for "Adjust Manhole".

The Engineer may direct adjustment of manholes that were not included in these plans. Payment for adjusting manholes that were not included in the plans will be at the contract unit price per each for "Adjust Manhole" and "Reset Manhole Frame and Lid".

#### **ADJUSTMENT OF WATER VALVES AND CURB STOPS**

The Contractor will adjust water valves and water stops to the extent necessary on this project. Adjusting the water valves and curb stops may consist of removing the valve risers or curb stop riser and replacing or adjusting risers. The elevation of the valve lid or curb stop will be set at the same elevation of the adjacent new pavement or surrounding ground. All frames and Risers that are cracked or broken due to carelessness of the Contractor will be replaced with new lids and risers that conform with the Specifications at the Contractor's expense. Valves and water stops will be adjusted to the satisfaction of the Engineer. All costs involved in adjusting the Water Valves and Water Stops will be incidental to the contract unit price "Adjust Water Valve Box".

The Engineer may direct adjustment of Water Valves that were not included in these plans. Payment for adjusting valves and water stops that were not included in the plans will be incidental to the contract unit price "Adjust Water Valve Box".

#### **TABLE FOR ADJUSTMENT OF WATER VALVES**

Station	Adjustment
43+52.74 R	Vertical
43+96.41 R	Vertical
44+00.57 R	Vertical
99+88.53 L	Vertical
99+88.18 R	Vertical
100+40 R	Vertical
102+87 R	Vertical

#### TABLE OF TYPE B66 CONCRETE CURB AND GUTTER

			Quantity
Station to	Station	L/R	(Ft)
43+27.50	43+45.13	R	14.7
43+60.13	43+60.13	R	5.0
44+06.78	44+06.89	R	30.7
44+26.78	44+48.46	R	18.7
80+36.36	80+67.62	L/R	37.1
81+07.75	84+41.85	L	334.1
80+53.25	80+96.48	R	43.2
81+14.73	81+35.22	R	20.5
81+63.21	81+93.22	R	30
82+16.22	83+57.45	R	141.2
84+23.39	84+41.85	R	18.5
100+14.63	100+15.83	R	20
100+22.96	100+23.94	L	17.51
100+42.95	101+55.52	L	112.6
101+71.53	102+71.46	L	99.9
100+43.69	101+49.12	R	92.4
101+69.13	102+51.63	R	82.5
102+71.63	102+78.31	R	6.7
		Total:	1125.29

#### TABLE OF TYPE P6 CONCRETE CURB AND GUTTER

			Quantity
Station to	Station	L/R	(Ft)
80+67.62	81+17.75	L	50.13
80+96.48	81+14.73	R	18.25
81+35.22	81+63.21	R	27.99
81+93.22	82+16.22	R	23.00
102+71.46	102+87.46	L	16.00
101+49.12	101+69.13	R	20.01
101+55.52	101+71.53	L	16.01
102+51.63	102+71.63	R	20.00
		Total:	191.39

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Payment for "6" PCC Fillet Section" will be based on plans quantity. If additions or reductions to the area of PCC fillet sections are ordered by the Engineer, payment will be made in accordance with the contract unit price per square yard for "6" PCC Fillet Section".

#### **TABLE OF 6" PCC FILLET SECTION**

			Radius	Quantity
Station to	Station	L/R	(Ft)	(SqYd)
43+45.12	43+60.13	R	17.67	15.7
44+06.78	44+26.78	R	22.67	22.2
83+57.45	83+74.77	R	18.67	13.24
84+01.89	84+23.39	R	18.67	20.22
100+14.63	100+53.68	R	42.67	62.2
100+22.96	100+42.96	L _	22.67	22.3
		_	Total:	155.86



#### **WATER MAIN**

Contractors License: The Contractor will obtain a "South Dakota State Sewer and Water Plumbing Contractor's License" prior to commencing construction.

#### **WATER MAIN LOWERING**

An estimated <u>5</u> water main lowering(s) will be required to install the proposed storm sewer. All costs to lower the water main at necessary crossings included but not limited to, furnishing and installing piping, bends, and restraints will be included in the contract unit price "Water Main Lowering".

All costs to reconnect and reestablish service to the Water Main Lowering(s) will be included in the contract unit price "Connect to Existing Water Main".

#### WATERMAIN SHUTDOWNS AND TEMPORARY FACILITIES

All valve operation will be done by City of Lemmon staff.

Provide minimum 48-hour notice for any shutdowns of existing watermains with the City of Lemmon.

The Contractor is responsible for all communication and notification with adjacent properties. Plan in advance to have all required equipment, materials, and labor on hand at the time of undertaking the connections so that work can proceed continuously to complete connections in the minimum time. Coordinate operation of existing valves and other appurtenances with the City of Lemmon. If a water service interruption affects a customer, who for legitimate reasons cannot be without service for the time in question, either reschedule the work to a time the customer can be without service or arrange to supply temporary service for said customer.

If service will not be restored within  $\underline{\mathbf{8}}$  hours from the start of loss to service, a temporary water supply will be required.

Provide temporary sanitary facilities for businesses or customers that will be without service during the connections.

All costs will be incidental to the contract unit prices for related items.

#### WATER MAIN AND APPURTENANCES

All ductile iron pipe and fittings will be wrapped with polyethylene tube material to protect the pipe from any future corrosion. The poly material will be installed per manufacturers specifications and the ductile iron handbook from DIPRA and ANSI A21.5 (AWWA C105).

Provide Polyvinyl Chloride Pipe meeting the requirements of AWWA C900 or C905 or C909 or the latest revision thereof and furnished in Cast Iron Pipe equivalent outside diameters with elastomeric joints.

Furnish PVC pipes smaller than 16-inches with a pressure class of PC150 with a DR of 18, PVC pipes 16-inches and larger with a pressure class of PC235 with a DR of 18.

Utilize a pipe material supplier that is ISO 9001 or 9002 registered. Provide shop drawings for Engineer approval.

Provide restrained joints for vertical adjustments. Obtain approval for all restrained jointing systems, except for preapproved systems. Preapproved restraining systems for Polyvinyl Chloride pipe include Certa-Lok, Yellowmine, and EBAA Iron Inc. Furnish Ductile Iron fittings manufactured in accordance with AWWA/ANSI C153/A21.53 or AWWA/ANSI C110/A21.10. Furnish Ductile Iron fittings with a working pressure of 350 pounds per square inch conforming with AWWA/ANSI C153/A21.53 or AWWA/ANSI C110/A21.10.

Provide all Cast Iron and Ductile Iron fittings as cement mortar lined and with an exterior bituminous seal conforming with AWWA/ANSI C104/A21.4. Install all fittings utilizing mechanical restrains in addition to thrust blocking.

All water distribution materials will meet NSF / ANSI Standard 61 – Drinking Water System Components, Health Effects, NSF/ANSI 61, and NSF/ANSI 372. The Contractor or Supplier may submit appropriate documentation to the Engineer for any materials not listed in the Supplemental Standard Specifications. This documentation must be provided no later than 7 days prior to bid opening



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Furnish and install the insulation required to protect the watermain at all water main lowering(s) at storm sewer crossings with less than 36" of horizontal and/or vertical cover. Provide insulation board meeting the following requirements:

- Thermal conductivity of not more than 0.28 BTU per hour per square foot per degree Fahrenheit per inch of thickness as tested in accordance with ASTM C177 is required.
- Not to absorb moisture to an extent greater than 2.5 percent by volume as tested in accordance with ASTM D2842.
- Compression strength greater than 20 psi as tested in accordance with ASTM D-1621.
- Minimum density of 1.3 pounds per cubic feet as tested in accordance with ASTM D-1622.
- Specifically designed for protection of underground utilities. Install in accordance with the manufacturer's recommendations.

Furnish and install a minimum of 4-inches thick of insulation centered over the watermain. Offset the insulation board joints between each layer. Install pipe bedding material between pipe and insulation.

All costs to furnish and install insulation board and pipe bedding will be included in the contract unit prices "Water Main Lowering".

#### **WATER MAIN DISINFECTION**

The Contractor must submit a plan to complete the water main lowering. When minor water main work occurs (i.e. tie-in connections of new water main to existing water main, water main adjustments, installation of new valves on existing main or any other work deemed minor by the Engineer) the existing main may be returned to service once the line has been flushed by City Staff.

Water that is discharged during water main flushing must not reach a stream, river, or water way if the chlorine residual exceeds 0.05 mg/L.

#### **TABLE OF WATERMAIN LOWERINGS**

Station to	L/R	Station	L/R	Water Main Lowering (Each)	Reconnect to Existing Water Main (Each)
44+31.68	R	44+51.85	R	1	2
83+97.02*	L	83+99.05*	L	1	2
100+00.76	R	100+20.77	R	1	2
100+52.52	R	100+79.52	R	1	2
205+66.29	L	205+66.29	R	1	2
			Total:	5	10

\*Water Main Lowering will require Vertical and Horizontal Offset to avoid Proposed Manhole.

#### **SANITARY SEWER - GENERAL**

- 1. Contractors License. The Contractor will obtain a "South Dakota State Sewer and Water Plumbing Contractor's License" prior to commencing construction.
- 2. The Contractor will notify the Engineer and City Staff upon completion of the sanitary sewer work. Inspection of the sanitary sewer will be made by the City, or consultant if applicable, and the Contractor to identify and note any deficiencies. The expense of any necessary television inspection will be borne entirely by the Contractor. The expense of any additional television inspections beyond the initial inspection due to Contractor errors will be borne entirely by the Contractor. Televising will not occur unless the following conditions are met:

Appropriate vehicle access i.e. level, free of mud holes, and
capable of supporting televising vehicles is provided to all sanitary
sewer manholes access points.

- Manholes/structures are exposed and easily accessible. Buried manholes are not acceptable.
- Manhole frames and lids per the City's standard plates are properly installed.
- Interior manhole work is complete, i.e. manhole inverts and pipe penetration complete, welding complete.
- Manholes, pipes, etc. are clean and free of debris including lumber or wood forms, gravel/dirt, etc. More than a one-half gallon container of gravel or dirt per 400' and in each manhole is unacceptable.
- The Contractor will place water in pipes to assist in identifying sags

Corrective actions will be provided to the project manager, who will provide them to the Contractor. After corrections are made, the television request may be submitted again, in writing, and the process will begin again and be repeated until the infrastructure has been accepted. Paving may occur but is not recommended until the sewer infrastructure has been reviewed and accepted. A time extension will not be considered due to the Contractor waiting for the sewer to be televised. The Contractor is still responsible for correcting items not meeting the engineering design standards after the initial inspection through the warranty period. Cleaning and maintenance of the sanitary sewer and storm drainage systems will be the responsibility of the Contractor until final completion. Weather delays and unforeseen circumstances may delay televising.

3. The Contractor is responsible for all backup prevention plans, which will be submitted to the Engineer for record.



#### SANITARY SEWER SERVICE CROSSINGS

An estimated <u>1</u> service crossing is included in the project quantities. All costs to adjust the sanitary service line will be included in the contract unit price "Adjust Sewer Service".

See "Table for Sanitary Sewer Pipe" for estimate quantities". These quantities are estimates only and the Contractor will consider the need for more or less pipe, based on field conditions, and should not expect to request additional compensation due to changes in pipe length, unless approved by the engineer.

Due to the depth of the Sanitary sewer main, all service adjustments will be made to lower the service line below the proposed storm sewer pipe. A minimum 18" of clearance will be maintained between the service line and the proposed storm sewer. The Contractor may lower the crossing to maintain 36" of cover at their own cost, refer to the "Insulation Board" note in other sheets of this note section for all sewer service crossings.

All mainline sanitary sewer pipe and fittings must be SDR 26 in accordance with the Special Provisions. All costs associated with this pipe material classification will be incidental to the respective PVC sanitary sewer pipe bid items.

All 4" sanitary sewer service pipe and fittings must meet Schedule 40 wall thickness. Sewer Services 6" or larger must meet mainline pipe note above.

Reconnect existing sanitary sewer service lines to new sewer service lines using Fernco Strongback RC Series Repair Couplings, Shear Guard repair couplings by Indiana Seal, or approved equal.

All costs to reconnect to the existing sanitary sewer service lines will be included in the contract unit price "Reconnect Sewer Service"

All costs to connect the sewer service to the existing sanitary sewer main will be included in the contract unit price "Connect to existing Sewer Main".

Make connections to PVC pipe by coring a hole and install an approved saddle-type tapping tee.

Alternatively, the Contractor may install a new manufactured PVC tee by cutting out a section of pipe and installing a new section with a tee or wye. Make connections to the host pipe at a smooth vertical cut, butted closely to the new PVC using a shielded flexible gasketed repair coupling to hold it in place. Ensure the host pipe inverts between new and old pipe are smooth.

If the Contractor uses the alternative method, CCTV inspection video for manhole to manhole pipe inspection will be submitted to the engineer for review at no additional cost to the Owner.

The City of Lemmon may not be able to field locate existing water or sanitary sewer service lines to properties. The locations of all sanitary sewer service lines shown on the Construction Drawings are taken from existing plans or "2014 Sewer Televising Report". These documents may be requested from HDR Engineering, 3231 Greensboro Dr, Suite 200, Bismarck. ND 58503.

All sewer pipe relocation not noted above will be included in the contract unit price "4" PVC Sewer Pipe"

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TABLE OF PVC SANITARY SEWER PIPE

Station	L/R	Station	L/R	Circular Pipe					
to	L/IX	Station	L/IX	4"	6"	8"	10"		
**83+84.59	L	84+43.31	L	120					
*102+12.80	L	102+12.10	R	26					
		7	146	0	0	0			

#### Referenced Standards:

- 1. American Iron and Steel Institute (AISI).
- 2. ASTM International (ASTM):
- a. C443, Standard Specification for Joints for Concrete Pipe and Manholes, Using Rubber Gaskets.
- b. D1785, Standard Specification for Poly(Vinyl Chloride) (PVC) Plastic Pipe, Schedules 40, 80, and 120.
- c. D2466, Standard Specification for Poly(Vinyl Chloride) (PVC) Plastic Pipe Fittings, Schedule 40.
- d. D2467, Standard Specification for Poly(Vinyl Chloride) (PVC) Plastic Pipe Fittings, Schedule 80.
- 3. American Water Works Association (AWWA):
- a. C800, Standard for Underground Service Line Valves and Fittings.
- 4. International Plumbing Code (IPC).
- 5. Underwriters Laboratories, Inc. (UL).

#### **TABLE PVC SANITARY SEWER CONNECTIONS**

Station to	L/R	Station	L/R	Reconnect Sanitary Sewer Service (Each)	Connect to Existing Sanitary Sewer Main (Each)
83+84.59	L	84+69.27	L	2	
102+12.80	2.80 L 102+12.10		R	1	1
		Total:		3	1

<sup>\*</sup>Service Crossing bid item

<sup>\*\*</sup> PVC Sewer Pipe bid item

#### **ALTERNATE BID A/B:**

In evaluating Bids, the Owner will consider whether or not the Bids comply with the prescribed requirements, and such alternates, unit prices, and other data, as may be requested in the Bid Form or prior to the Notice of Award.

After determination of the Successful Bidder based on this comparative process and on the responsiveness, responsibility, and other factors set forth in these Instructions, the award may be made to said Successful Bidder on its base Bid and any combination of its additive or substitute Bids for which Owner determines funds will be available at the time of award.

#### **HDPE STORM SEWER**

High-density polyethylene (HDPE) pipe or HDPE Steel-Reinforced Polyethylene (SRPE) will meet the material and performance requirements of ASTM F2306 or the latest edition of AASHTO M294 Type S or AASHTO M252 Type S or ASTM F2562"Standard Specification for Steel Reinforced Thermoplastic Ribbed Pipe and Fittings for Non-Pressure Drainage and Sewerage" or AASHTO Designation M335 and MP-40. HDPE pipe will be joined with soil tight joints meeting the requirements of AASHTO M294 Paragraph 7.9.3 or ASTM F2306 Paragraph 6.6.3.3 or ASTM D3212 "Standard Specification for Joints for Drain and Sewer Plastic Pipes Using Flexible Elastomeric Seals. Fittings may be joined using soil tight couplers or at manufacturers requirements. Junctions with concrete structures will meet requirements of ASTM C923. Installation will be per manufacturer's recommendations and/or ASTM D2331, whichever is more stringent. The Contractor will provide the Engineer all pipe product information, installation recommendations and instructions, and additional literature, as requested, for review and approval prior to ordering pipe.

The classification of HDPE or HDPE Steel-Reinforced Polyethylene (SRPE) pipe supplied will meet E-80 load rating for this project.

The Contractor will include all necessary data to ensure the installed pipe will maintain line and grade within the pipe trench during high flow events for construction and post construction. An example would be an anchoring system.

If additional pipe bedding or pipe undercutting is required per manufacturers installation requirements, all additional costs will be included in the associated bid items.

#### Approved List of HDPE Pipe:

<u>Product</u>	<u>Manufacturer</u>
DuroMaxx (30" or Larger) A-2000 (15"-36")	Contech ES West Chester, OH 800-338-1122 https://www.conteches.com/
SRPE Pipe	Colonial Construction Materials Oilville, VA 23129 800-436-6287 https://colonial-materials.com/

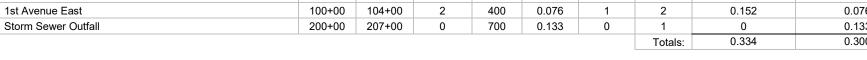




#### Revised 06/18/2025 STATE OF SOUTH DAKOTA SHEET P 6542(04) B12 B54

TABLE OF CONSTRUCTION STAKING
(See Special Provision for Contractor Staking)

						G	rade Staking		
Roadway and Description	Begin Station	End Station	Number of Lanes	Length (Ft)	Length (Mile)	Lane Factor	*Sets of Stakes	**Grade Staking Quantity (Mile)	Miscellaneous Staking Quantity (Mile)
1st Street East	80+00	84+82	2	482	0.091	1	2	0.182	0.091
1st Avenue East	100+00	104+00	2	400	0.076	1	2	0.152	0.076
Storm Sewer Outfall	200+00	207+00	0	700	0.133	0	1	0	0.133
	·						Totals:	0.334	0.300



<sup>\* 1 =</sup> Blue Top Stakes Only (Asphalt Concrete Pavement)2 = Blue Top and Paving Hub

An estimated 30 hours of miscellaneous surveying/staking has been included in the project quantities. All cost will be included in the bid item "Engineer Directed Surveying/Staking".

		TABLE OF EXCAV	ATION QUANT	TITIES	·
				(2)	(1)
			Topsoil Exc.	Excavation	Total Excavation
Station	to	Station	(CuYd)	(CuYd)	(CuYd)
2nd Street					
43+25		44+63	7.0	120	127
1st Street					
80+49.66		84+52.20	15.0	854	869
1st Avenue					
100+42.96		102+87.46	13.0	242	255
Storm Sewer R	R Crossing	and Outlet			
200+00.00		207+20.00	187.0		187
		Project Total	222	1,216	1,438

<sup>(1)</sup> The quantities for these items are in the Estimate of Quantities under their respective contract

<sup>(2)</sup> It is anticipated that there will be waste from grading, underground utility installation and excavation activities. It is the Contractor's responsibility to dispose of the excess material per SDDOT requirements.

					TABLE OF S	STORM SEWER	DROP IN	NLETS A	AND MAI	NHOLES						
					Pre-Cast	Drop Inlet	Pr	e-Cast Ma	nhole Rou	ınd	Frame an	and Grate / Lid			Adjustments	
					Si	ze		Si	ize				Reset		A -15	D 04
											Type B	Туре	Manhole Frame	Adjust	Adjustment Ring	Pre-Cast Collar
					2'x3'	5.5'x3'	72"	96"	120"	144"	Frame and Grate	A7	and Lid	Manhole	(Incidential)	(Incidential)
ID	Station	Offse	et	Туре	(Each)	(Each)	(Each)	(Each)	(Each)	(Each)	(Each)	(Each)	(Each)	(Each)	(Each)	(Each)
DI1	84+29.36	21.67	L	Precast 2x3 Type B	1						1					1
DI1	84+29.36	21.67	L	Precast 2x3 Type B	1						1					1
DI2	84+31.13	21.67	R	Precast 2x3 Type B	1						1					1
DI3	83+30.18	21.67	L	Precast 2x3 Type B	1						1					1
DI4	83+29.51	21.67	R	Precast 2x3 Type B	1						1					1
DI5	82+45.12	21.67	L	Precast 2x3 Type B	1						1					1
DI6	81+23.02	21.67	L	Precast 2x3 Type B	1						1					1
DI7	81+22.98	21.67	R	Precast 2x3 Type B	1						1					1
DI8	100+71.80	15.67	R	Precast 2x3 Type B	1						1					1
DI9	43+37.29	21.64	L	Precast 2x3 Type B	1						1					1
DI10	43+41.73	21.67	R	Precast 2x3 Type B	1						1					1
DI11A	44+42.28	11.92	L	Precast 5.5x3 Type B		1					1					1
DI11B	44+45.87	11.92	L	Precast 5.5x3 Type B		1					1					1
DI12	44+41.65	13.67	R	Precast 2x3 Type B	1						1					1
MH1	204+96.50	0.00	L/R	Precast 96" Round				1				1			6	
MH2	203+10.30	0.00	L/R	Precast 96" Round				1				1			6	
MH3	83+95.30	20.40	L	Precast 144" Round						1		1			6	
MH4	83+75.00	20.50	L	Precast 144" Round						1		1			6	
MH5	83+30.10	10.70	L	Precast 96" Round				1				1			6	
MH6	82+45.40	10.80	L	Precast 72" Round			1					1			6	
MH7	81+23.00	10.70	L	Precast 72" Round			1					1			6	
MH8	102+96.80	14.40	L	Precast 96" Round				1				1			6	
MH9	100+71.80	15.80	L	Precast 96" Round				1			2					
MH10	43+74.11	10.03	L	Precast 120" Round					1			1			1	
MH11	43+74.17	35.23	R	Precast 72" Round			1					1			1	
MH12	43+38.18	9.71	L	Precast 72" Round			1					1			1	
EXSSMH1	83+89.35	0.83	R	Existing Sanitary Sewer Manhole									1	1		
EXSSMH2	99+99.95	0.27	R	Existing Sanitary Sewer Manhole									1	1		
				Total:	12	2	4	5	1	2	16	11	2	2	51	14

<sup>\*\*</sup> Grade Staking Quantity = (Length) x (Lane Factor) x (Sets of Stakes)

	TABLI	E OF STO	RM SEWE	R PIPE - Al	t A			
				Circ	cular			
			Reinforce	ed Concrete			Steel Encas	sement Pipe
	15"	18"	36"	48"	60"	Flared End	66"	
	Cl 2	Cl 2	Cl 2	Cl 2	Cl 2	60"		
Station Offset (L/R)	(Ft)	(Ft)	(Ft)	(Ft)	(Ft)	(EA)	(Ft)	
2nd Street								
43+74.11 - 10.03' L to 43+37.29 - 21.64' L			30					
43+38.18 - 9.71' L to 43+37.29 - 21.64' L	10							
43+38.18 - 9.71' L to 43+41.73 - 21.67' R	28							
43+71.11 - 10.03' L to 44+45.87 - 11.92' L				64				
44+42.28 - 11.92' L to 44+41.65 - 13.67' R	22							
43+74.11 - 10.03' L to 43+74.17 - 35.23' R				40				
43+74.17 - 35.23' R to 43+74.19 - 44.81' R				8				
1st Street								
81+23.09 13.6' L to 81+23.07 20.7' L	8							
81+23.01 7.8' L to 81+22.99 20.7' R	30							
82+45.36 13.7' L to 82+45.19 20.7' L	8							
83+29.99 6.8' L to 83+29.52 20.7' R	28							
83+30.13 14.6' L to 83+30.25 20.7' L	8							
83+33.17 11.4' L to 83+69.66 19.3' L				38				
83+80.50 20.5' L to 83+89.84 20.3' L				10				
84+01.24 20.7' L to 84+26.07 21.62' L		26						
84+31+19 20.7' L to 84+31.18 20.7' R	42							
1st Avenue								
103+38.50 7.8' L to 102+99.27 13.8' L					40			
102+94.28 14.4' L to 100+74.43 15.8' L					220			
100+71.78 11.9' L to 100+71.78 14.7' R	28							
100+70.28 15.7' R to 100+56.98 15.7' R	14							
100+69.15 15.5' L to 100+14.19 9.2' L					56			
100+05.84 8.7' L to 99+97.83 8.7' L				8				
100+10.18 13.4' L to 100+10.13 21.1' L			8					
100+10.23-4.0' L to 100+10.25-4.0' R			8					
Railroad Crossing/North Lemmon								
200+26.89 17.8' L to 200+18.89 17.7' L					8			
206+84.35 0' L/R						1		
206+81.32 0' L/R to 204+99.18 0' L/R					176			
204+93.88 0' L/R to 203+12.98 0' L/R					182			
203+10.3 0' L/R to 200+35.24 0' L/R				280				
202+72.53 0' L/R to 200+92.51 0' L/R							180	
Project Total:	226	26	46	448	682	1	180	



## FOR BIDDING PURPOSES ONL

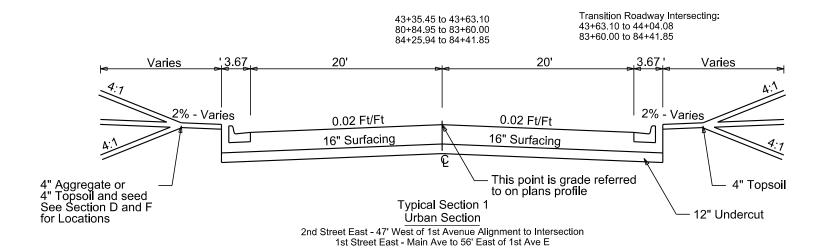
Т	ABLE OF A	ALTERNA	TIVE STOF	M SEWER	PIPE - Alt	В		
				(	Circular			
		High Dei	nsity Polyethy	lene Pipe		Corrugated Metal	Steel Enca	sement Pipe
	15"	18"	36"	48"	60"	Flared End	66"	
						60"		
Station Offset (L/R)	(Ft)	(Ft)		(Ft)	(Ft)	(EA)	(Ft)	
2nd Street								
43+74.11 - 10.03' L to 43+37.29 - 21.64' L			30					
43+38.18 - 9.71' L to 43+37.29 - 21.64' L	10							
43+38.18 - 9.71' L to 43+41.73 - 21.67' R	28							
43+71.11 - 10.03' L to 44+45.87 - 11.92' L				64				
44+42.28 - 11.92' L to 44+41.65 - 13.67' R	22							
43+74.11 - 10.03' L to 43+74.17 - 35.23' R				40				
43+74.17 - 35.23' R to 43+74.19 - 44.81' R				8				
1st Street								
81+23.09 13.6' L to 81+23.07 20.7' L	8							
81+23.01 7.8' L to 81+22.99 20.7' R	30							
82+45.36 13.7' L to 82+45.19 20.7' L	8							
83+29.99 6.8' L to 83+29.52 20.7' R	28							
83+30.13 14.6' L to 83+30.25 20.7' L	8							
83+33.17 11.4' L to 83+69.66 19.3' L				38				
83+80.50 20.5' L to 83+89.84 20.3' L					10			
84+01.24 20.7' L to 84+26.07 21.62' L		26						
84+31+19 20.7' L to 84+31.18 20.7' R	42							
1st Avenue								
103+38.50 7.8' L to 102+99.27 13.8' L					40			
102+94.28 14.4' L to 100+74.43 15.8' L					220			
100+71.78 11.9' L to 100+71.78 14.7' R	28							
100+70.28 15.7' R to 100+56.98 15.7' R	14							
100+69.15 15.5' L to 100+14.19 9.2' L					58			
100+05.84 8.7' L to 99+97.83 8.7' L				8				
100+10.18 13.4' L to 100+10.13 21.1' L			8					
100+10.23-4.0' L to 100+10.25-4.0' R			8					
Railroad Crossing/North Lemmon								
200+26.89 17.8' L to 200+18.89 17.7' L					8			
206+84.35 0' L/R						1		
206+81.32 0' L/R to 204+99.18 0' L/R					176			
204+93.88 0' L/R to 203+12.98 0' L/R					182			
203+10.3 0' L/R to 200+35.24 0' L/R					280			
202+72.53 0' L/R to 200+92.51 0' L/R							180	
Project Total:	226	26	46	158	974	1	180	

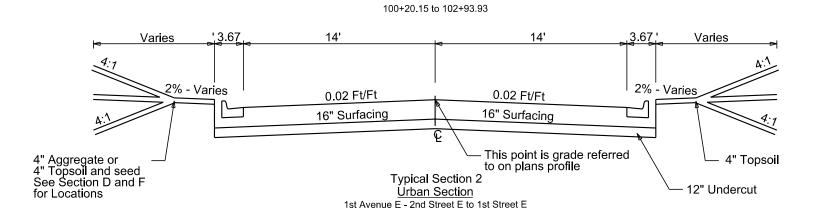
PROJECT P 6542(04) SHEET TOTAL SHEETS

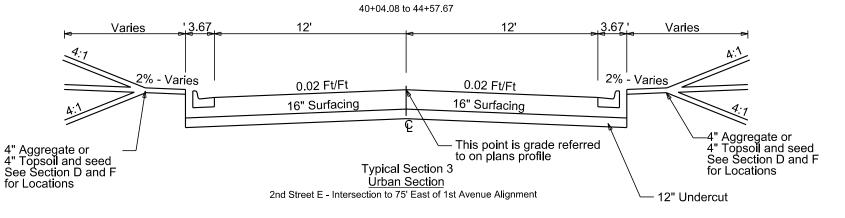
B14 B54

Plotting Date: 04-03-2025











# HORIZONTAL ALIGNMENT DATA STATE OF SOUTH PROPERTY DATA

STATE OF PROJECT SHEET TOTAL SHEETS

OUTH DAKOTA P 6542(04) B15 B54

#### 2<sup>nd</sup> Street East Mainline

#### 1<sup>st</sup> Street East Mainline

Type	Station			Northing	Easting
POB	80+00.00			776856.180	1419184.288
		TL= 24.82	S 87°27'12 <b>"</b> E		
PC	80+24.82			776855.077	1419209.083
ΡI	80+42.98	R = 300.00	Delta = 06°56'06" R	776854.270	1419227.243
PT	80+61.13			776851.275	1419245.173
		TL= 421.32	S 80°31'05 <b>"</b> E		
POE	84+82.45			776781.870	1419660.733

#### 1<sup>st</sup> Avenue East Mainline

Type	Station			Northing	Easting
POB	99+00.00			776376.138	1419553.383
		TL= 100.00	N 01°36'17" E		
PI	100+00.00			776476.099	1419556.183
		TL= 400.00	N 01°29'48" E		
POE	104+00.00			776875.962	1419566.630

#### **Outfall Pipe Alignment**

Type	Station			Northing	Easting
POB	200+00.00			776797.990	1419564.220
		TL= 310.34	N 37°14'60" E		
PT	203+10.34			777045.020	1419752.060
		TL= 186.19	N 82°08'56" E		
PT	204+96.53			777070.450	1419936.510
		TL= 223.96	N 53°01'48" E		
POE	207+20.49			777205.140	1420115.440



Type	Station			Northing	Easting
POB	40+00.00			776487.580	1419173.516
		TL= 840.62	S 88°18'21" E		
PI	48+40.62			776462.729	1420013.771
		TL= 733.13	S 88°12'52" E		
PI	55+73.75			776439.884	1420746.546
		TL= 1121.52	S 88°24'12" E		
POE	57+50.00			776434.974	1420945.474

# CONTROL DATA

FOR BIDDING PURPOSES ONL Y DAKOTA P 6542(04) B16 B54

HORIZONTAL AND VERTICAL CONTROL POINTS							
POINT	STATION	OFFSET	DESCRIPTION	NORTHING	EASTING	ELEVATION	
CP 150	NOT ON I	PROJECT	Iron Pin	776500.544	1419890.420	2565.17	
CP 160	NOT ON F	PROJECT	Iron Pin	774620.999	1420354.887	2568.68	
CP 151	NOT ON F	PROJECT	Iron Pin	776708.732	1419896.538	2566.70	
CP 152	NOT ON I	PROJECT	Iron Pin	776378.145	1421928.900	2564.49	
CP 153	NOT ON I	PROJECT	Iron Pin	776563.312	1421934.418	2563.00	
CP 154	NOT ON I	PROJECT	Iron Pin	774390.306	1420284.139	2566.69	
CP 155	NOT ON F	PROJECT	Iron Pin	774395.461	1420142.241	2565.89	
CP 156	NOT ON F	PROJECT	Iron Pin	774294.051	1420141.220	2566.17	
CP 157	NOT ON F	PROJECT	Iron Pin	774289.754	1420281.055	2567.06	
CP 158	NOT ON F	PROJECT	Iron Pin	775384.397	1420380.306	2562.15	
CP 159	NOT ON F	PROJECT	Iron Pin	774616.766	1420504.833	2575.02	
CP 800	NOT ON F	PROJECT	Iron Pin	776748.493	1419658.996	2565.30	
CP 801	NOT ON F	PROJECT	Iron Pin	776811.440	1419647.133	2566.01	
CP 802	NOT ON F	PROJECT	Iron Pin	777069.558	1420454.232	2567.90	
CP 803	NOT ON F	PROJECT	Iron Pin	776940.813	1419480.077	2569.53	
CP 804	NOT ON F	PROJECT	Iron Pin	776925.965	1419477.550	2569.10	
CP 805	NOT ON I	PROJECT	Rebar	776910.028	1419663.705	2569.38	
CP 806	NOT ON I	PROJECT	Iron Pin	776966.117	1419237.863	2567.60	
CP 807	NOT ON I	PROJECT	Rebar	777151.073	1419907.917	2568.56	
CP 808	NOT ON F	PROJECT	Stone	777030.682	1423079.043	2557.58	



FOR BIDDING PURPOSES ONLY

STATE OF SOUTH DAKOTA

PROJECT SHEET
P 6542(04) B17

TOTAL SHEETS

B54

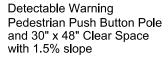
Plotting Date: 04-03-2025

Anchor	<del></del>	Mailbox	۵
Antenna	Δ,	Manhole Electric	0
Approach		Manhole Gas	0
Assumed Corner	<b>②</b>	Manhole Miscellaneous	0
Azimuth Marker	<b>A</b>	Manhole Sanitary Sewer	0
BBQ Grill/ Fireplace	▲	Manhole Storm Sewer	0
Bearing Tree	<b>(1)</b>	Manhole Telephone	0
Bench Mark	<b>≜</b>	Manhole Water	0
Box Culvert		Merry-Go-Round	*
Bridge		Microwave Radio Tower	夲
Brush/Hedge	62523	Miscellaneous Line	
Buildings		Miscellaneous Property Corner	<u> </u>
Bulk Tank	$\bigcirc$	Miscellaneous Post	0
Cattle Guard	<b>==</b>	Overhang Or Encroachment	
Cemetery	+	Overhead Utility Line	— OH —
Centerline		Parking Meter	Ŷ
Cistern	©	Pedestrian Push Button Pole	0
Clothes Line		Pipe With End Section	<b></b>
Concrete Symbol	439	Pipe With Headwall	<b>——</b>
Control Point	₾	Pipe Without End Section	
Creek Edge		Playground Slide	$\leftarrow$
Curb/Gutter	======	Playground Swing	<b>&gt;</b> →+K
Curb		Power And Light Pole	<b>-</b>
Dam Grade/Dike/Levee	<del></del>	Power And Telephone Pole	Ď
Deck Edge		Power Meter	* * * * *
Ditch Block	2500	Power Pole	Ø
Doorway Threshold	<del></del>	Power Pole And Transformer	- <b>\</b> -
Drainage Profile		Power Tower Structure	☆
Drop Inlet		Propane Tank	
Edge Of Asphalt		Property Pipe	$\odot$
Edge Of Concrete		Property Pipe With Cap	<b>(9)</b>
Edge Of Gravel		Property Stone	PS
Edge Of Other		Public Telephone	<u> </u>
Edge Of Shoulder	<del></del>	Railroad Crossing Signal	- <b>∳</b> 4
Electric Transformer/Power Junction	n Box 🕑	Railroad Milepost Marker	·
Fence Barbwire		Railroad Profile	
Fence Chainlink		Railroad ROW Marker	0
Fence Electric		Railroad Signs	þ
Fence Miscellaneous	<i></i>	Railroad Switch	
Fence Rock		Railroad Track	
Fence Snow		Railroad Trestle	
Fence Wood		Rebar	Æ
Fence Woven		Rebar With Cap	A
Fire Hydrant	රිා	Reference Mark	<u> </u>
Flag Pole	P	Retaining Wall	
Flower Bed	7777	Riprap	σσσσσα
Gas Valve Or Meter	•	River Edge	
Gas Pump Island	© 3	Rock And Wire Baskets	<u> </u>
Grain Bin	<u> </u>	Rockpiles	— - ' ₽\$&
Guardrail		Satellite Dish	4
Gutter	2222	Septic Tank	Ψ
Guy Pole	9	Shrub Tree	T \$
Haystack	<b>₩</b>	Sidewalk	
Highway ROW Marker	<b>*</b>	Sign Face	
Interstate Close Gate	Ţ- <b>ÿ</b>		•
Interstate Close Gate Iron Pin	⊙ .1	Sign Post	<u>                               </u>
		Slough Or Marsh	<u></u>
Irrigation Ditch		Spring	·
Lake Edge	<del></del> -	Stream Gauge	Ø
LAWILSHINKEE	19	STEAR MARKAT	h

Street Marker

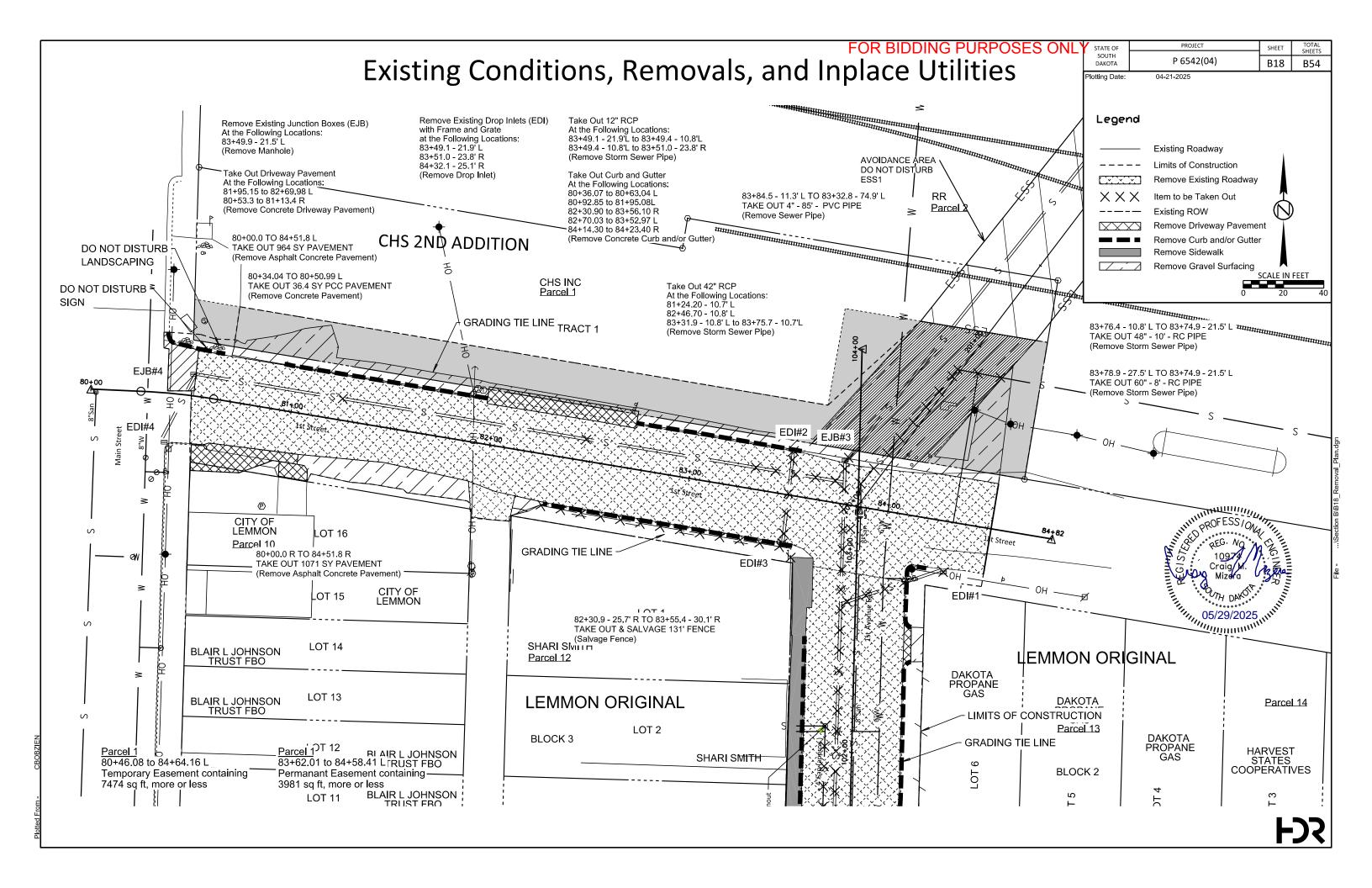
Lawn Sprinkler

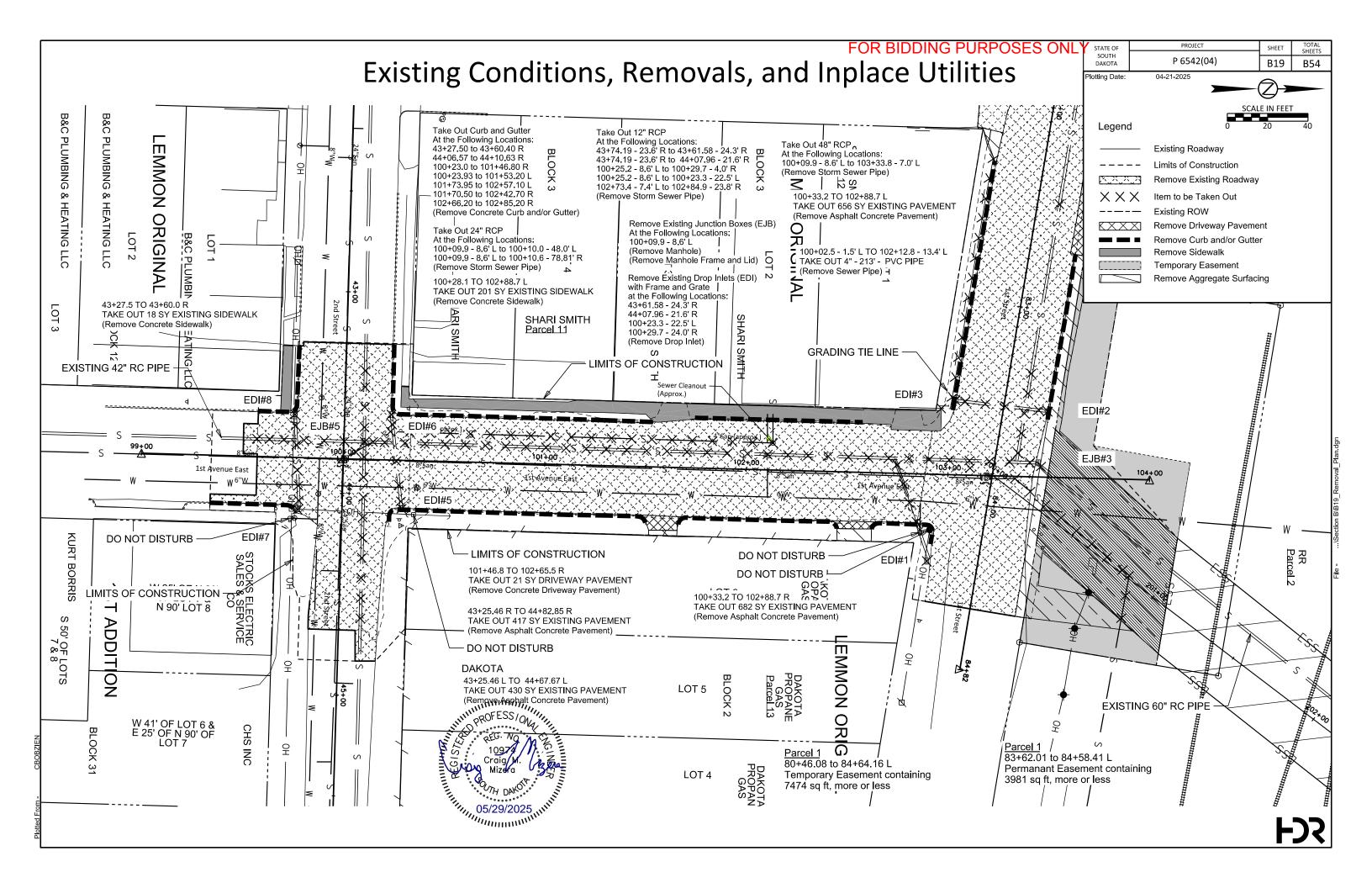
State and National Line County Line Section Line Quarter Line Sixteenth Line Property Line Construction Line Existing R.O.W. Line New R.O.W. Line Cut and Fill Limits Existing Control of Access Line New Control of Access Line Proposed R.O.W. Line (After Property Disposal) Drainage Arrow Extremely Sensitive Area – ESS – Remove Concrete Pavement Remove Concrete Driveway Pavement Remove Asphalt Concrete Pavement Remove Concrete Sidewalk Remove Concrete Median Pavement Remove Concrete Curb and/or Gutter

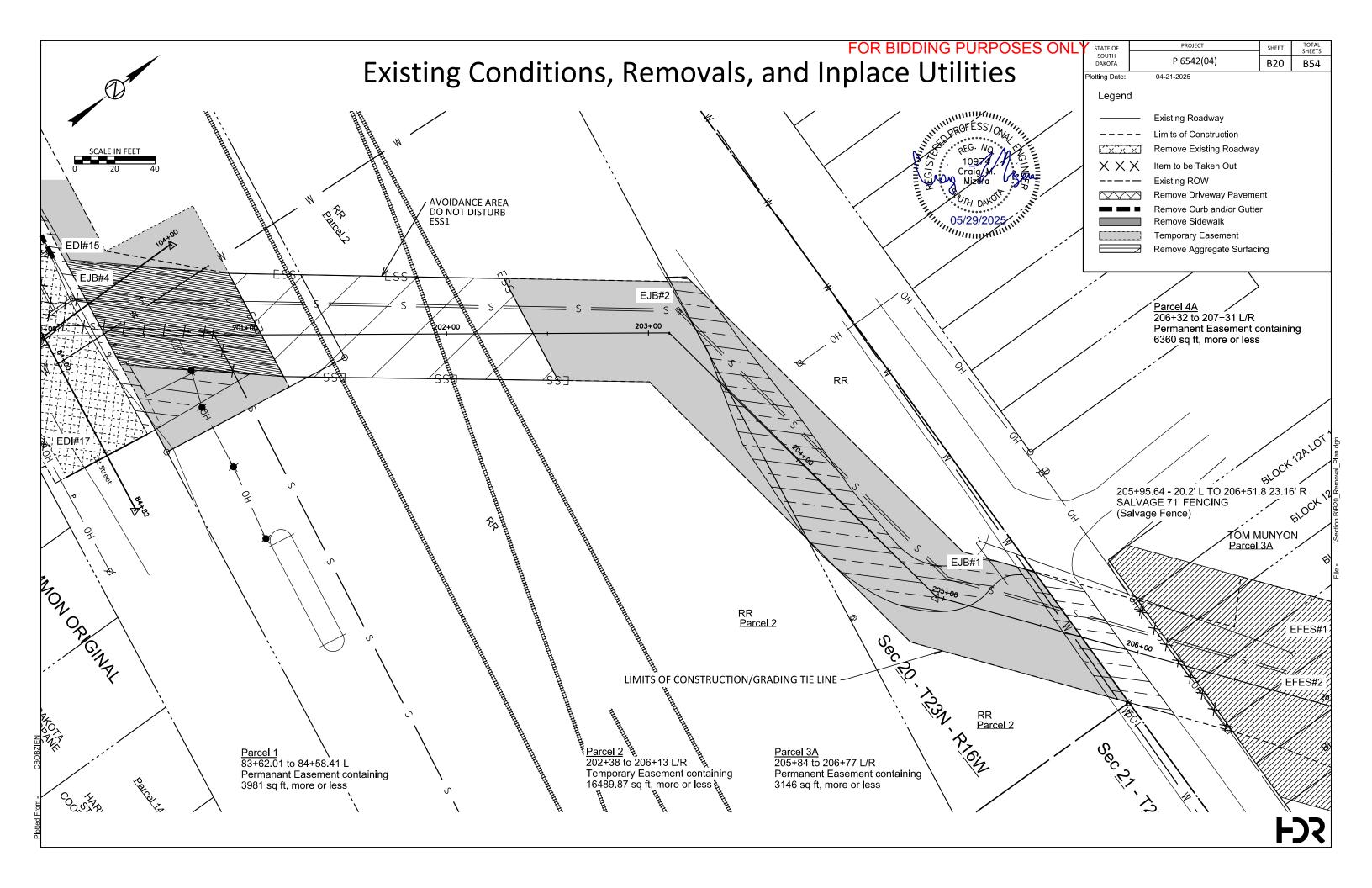


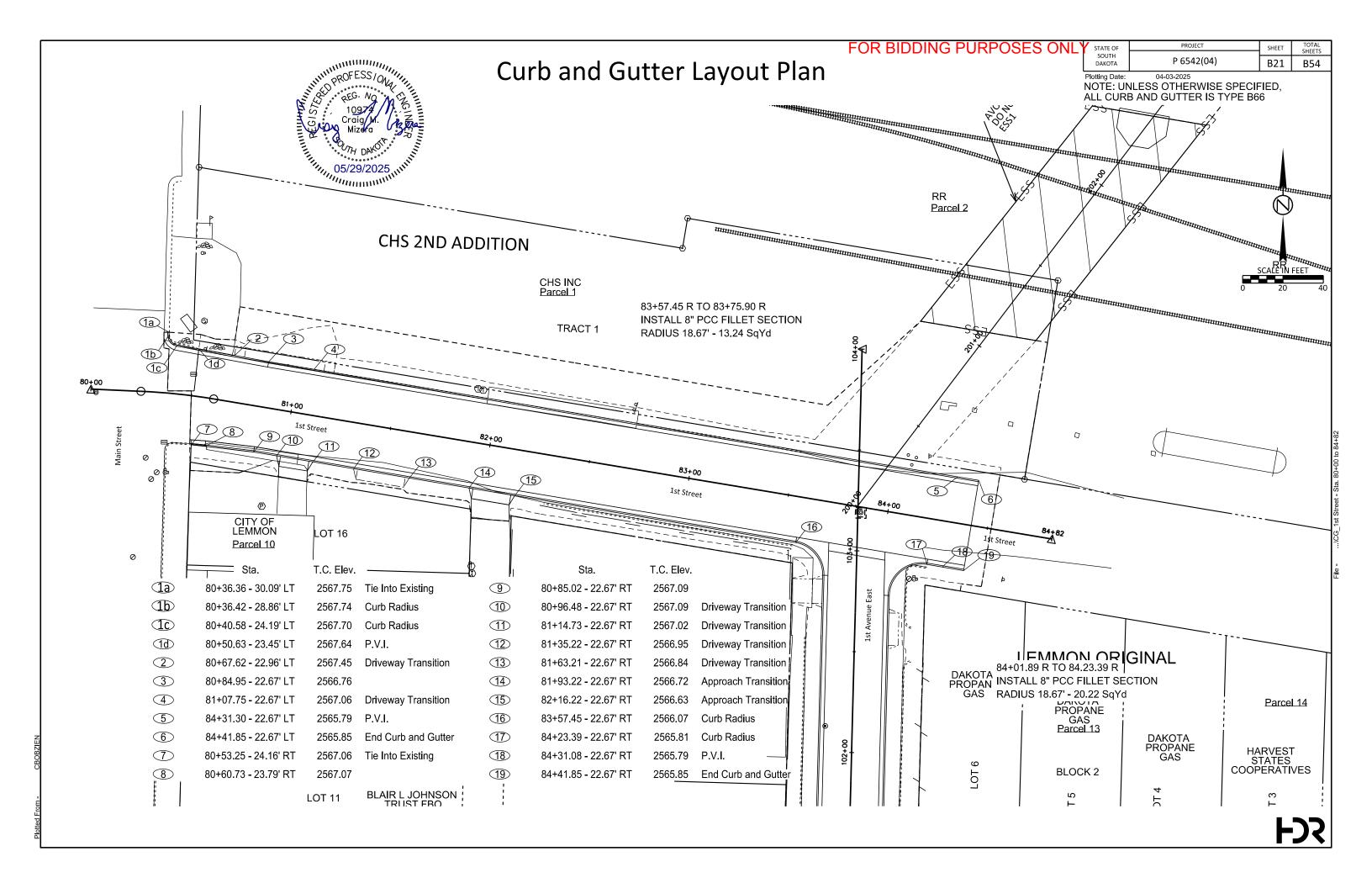


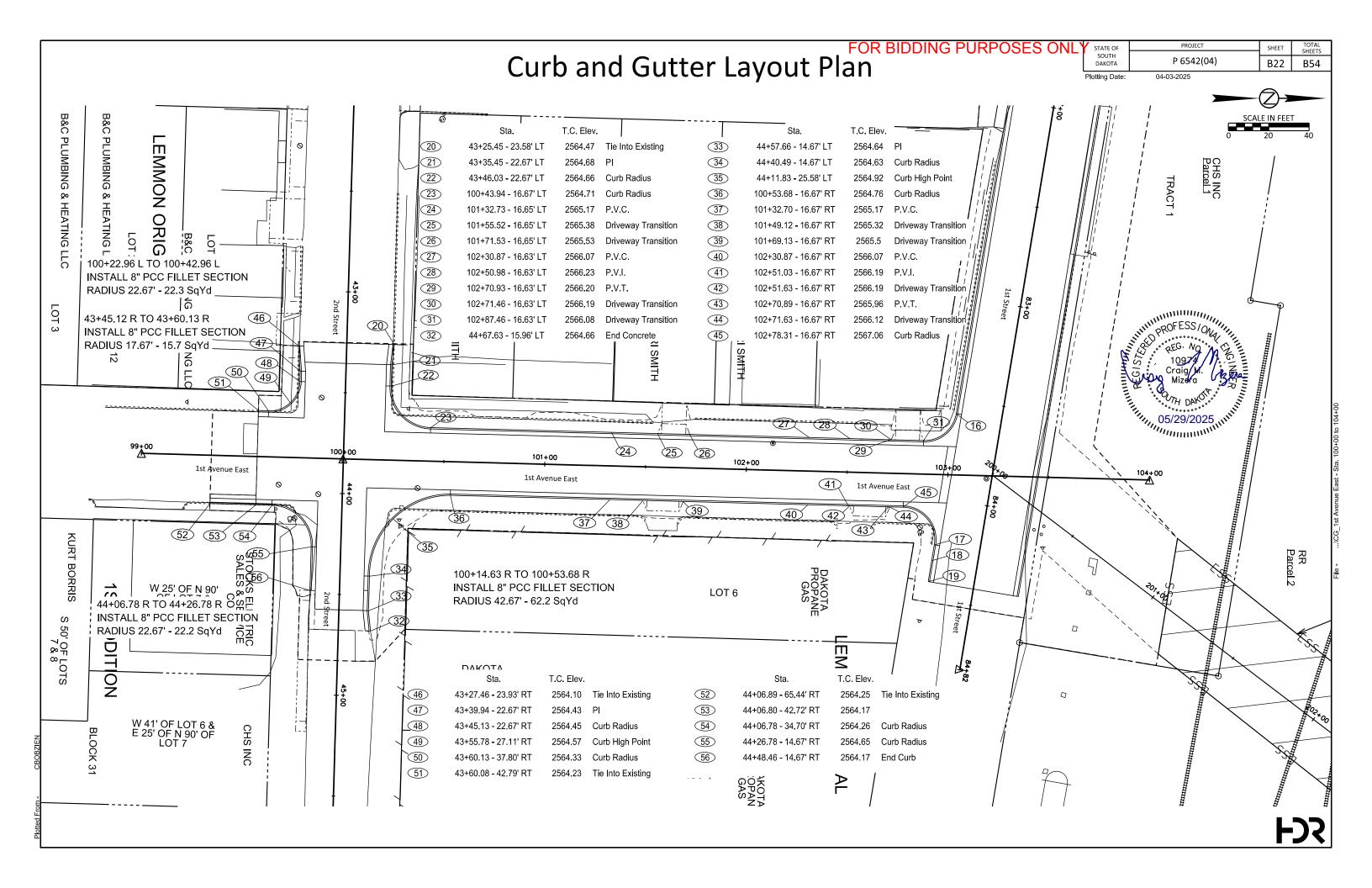


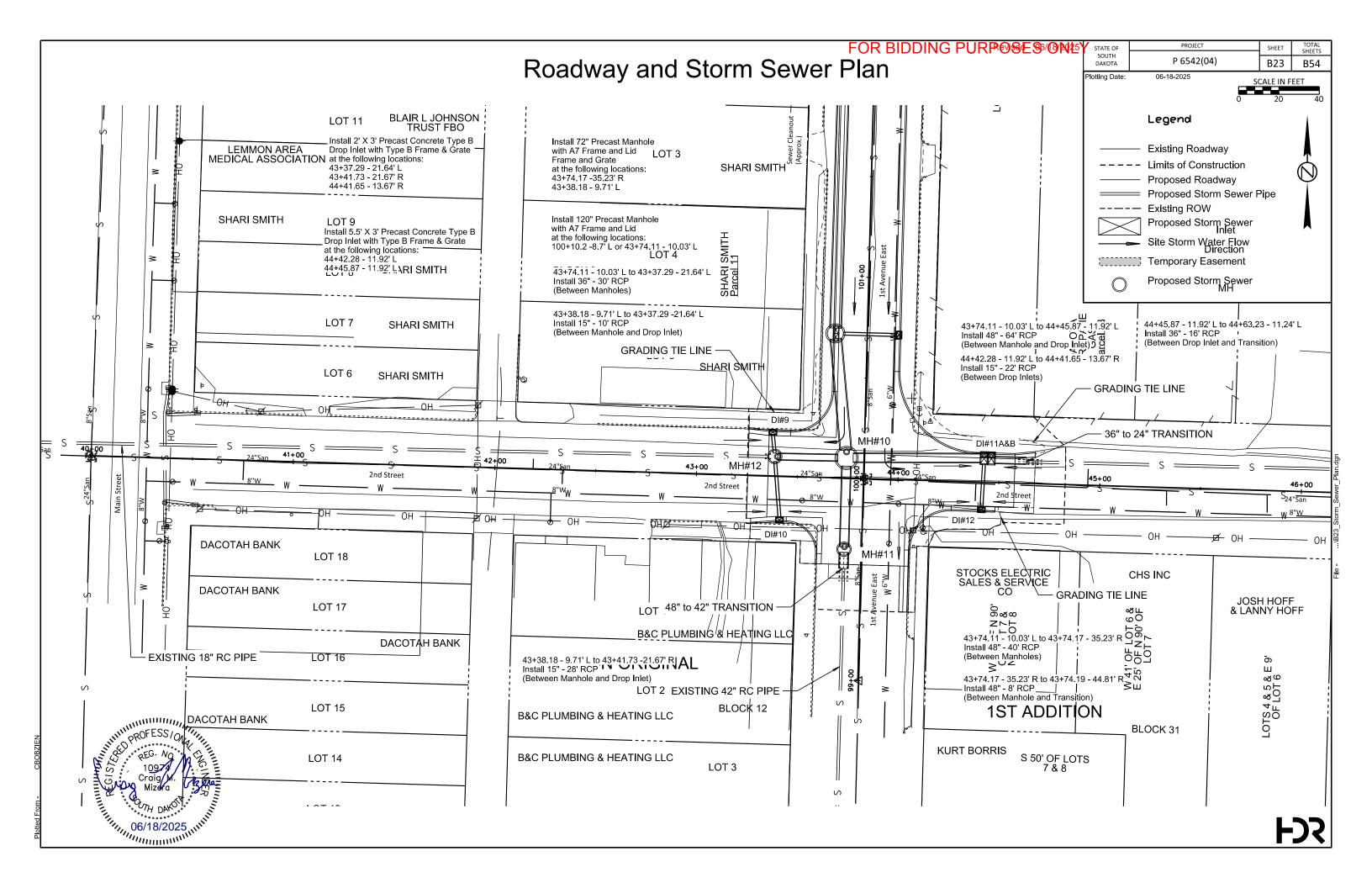


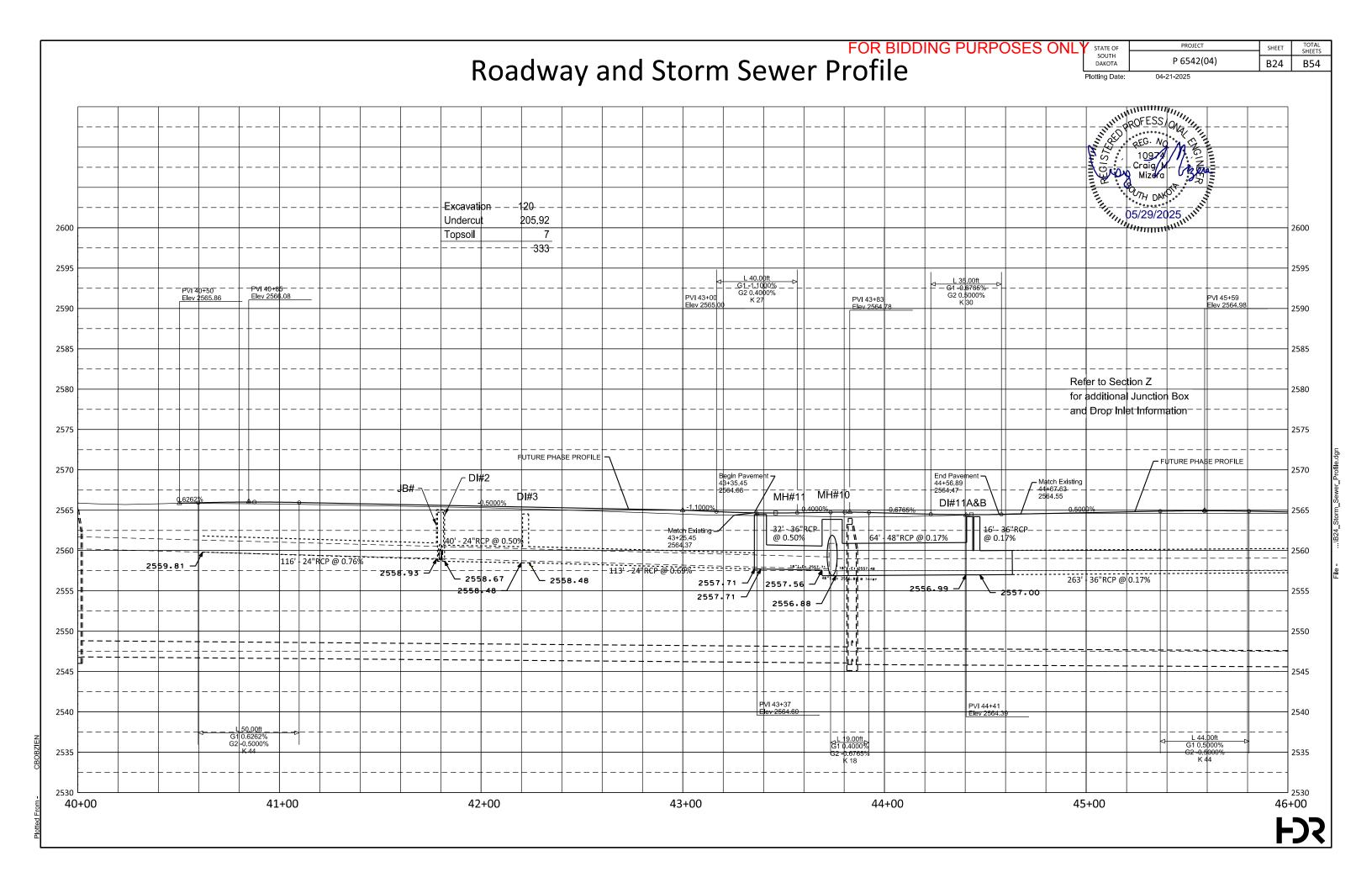


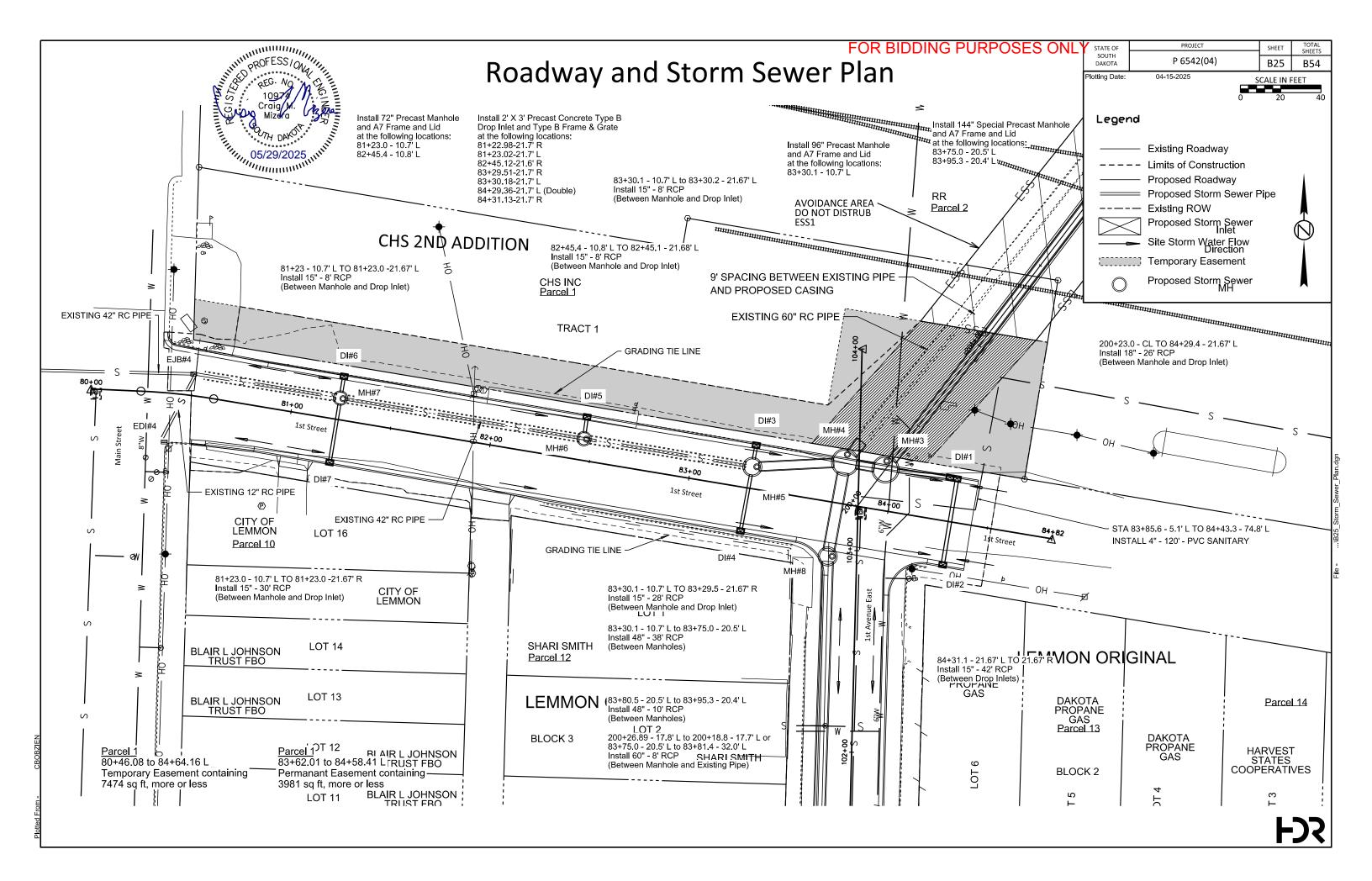


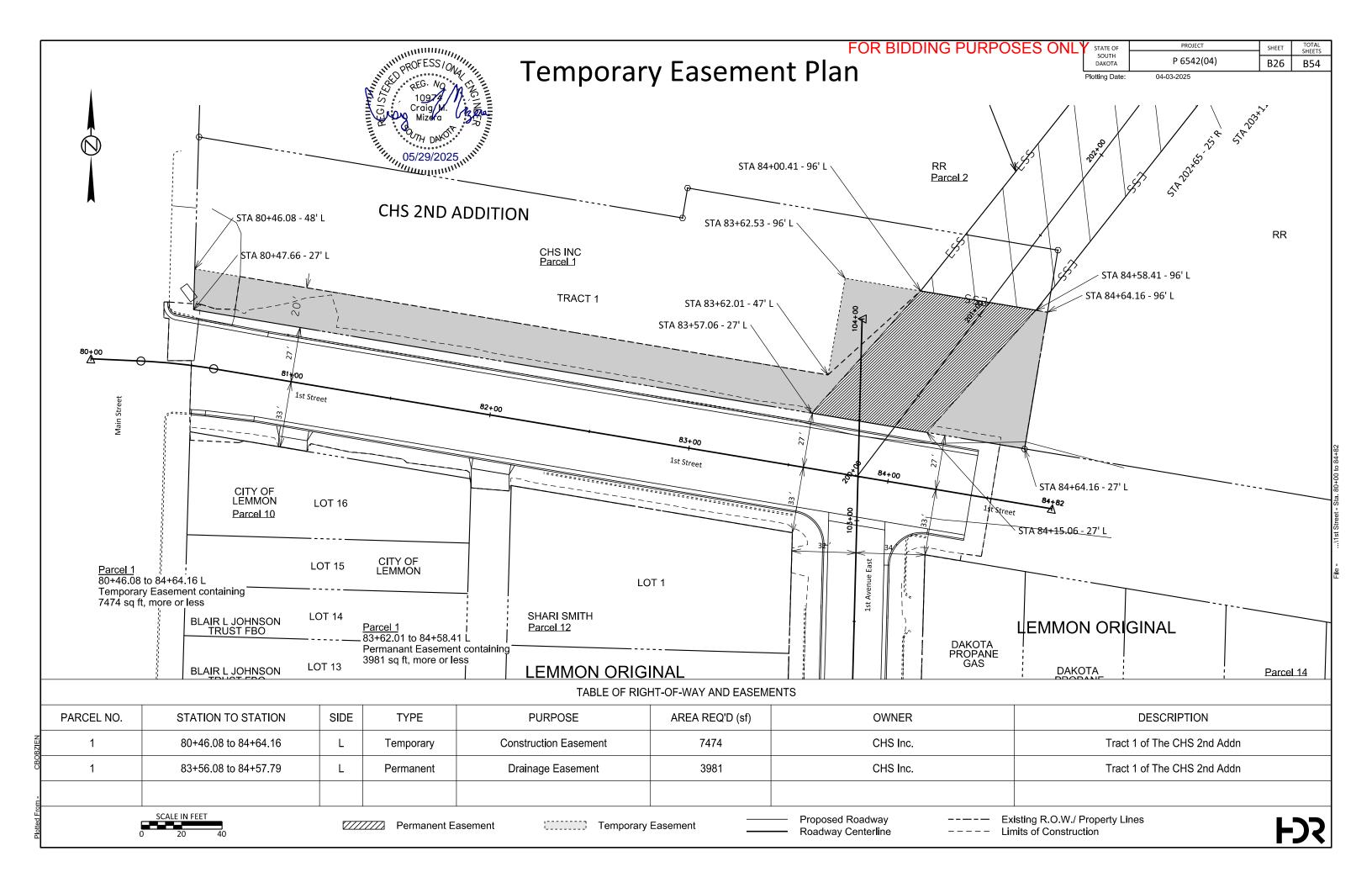


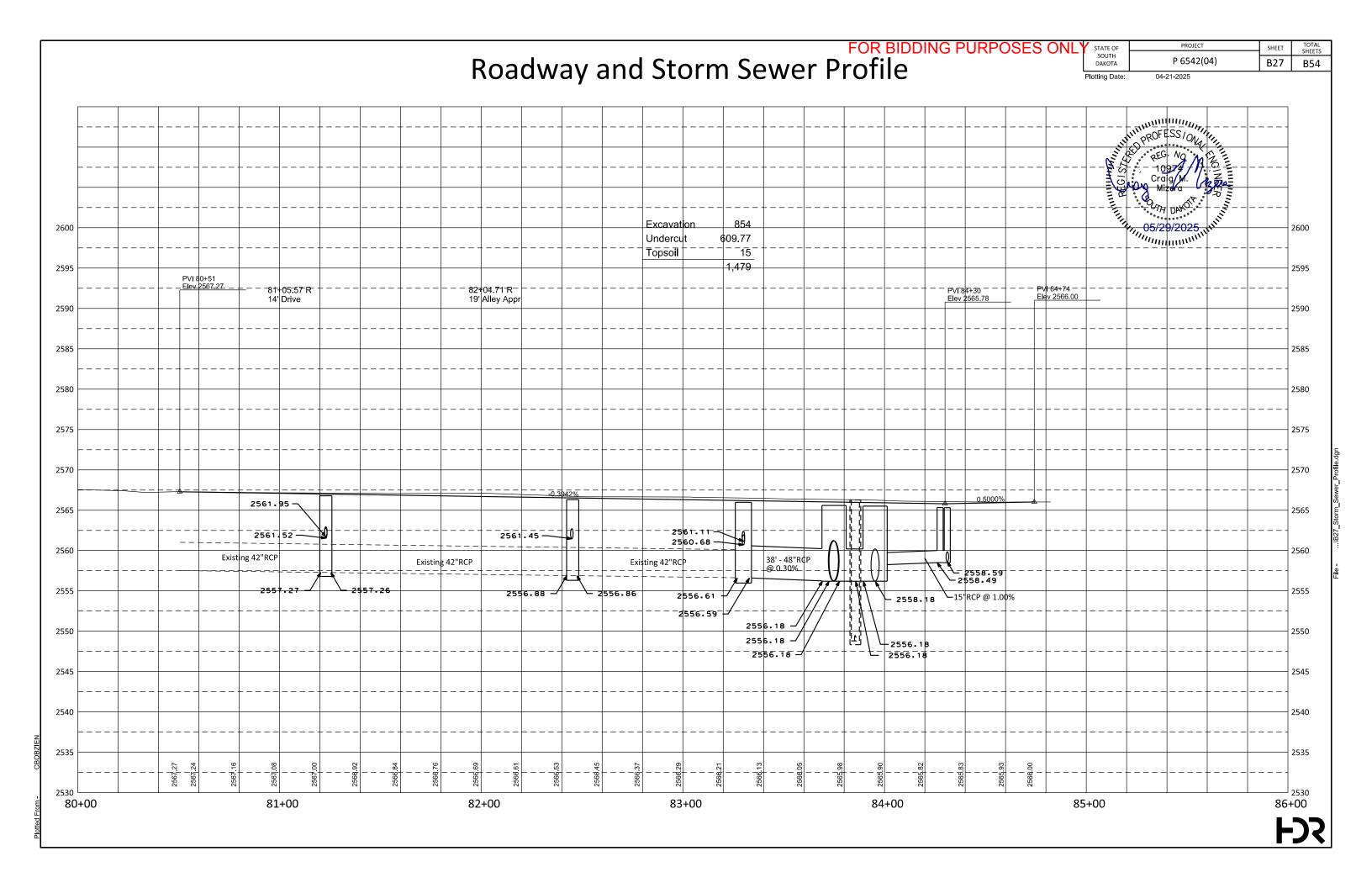


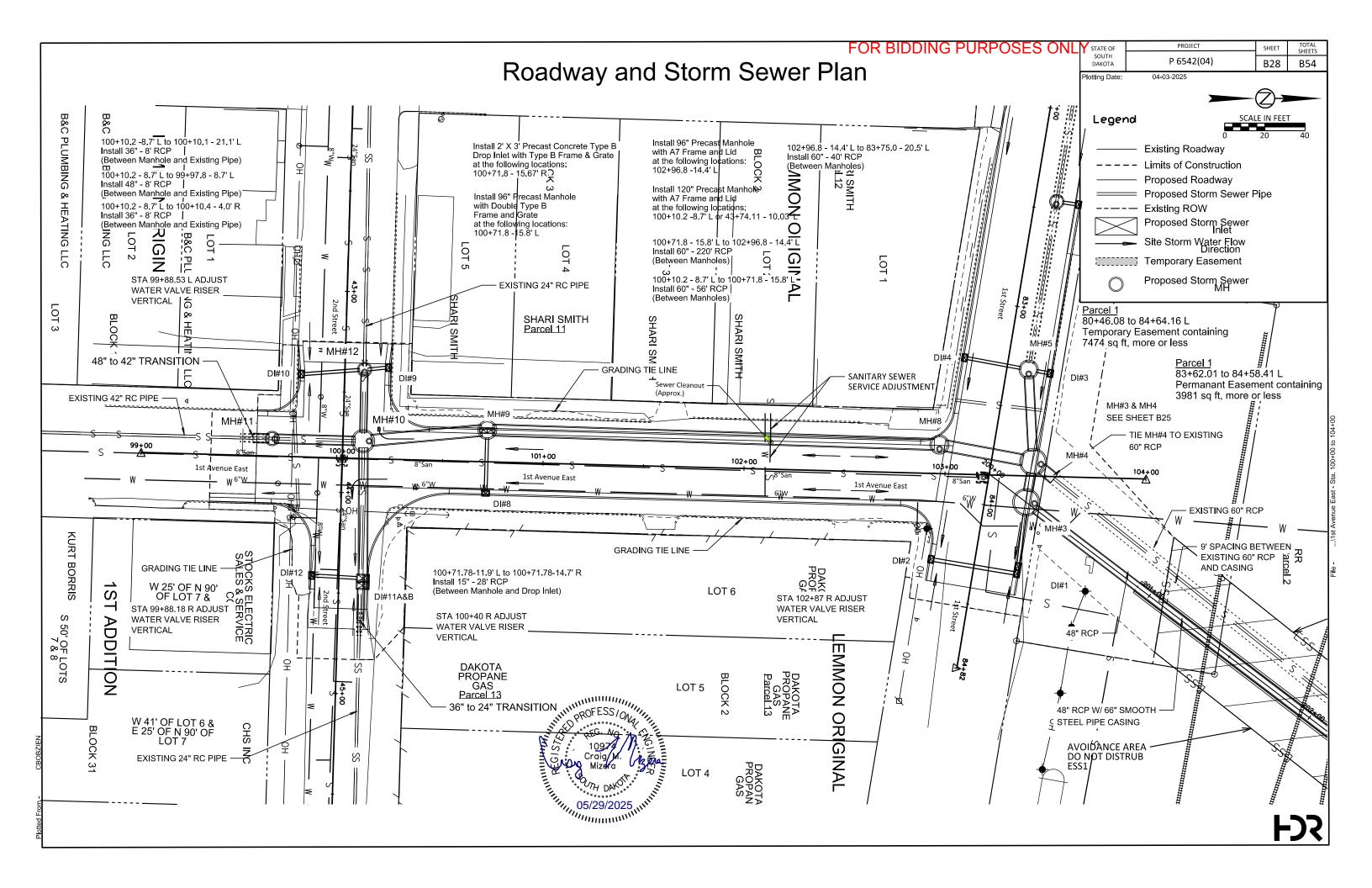


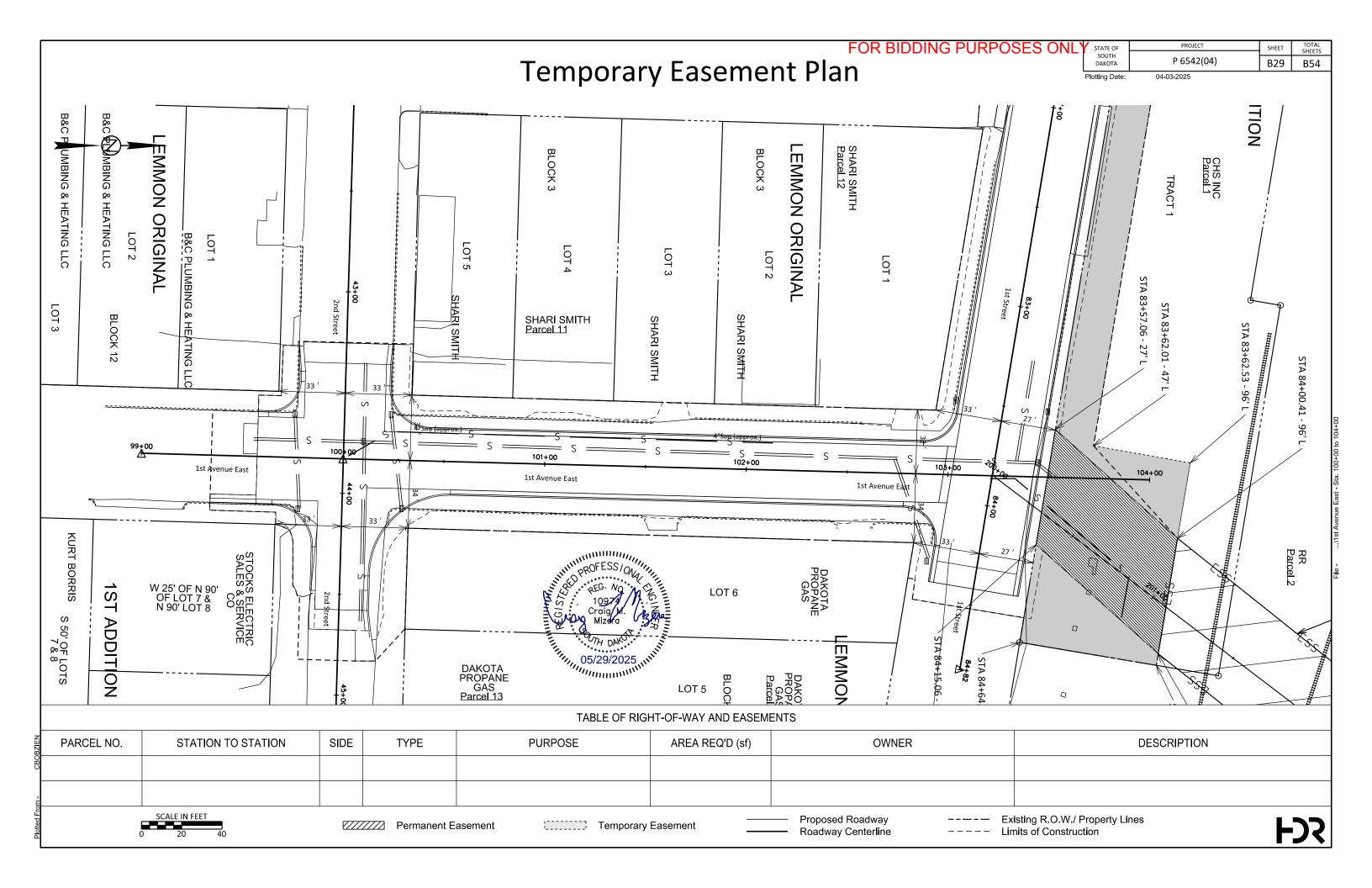


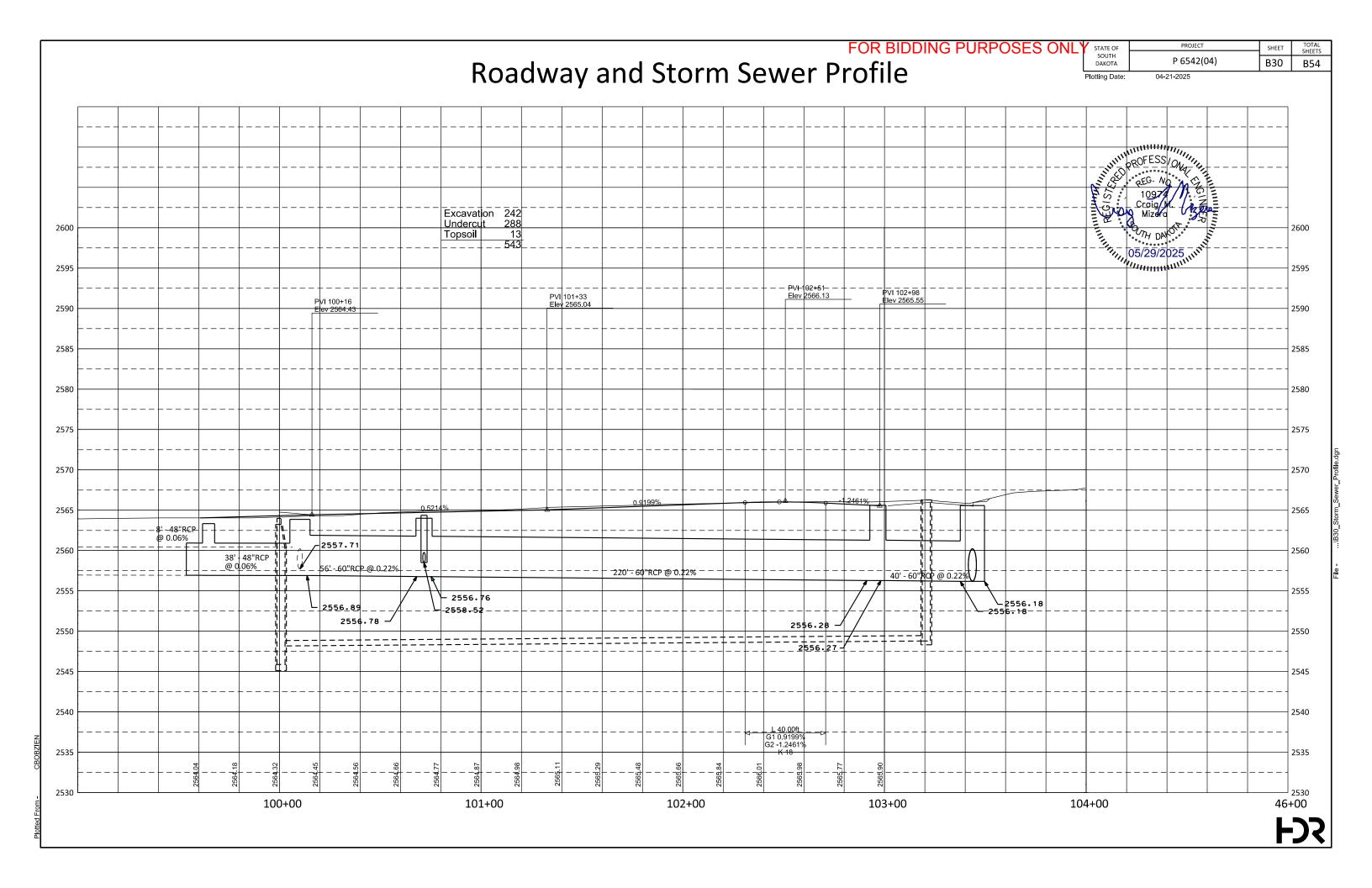


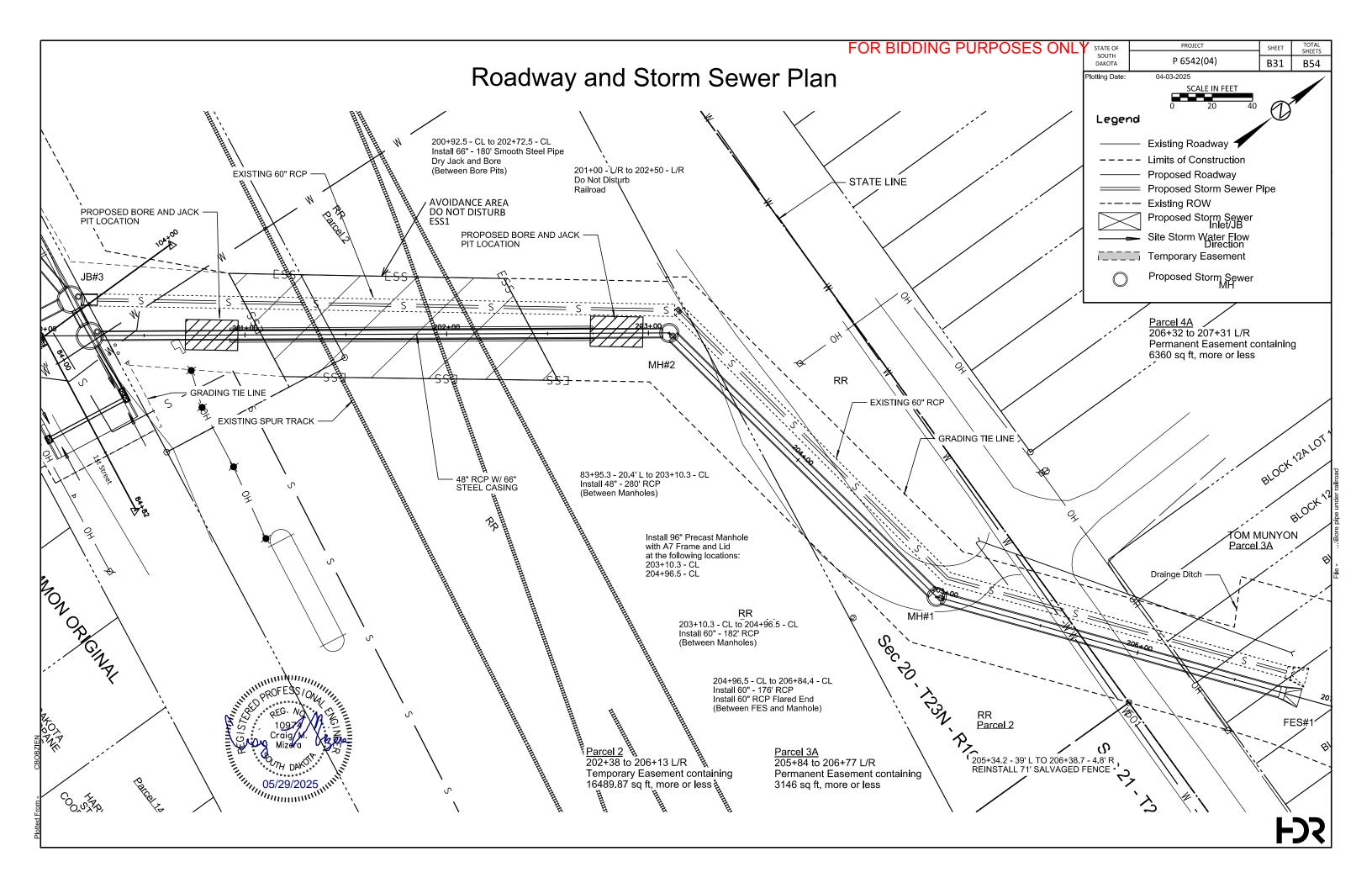


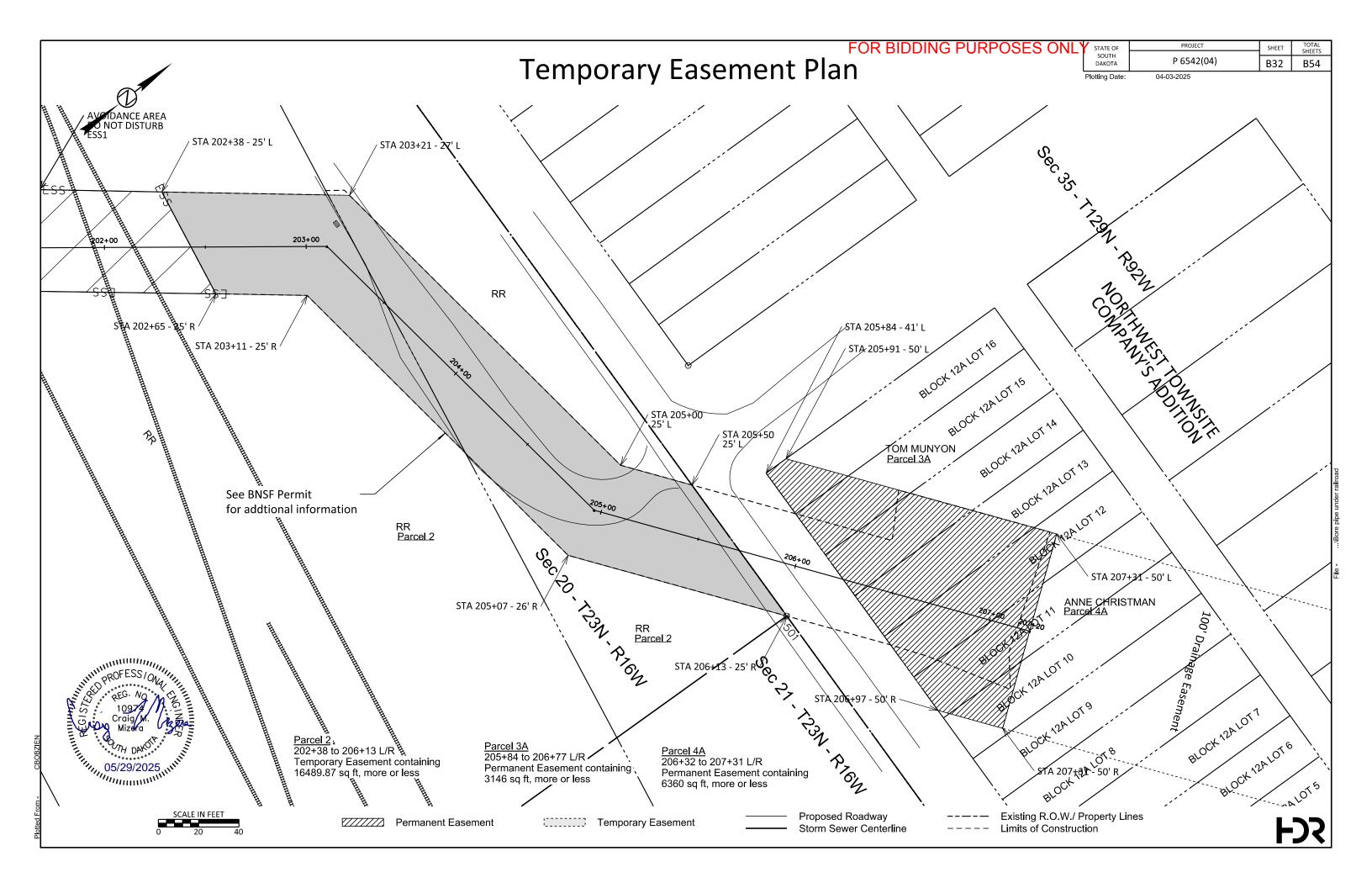


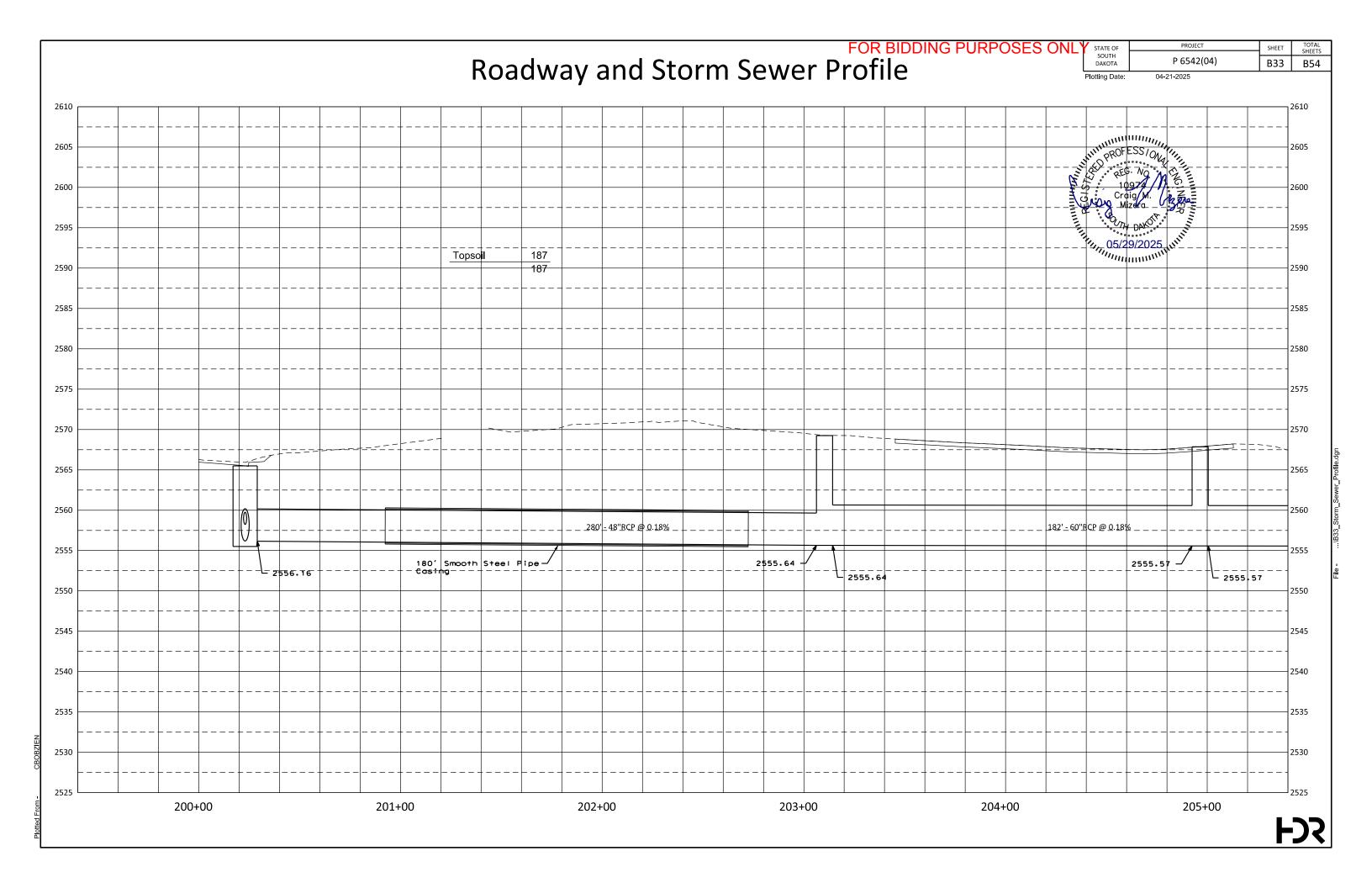


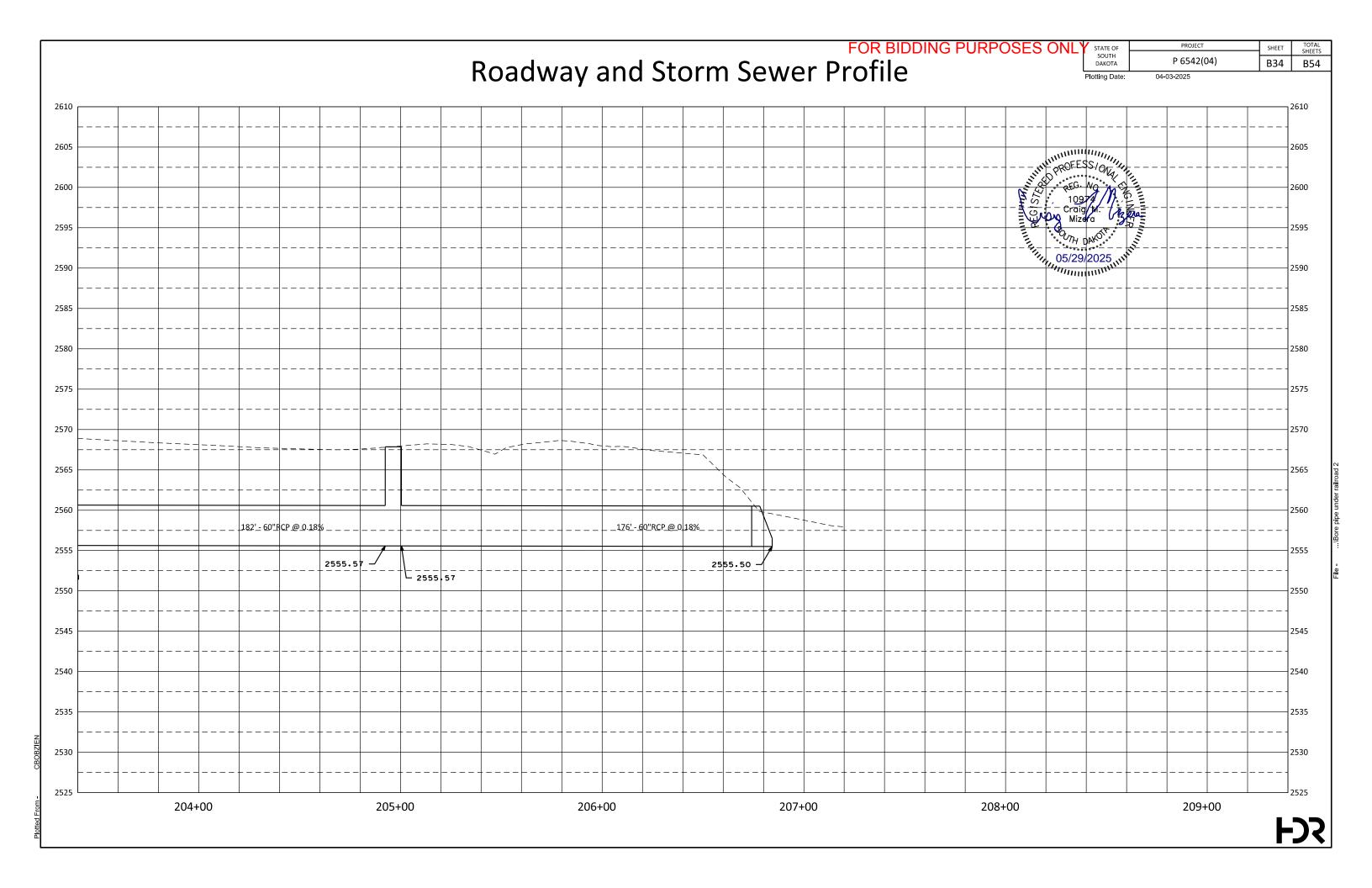










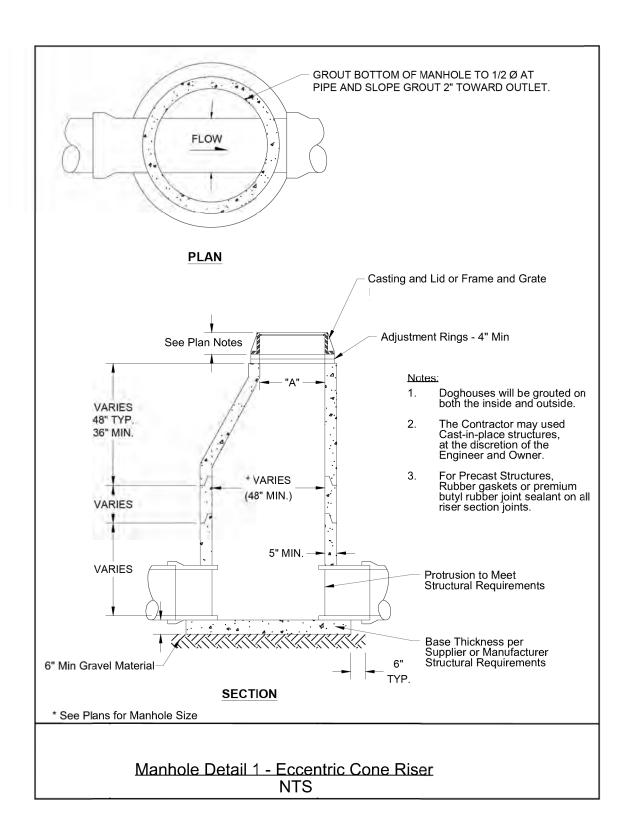


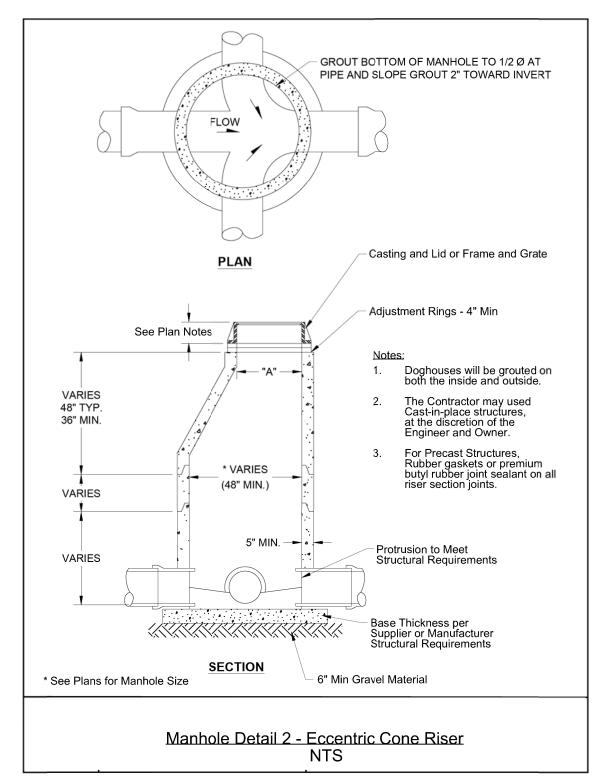
SHEET TOTAL SHEETS

D4) B35 B54

Plotting Date: 04-15-2025

# **Special Plates**





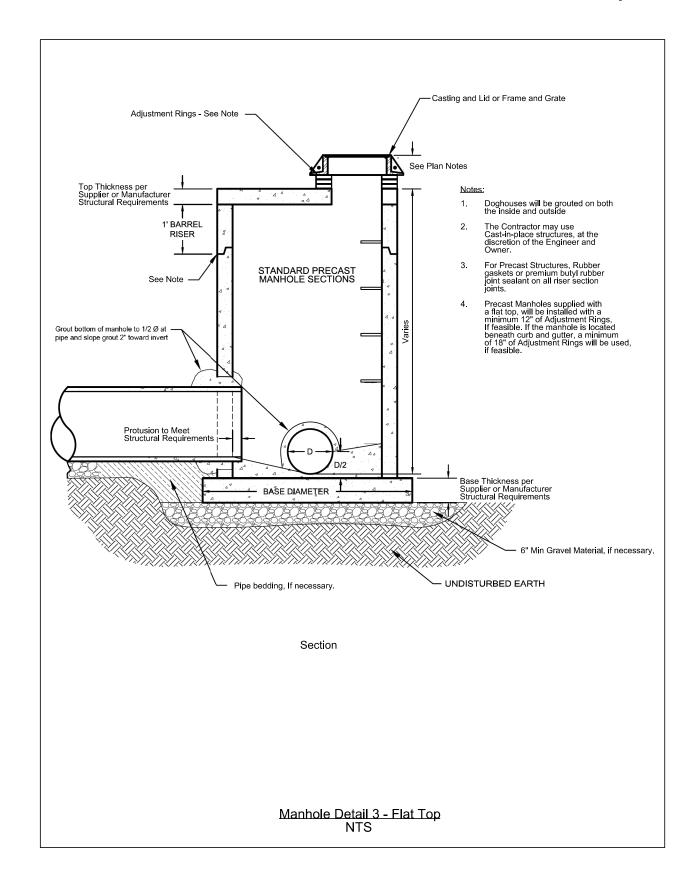


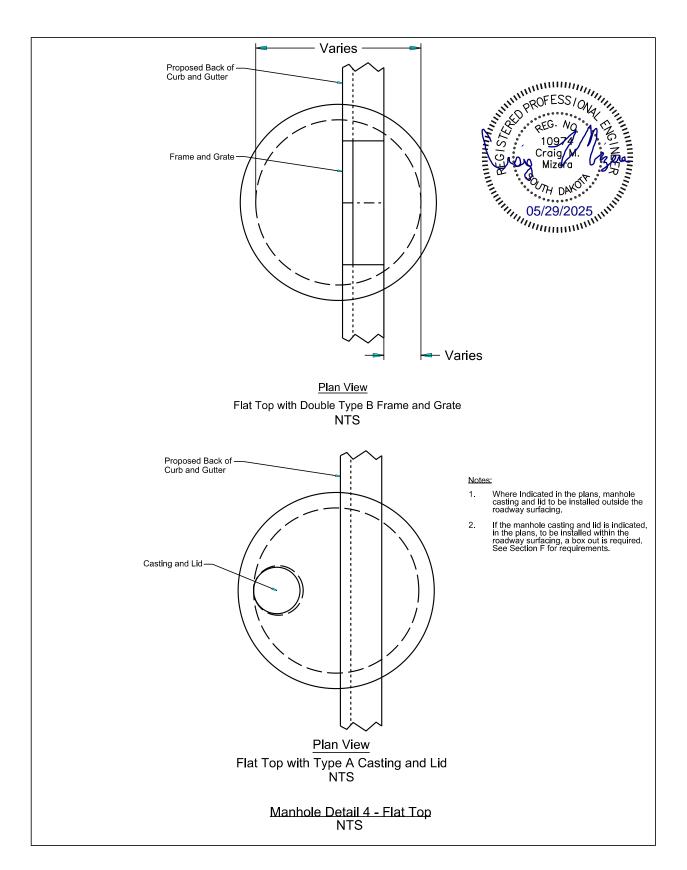
# **Special Plates**

FOR BIDDING PURPOSES ONLY

STATE OF PROJECT SHEET TOTAL SHEETS OUTH DAKOTA P 6542(04) B36 B54

Plotting Date: 04-03-2025





# **Special Plates**

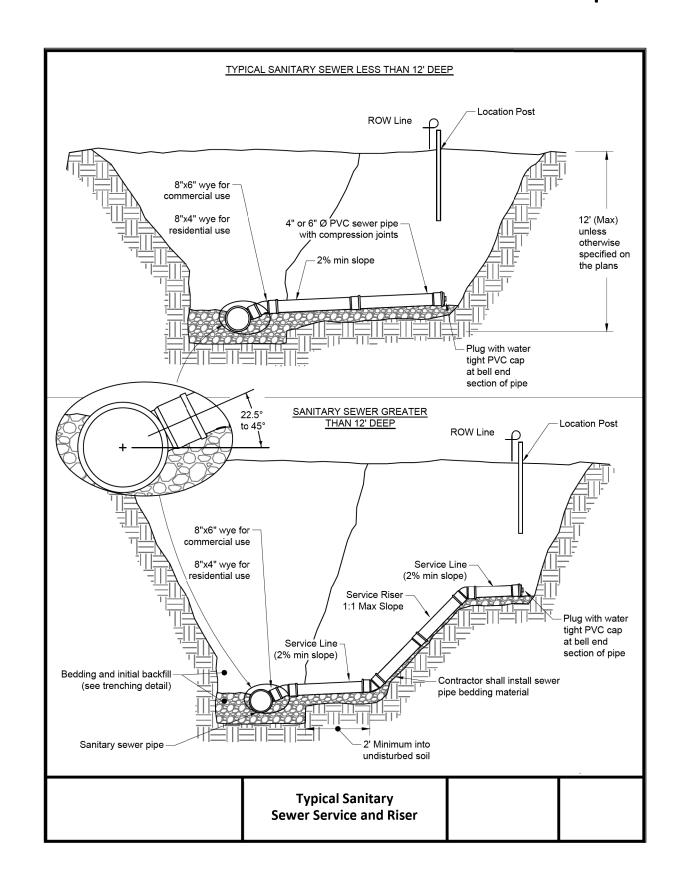
FOR BIDDING PURPOSES ONLY 5

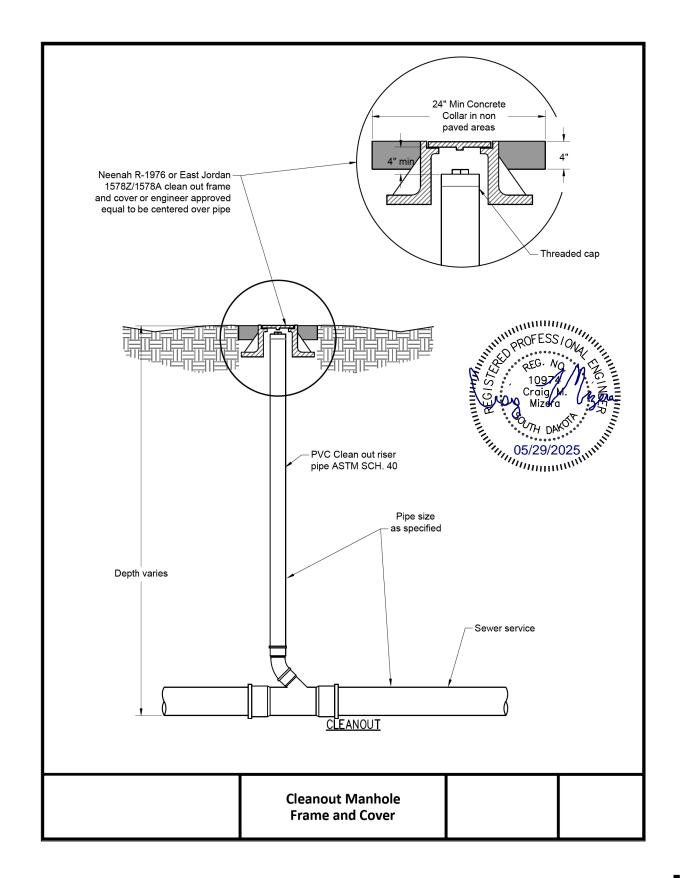
 STATE OF SOUTH DAKOTA
 PROJECT PROJECT
 SHEET

 P 6542(04)
 B37

B54

Plotting Date: 04-03-2025





# **Special Plate**

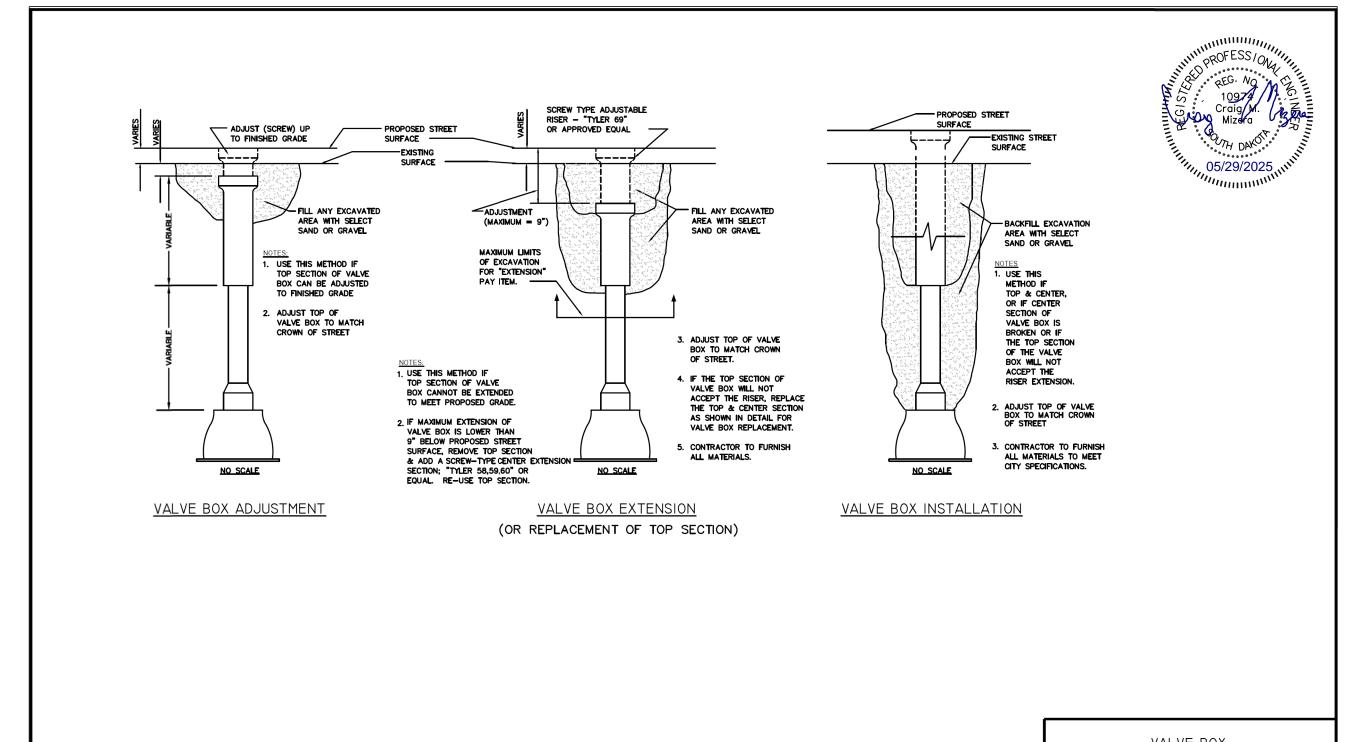
FOR BIDDING PURPOSES ONLY 57

STATE OF PROJECT SHEET
SOUTH
DAKOTA P 6542(04) B38

TOTAL SHEETS

B54

Plotting Date: 04-03-2025



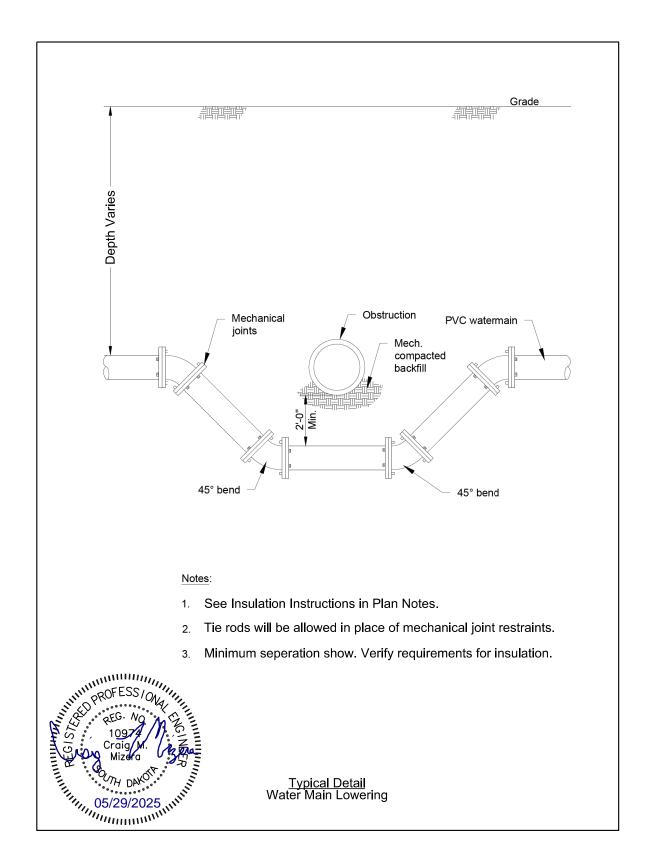
VALVE BOX ADJUSTMENT/ REPLACEMENT

Plotting Date: 04-03-2025

B39

B54

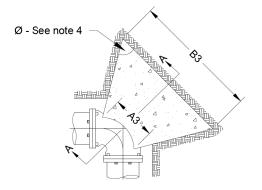
## **Special Plate**



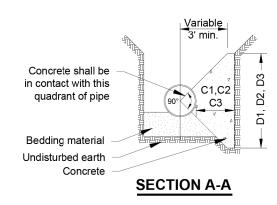
#### Notes:

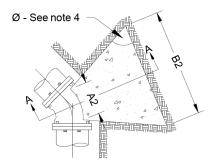
- 1. Shape of back of buttress may vary as long as poured against firm undisturbed earth.
- 2. Dimension C1, C2,& C3 should be large enough to make angle Ø equal to or larger than 45°.
- 3. Dimension A1, A2, & A3 should be as large as possible without interfering with MJ bolts.
- 4.  $\emptyset = 45^{\circ}$  Minimum.
- 5. Place polyethylene between concrete & pipe.

PIPE SIZE	22 1/2 °	BEND	45° B	END	90° BEND		
	B1	D1	B2	D2	В3	D3	
6"	1'-5"	1'-5"	1'-5"	1'-5"	1'-5"	1'-5"	
8"	1'-5"	1'-5"	2'-1"	1'-6"	2'-8"	2'-0"	
12"	1'-10"	1'-10"	3'-4"	2'-0"	4'-9"	2'-6"	
16"	3'-0"	2'-0"	3'-10"	3'-0"	6'-2"	3'-6"	
20"	3'-6"	2'-8"	5'-6"	3'-4"	8'-4"	4'-0"	
24"	4'-4"	3'-0"	6'-10"	3'-10"	6'-2"	3'-6"	
30"	-	-	9'-3"	6'-0"	17'-0"	6'-0"	

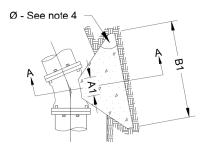








### **PLAN 45° BENDS**



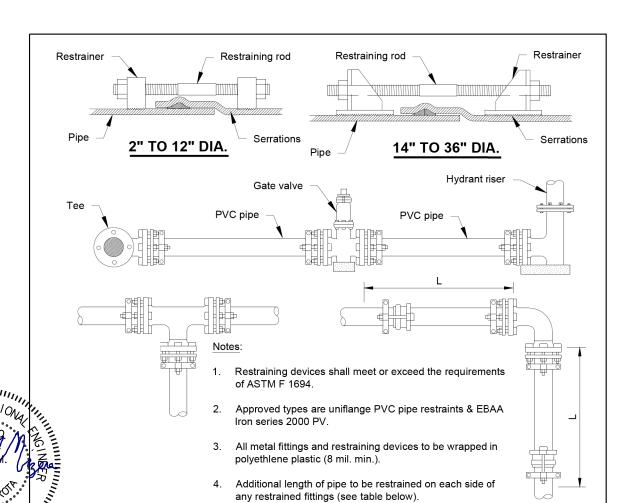
PLAN 22 1/2° BENDS

<u>Typical Detail</u> Concrete Thrust Block

## **Special Plate**

FOR BIDDING PURPOSES ONLY

Plotting Date: 04-03-2025



	RESTRAINED LENGTHS OF PVC PIPE										
NOM. PIPE SIZE	90° BEND (L)	45° BEND (L)	22.5° BEND (L)	11.25° BEND (L)	SIZE ON SIZE TEE (L)*	VALVE DEAD- END (L)					
6"	19'	8'	4'	2'	2'	35'					
8"	25'	11'	5'	3'	13'	45'					
10"	31'	13'	6'	3'	23'	55'					
12"	36'	15'	8'	4'	33'	65'					
16"	47'	20'	10'	5'	52'	84'					

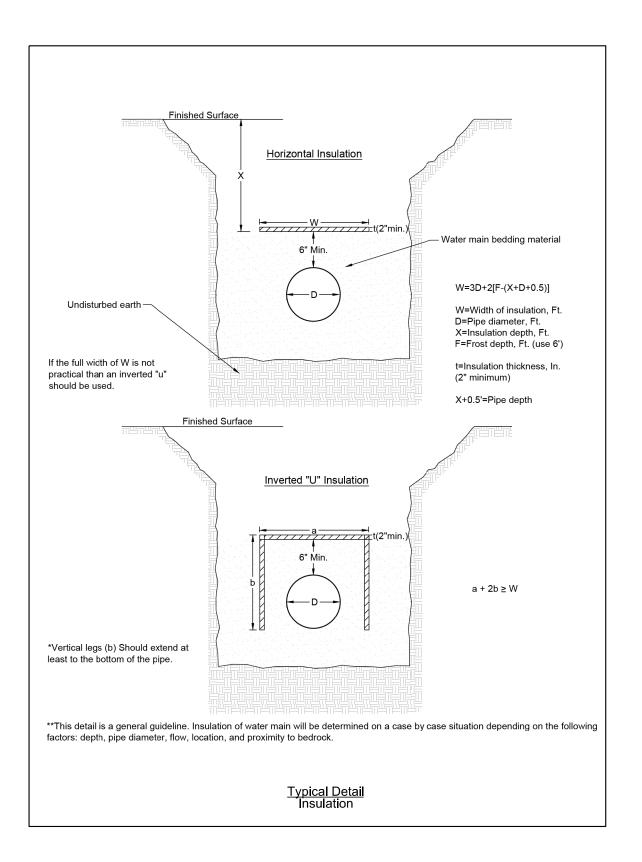
<sup>\*</sup> Recommended restrained lengths for tees are for the branch outlet and assume a minimum 10' section of pipe attached to each side of the run. Restraint devices are also required on both run joints of the tee itself.

SIZE	45° VERT. OFFSET * (L)	22.5° VERT. OFFSET * (L)
6"	15'/8'	7'/4'
8"	19'/11'	9'/5'
10"	23'/13'	11'/6'
12"	27'/15'	13'/8'
16"	35'/20'	17'/10'

<sup>\*</sup> First number is the recommended restrained length on each side of the down bend. The second number is the length for each side of the up bend.

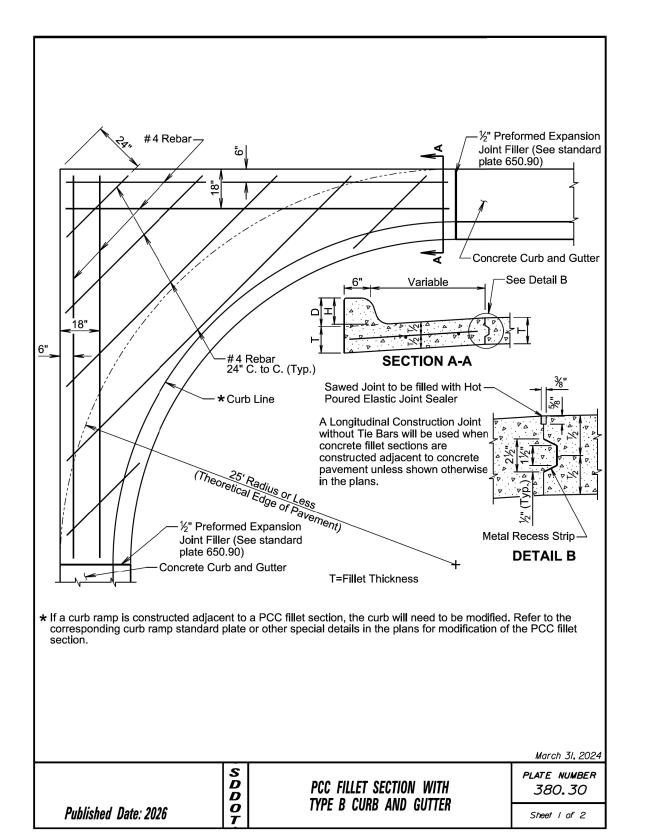
Typical Detail
Example of Restaining Rod

**TYPICAL INSTALLATIONS** 



Plotting Date:

### **Standard Plates**



#### **GENERAL NOTES:**

For fillets with irregular shapes or bump outs:

- 1) The 6" and 18" offset #4 rebar will be included on any side next to pavement or driveways (not along the Curb and Gutter).
- 2) All remaining area will have #4 rebar spaced 24" center to center in a square pattern.

Dimensions D, H, and T will conform to those shown on the appropriate curb and gutter standard plate.

All rebar will be in conformance with Sections 480 and 1010 of the Specifications. All rebar will have a minimum of 3 inches of clear cover.

Class M6 Concrete will be used in construction of the fillets.

The concrete curb will be monolithic with the concrete fillet. No separate payment for this curb will be made as the curb is considered a part of the fillet.

Joints will be constructed at 10-foot intervals except when fillets are constructed adjacent to PCC Pavement. If there is adjacent PCC Pavement the joints will be extended from edge of pavement through the fillet section as directed by the Engineer.

The cost for all materials, labor, and incidentals necessary to construct the PCC fillet section with curb and gutter will be incidental to the contract unit price per square yard for the corresponding PCC fillet section contract item.

March 31, 2024

Published Date: 2026

PCC FILLET SECTION WITH TYPE B CURB AND GUTTER

PLATE NUMBER 380.30

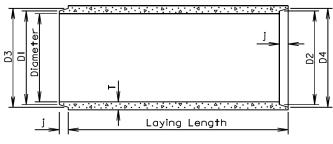
Sheet 2 of 2

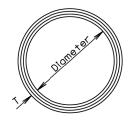
Typical Inslope

### TOLERANCES IN DIMENSIONS

Diameter:  $\pm 1.5\%$  for 24" Dia. or less and  $\pm 1\%$  or  $\frac{3}{6}$ " whichever is more for 27" Dia. or greater. Diameters at joints:  $\pm \frac{3}{6}$ " for 30" Dia. or less and  $\pm \frac{1}{4}$ " for 36" or greater. Length of joint (j):  $\pm \frac{1}{4}$ ".

Wall thickness (T): not less than design T by more than 5% or  $\frac{3}{6}$ , whichever is greater. Laying length: shall not underrun by more than  $\frac{1}{2}$ ".





**Standard Plates** 

LONGITUDINAL SECTION

END VIEW

### GENERAL NOTES:

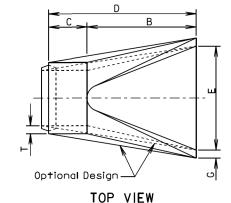
Construction of R.C.P. shall conform to the requirements of Section 990 of the Specifications.

Not more than 2 four-foot sections shall be permitted near the ends of any culvert. Four-foot lengths shall be used only to secure the required length of culvert.

Diam. (in.)	Approx. Wt./Ft. (Ib.)	T (in.)	J (in.)	DI (in.)	D2 (in.)	D3 (in.)	D4 (in.)
12	92	2	13/4	131/4	13%	13%	141/4
15	127	21/4	2	161/2	167/8	171/4	175/8
18	168	21/2	21/4	195/8	20	20%	20¾
21	214	23/4	21/2	22 1/8	231/4	23¾	241/8
24	265	3	23/4	26	26¾	27	273/8
27	322	31/4	3	291/4	295/8	301/4	30%
30	384	31/2	3 <sup>1</sup> / <sub>4</sub>	323/8	32¾	331/2	33%
36	524	4	33/4	38¾	391/4	40	401/2
42	685	41/2	4	45 <sup>1</sup> / <sub>8</sub>	45%	461/2	47
48	867	5	41/2	511/2	52	53	531/2
54	1070	51/2	41/2	57%	58 <b>¾</b>	59¾	59%
60	1296	6	5	641/4	64¾	66	661/2
66	1542	61/2	51/2	70%	711/8	721/2	73
72	1810	7	6	77	771/2	79	791/2
78	2098	71/2	61/2	83%	83%	85%	861/8
84	2410	8	7	89¾	901/4	921/8	925/8
90	2740	81/2	7	95¾	961/4	981/8	985/8
96	2950	9	7	1021/8	1025/8	1041/2	105
102	3075	91/2	71/2	109	1091/2	1111/2	112
108	3870	10	71/2	1151/2	116	118	1181/2

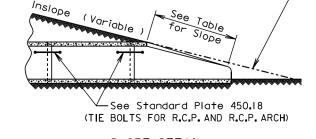
June 26, 2015

	S D D	REINFORCED CONCRETE PIPE	plate number 450.01
Published Date: 2026			Sheet I of I



-Tongue (Inlet) or Groove (Outlet)

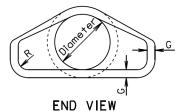
### GENERAL NOTES:



SLOPE DETAIL

Lengths of concrete pipe shown on plan sheets are between flared ends only.

Construction of R.C.P. Flared End shall conform to the requirements of Section 990 of the Specifications.



LONGITUDINAL	SECTION
LONGINODINAL	0_0.10.1

Dia. (in.)	Approx. Wt. of Section (Ibs.)	Approx. Slope (X to Y)	T (in.)	A (in.)	B (in.)	C (in.)	D (in.)	E (in.)	G (in.)	R (in₌)
12	530	2.4: 1	2	4	24	48 1/8	72%	24	2	11/2
15	740	2.4: 1	21/4	6	27	46	73	30	21/4	11/2
18	990	2.3: I	21/2	9	27	46	73	36	21/2	11/2
21	1280	2.4: I	23/4	9	36	371/2	731/2	42	23/4	11/2
24	1520	2.5: I	3	91/2	431/2	30	731/2	48	3	11/2
27	1930	2.5: I	31/4	101/2	491/2	24	731/2	54	31/4	11/2
30	2190	2 <b>.</b> 5: I	31/2	12	54	19¾	73¾	60	31/2	11/2
36	4100	2 <b>.</b> 5: I	4	15	63	34¾	973/4	72	4	11/2
42	5380	2 <b>.5:</b> I	41/2	21	63	35	98	78	41/2	11/2
48	6550	2.5: I	5	24	72	26	98	84	5	11/2
54	8240	2: 1	51/2	27	65	331/4	981/4	90	51/2	11/2
60	8730	1.9:1	6	35	60	39	99	96	5	11/2
66	10710	1.7:1	61/2	30	72	27	99	102	51/2	11/2
72	12520	1.8:1	7	36	78	21	99	108	6	11/2
78	14770	1.8: I	71/2	36	90	21	111	114	61/2	11/2
84	18160	1.6: I	8	36	901/2	21	1111/2	120	61/2	11/2
90	20900	1.5:1	81/2	41	871/2	24	1111/2	132	61/2	6

June 26, 2015

SDDOT

Published Date: 2026

R. C. P. FLARED ENDS

PLATE NUMBER 450.10

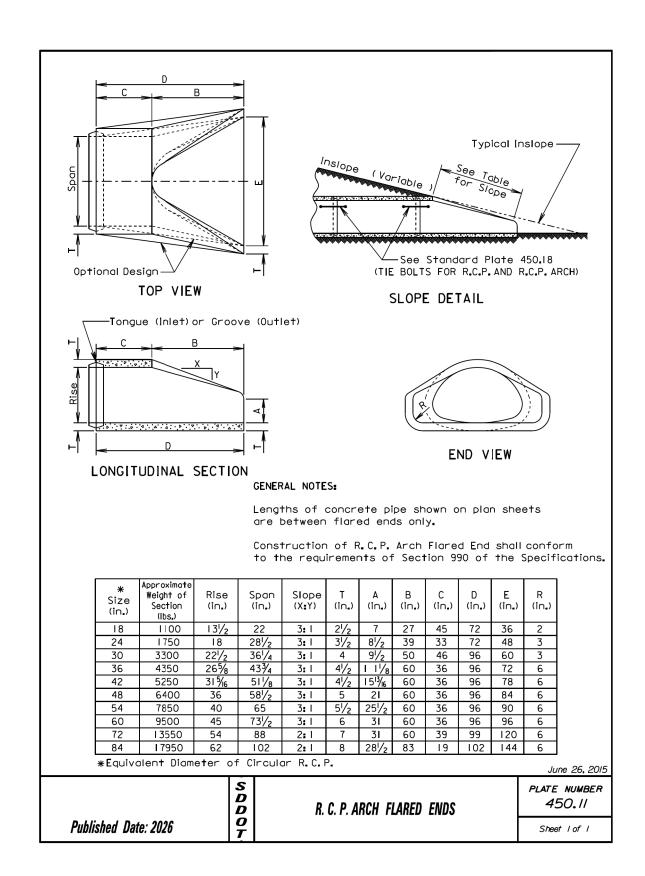
Sheet I of I

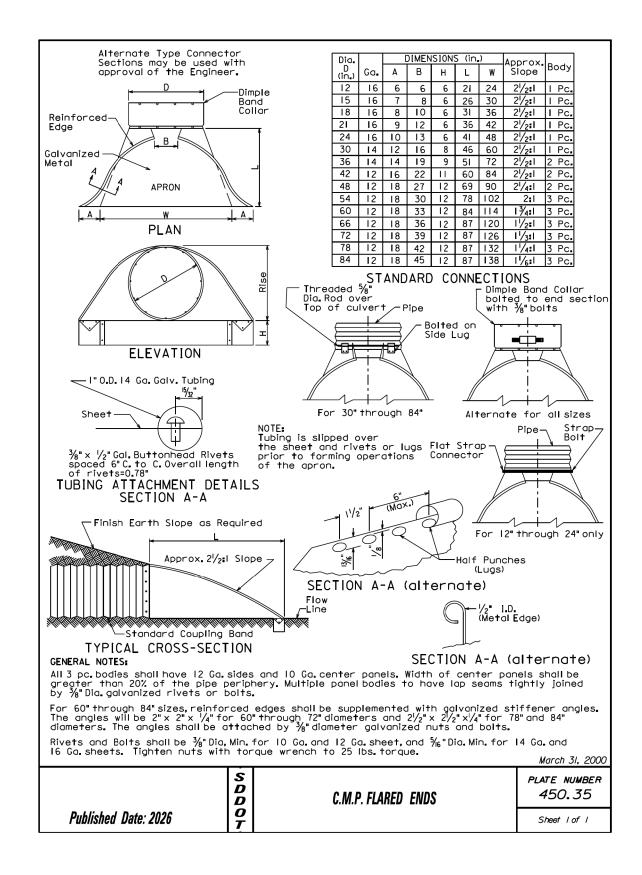
SHEET TOTAL SHEETS

B43 B54

Plotting Date: 06-18-2025

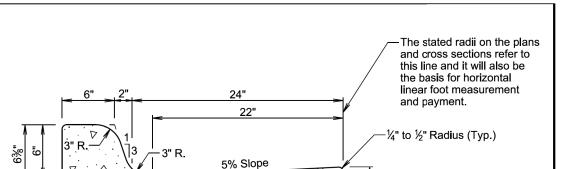






PROJECT

P 6542(04)



TYPE B CONCRETE CURB AND GUTTER								
ITEB			Cu. Yd.					
Type	T <sub>1</sub>	T2	Per	Per				
. 71	(Inches)	(Inches)		Cu. Yd.				
B66	6	51⁄16	0.057	17.7				
B67	7	61/16	0.065	15.4				
B68	8	7½ <sub>6</sub>	0.073	13.7				
B68.5	8.5	<b>7</b> % <sub>16</sub>	0.077	13.0				
B69	9	81/16	0.081	12.3				
B69.5	9.5	8%6	0.085	11.7				
B610	10	91/16	0.090	11.2				
B610.5	10.5	9%6	0.094	10.7				
B611	11	101/16	0.098	10.2				
B611.5	11.5	10%	0.102	9.8				
B612	12	111/16	0.106	9.4				

2% Slope

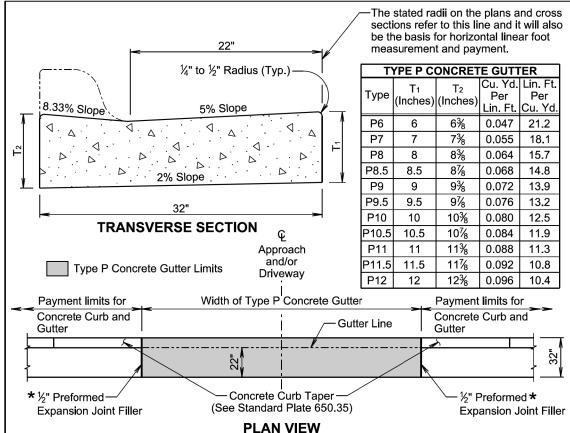
32"

#### **GENERAL NOTES:**

When concrete curb and gutter longitudinally adjoins new concrete pavement, the method of attachment will be by one of the methods shown on standard plate 380.21.

See standard plate 650.90 for expansion and contraction joints in the curb and gutter.

			January 22 <b>,</b> 2023
	S D D	TYPE B CONCRETE CURB AND GUTTER	PLATE NUMBER 650.01
Published Date: 2026	$  \frac{o}{T}  $		Sheet I of I



★ Joint will not be needed if concrete curb and gutter and type P concrete gutter is placed at the same time. If the ½" preformed expansion joint filler is provided, then the joint will be sealed in accordance with standard plate 650.90.

### **GENERAL NOTES:**

**Standard Plates** 

The concrete for the type P concrete gutter will comply with the requirements of the specifications for class

When concrete gutter longitudinally adjoins new concrete pavement, the method of attachment will be by one of the methods shown on standard plate 380.21.

Transverse contraction joints will be constructed at 10-foot intervals in the concrete gutter except when concrete gutter is constructed adjacent to mainline PCC pavement. When concrete gutter is constructed adjacent to mainline PCC pavement, a transverse contraction joint will be constructed in the concrete gutter at each mainline PCC pavement transverse contraction joint location.

When concrete gutter is placed monolithically with mainline PCC pavement, the transverse contraction joints in the concrete gutter will be sawed and sealed the same as the transverse contraction joints in the mainline PCC pavement.

When concrete gutter is not placed monolithically with the mainline PCC pavement and when the adjacent mainline surfacing is not PCC concrete, the transverse contraction joints in the concrete gutter will be 11/3 inches deep if formed in the fresh concrete using a suitable grooving tool. If a saw is used to cut the contraction joints, then the depth of the joint will be at least  $\frac{1}{4}$  the thickness of the concrete.

January 22 <b>,</b> 202
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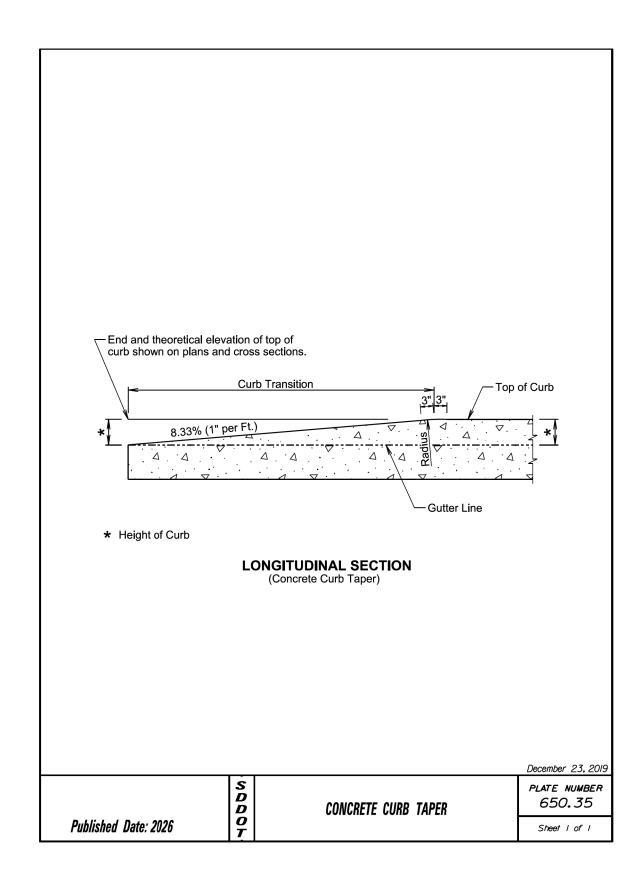
	D D	TYPE P CONCRETE GUTTER	650.30
Published Date: 2026	O   T		Sheet I of I

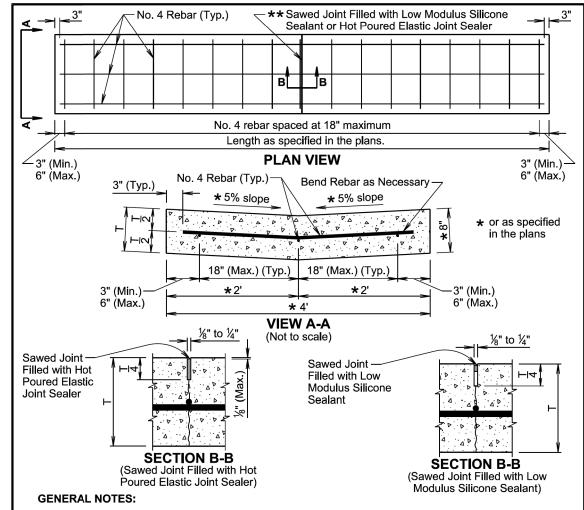
PROJECT STATE OF SHEET SOUTH DAKOTA P 6542(04) B45

B54

Plotting Date: 06-18-2025

### **Standard Plates**





The concrete will comply with the specifications for class M6 concrete.

The reinforcing steel will comply with the requirements of specification sections 480 and 1010.

If a lap splice is provided the No. 4 rebar will be lapped a minimum of 12 inches.

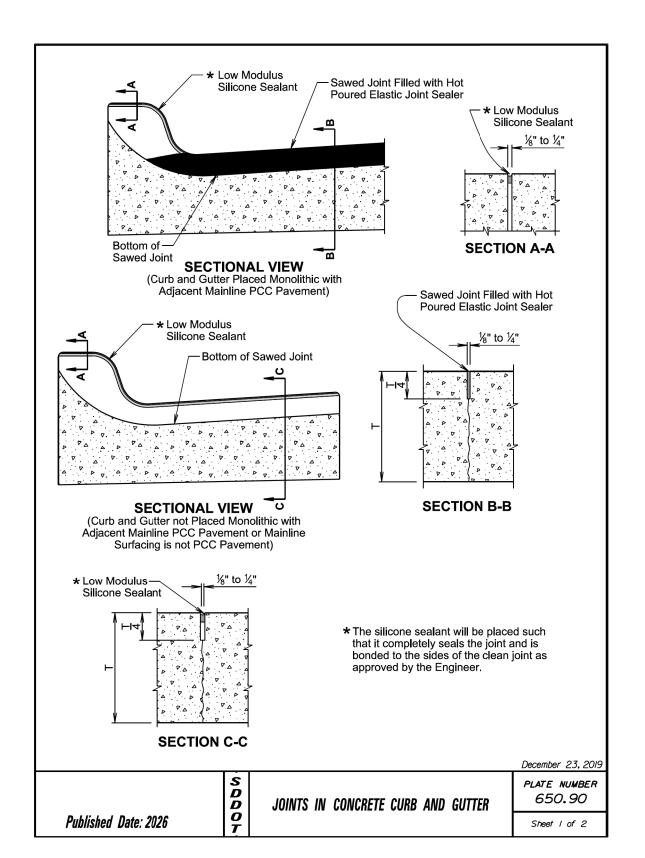
\*\* The sawed joints will be spaced at 12 feet; however, when the length of the valley gutter is 12 feet to 24 feet there will be a joint at the midpoint of the length. The saw cut to control cracking will be a minimum of ¼ the thickness of the pavement.

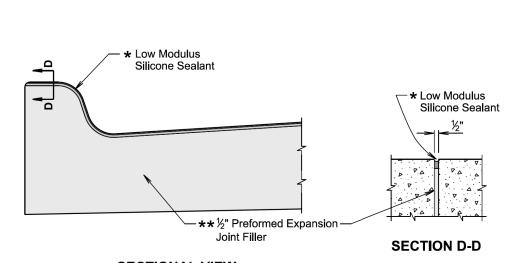
All hot poured elastic joint sealer material spilled on the surface of the concrete pavement will be removed as soon as the material has cooled. The extent of removal of material will be to the satisfaction of the Engineer. All costs for removal of the spilled joint sealer material will be borne by the Contractor.

The silicone sealant will be bonded to the sides of a clean joint to completely seal the joint as approved by the Engineer.

All costs for furnishing and installing the valley gutter including materials, equipment, labor, and incidentals will be included in the contract unit price per square yard for the corresponding Valley Gutter contract item.

	S D D	VALLEY GUTTER	PLATE NUMBER 650.40
olished Date: 2026			Sheet I of I





### SECTIONAL VIEW (Curb and Gutter at ½" Preformed Expansion Joint Filler Location)

\* The silicone sealant will be placed such that it completely seals the joint and is bonded to the sides of the clean joint as approved by the Engineer.

### **GENERAL NOTES:**

For illustrative reason, only the type B curb and gutter is shown.

★★ A ½-inch preformed expansion joint filler will be placed transversely in the curb and gutter at the following locations:

At each junction between the radius return of curb and gutter, and curb and gutter which is parallel to the project centerline.

At each junction between new curb and gutter and existing curb and gutter.

Transverse contraction joints will be constructed at 10 foot intervals in the concrete curb and gutter except when the concrete curb and gutter is constructed adjacent to mainline PCC pavement. When concrete curb and gutter is constructed adjacent to mainline PCC pavement, a transverse contraction joint will be constructed in the concrete curb and gutter at each mainline PCC pavement transverse contraction joint location.

When concrete curb and gutter is not placed monolithically with the mainline PCC pavement or when the adjacent mainline surfacing is not PCC concrete, the transverse contraction joints in the concrete curb and gutter will be  $1\frac{1}{2}$  inches deep if formed in the fresh concrete using a suitable grooving tool. If a saw is used to cut the contraction joints, then the depth of the joint will be at least  $\frac{1}{4}$  the thickness of the concrete and the joint will be sealed in accordance with the details shown above.

December	23.	2019

PLATE NUMBER 650.90

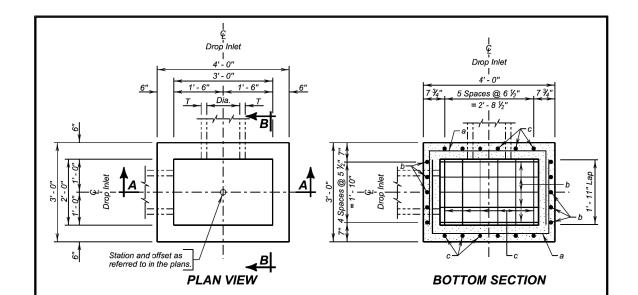
Sheet 2 of 2

SHEET TOTAL SHEETS

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Plotting Date: 06-18-2025





ESTIMATED QUANTITIES						
ITEM	UNIT	CONSTANT QUANTITY	VARIABLE QUANTITY			
→ Class M6 Concrete	Cu. Yd.	0.26	0.22H			
Reinforcing Steel	Lb.	51.19	28.97H			
Frame and Grate Assembly	Each	1				

#### DROP INLETS FOR 12" TO 24" DIAMETER PIPE

#### **SPECIFICATIONS**

Design Specifications: AASHTO LRFD Bridge Design Specifications, 2012 Edition.

Construction Specifications: South Dakota Standard Specifications for Roads and Bridges, Current Edition and required Provisions, Supplemental Specifications, and Special Provisions as included in the Proposal.

### **GENERAL NOTES:**

Design Live Load: HL-93. No construction loading in excess of legal load was considered.

Reinforcing steel shall conform to ASTM A615 grade 60. The d bars shall be lapped 12 inches with the b and c bars. Cut and bend reinforcing steel as required to place pipe(s) through the drop inlet wall.

Drop inlet may be precast. If precast drop inlet details differ from this standard plate, submit a checked design done by a SD registered P.E. and shop plans to the Office of Bridge Design for approval.

\* Reduce total quantities of concrete by the amount of concrete displaced by the pipe(s). The total quantity of concrete shall be computed to the nearest hundredth of a cubic yard. The total quantity of reinforcing steel shall be computed to the nearest pound.

Drop inlet shown may be modified by the addition or omission of connecting pipes as noted elsewhere in the plans. All pipes entering drop inlet must fit between the inside face of walls and shall not enter through the corners.

Maximum R.C.P. diameter shall not exceed 18 inches on the 2-foot wide side and shall not exceed 24 inches (24 inches for R.C. arch) on the 3-foot wide side of the drop inlet.

		PIPE DISPLACEMENT REDUCTIONS				
		Diameter (Inches)	Wall T (Inches)	Class M6 Concrete (Cu. Yd.)		
ı		12	2	0.03		
ı	R.C.P.	15	2 1/4	0.04		
ı	R.C	18	2 1/2	0.05		
ı		24	3	0.09		
ı	Ή	18	2 ½	0.05		
ı	RC	24	3 1/2	0.09		
R.C. A	R.C. ARCH					

S D D

0

The dimension of H is in feet. Maximum H is 10 feet.

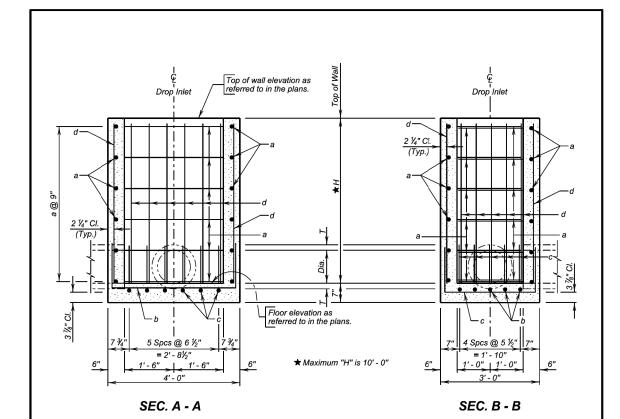
Published Date: 2026

2'X 3'TYPE B REINFORCED CONCRETE DROP INLET March 31, 2024

PLATE NUMBER

670.01

Sheet I of 2



REINFORCING SCHEDULE					
Mk.	No.	Size	Length	Туре	Bending Details
а	2.67H	4	8' - 0"	17	1 1 1
b	5	5	6' - 3"	17	2
С	6	4	5' - 3"	17	
d	22	4	H - 2"	Str.	
NOTE: All dimensions are out to out of bars.		of bars.			
a 2 - 2 ½ b 1' - 3 ½' c 1' - 3 ½'					

March 31, 2024

Published Date: 2026

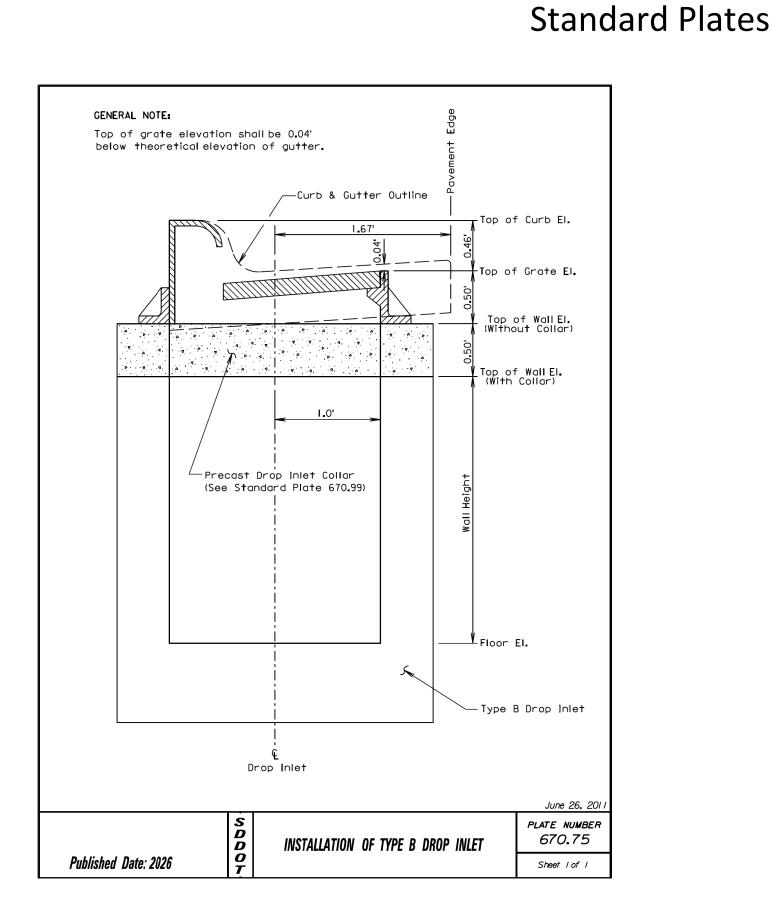
2'X 3'TYPE B REINFORCED CONCRETE DROP INLET PLATE NUMBER 670.01 Sheet 2 of 2

STATE OF PROJECT
SOUTH
DAKOTA P 6542(04)

sheet TOTAL SHEETS

4) B48 B54

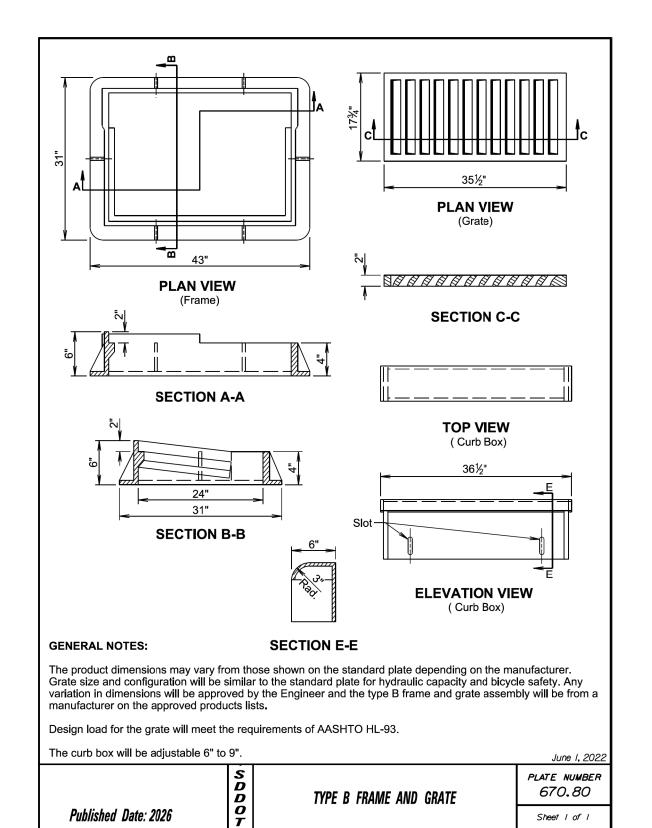
Plotting Date: 06-18-2025

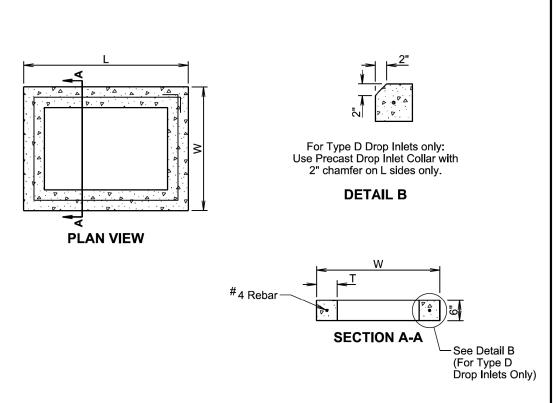


SHEET B49 B54

06-18-2025 Plotting Date:







INFORMATIONAL QUANTITIES					
FRAME AND GRATE TYPE	L (Ft-in)	W (Ft-in)	T (in)	CLASS M6 CONCRETE (CuYd)	REINFORCING STEEL (Lb)
TYPE A, B, and E	4'-0"	3'-0"	6	0.11	9
TYPE C	5'-0"	4'-0"	6	0.15	11
TYPE D	4'-0"	2'-6"	6	0.10	8

### **GENERAL NOTES:**

All reinforcing steel will conform to ASTM A615, Grade 60.

The  $\frac{1}{2}$ " diameter bar will lap 6"± and will be centered in the concrete.

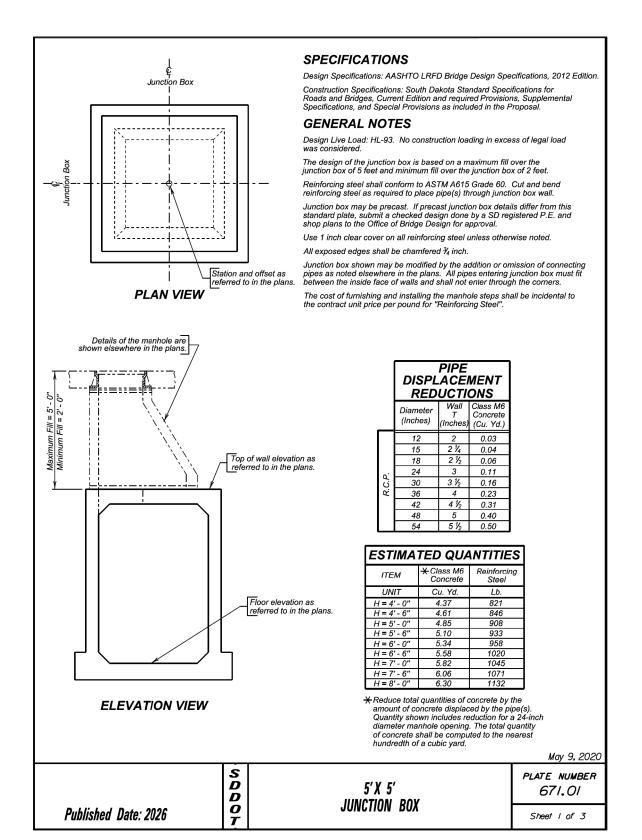
The cost of furnishing and installing Precast Drop Inlet Collars, including labor, materials, and incidentals will be incidental to the contract unit price per Each for "Precast Drop Inlet Collar".

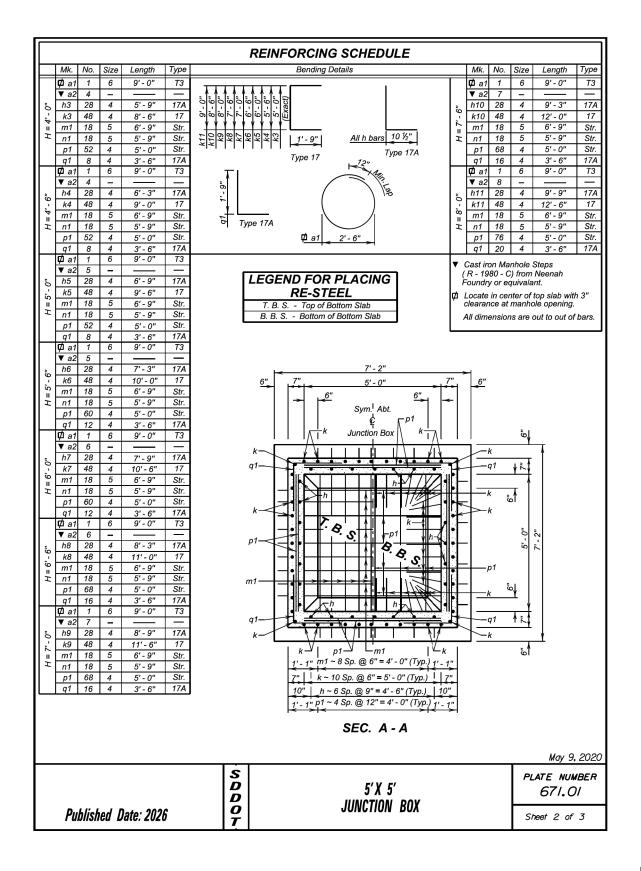
June 1, 2022

	S D D O T	PRECAST DROP INLET COLLAR	plate number 670.99
Published Date: 2026			Sheet I of I

Plotting Date: 06-18-2025

### **Standard Plates**





**Standard Plates** 

May 9,2020

PLATE NUMBER

671.01

Sheet 3 of 3

TOTAL SHEETS

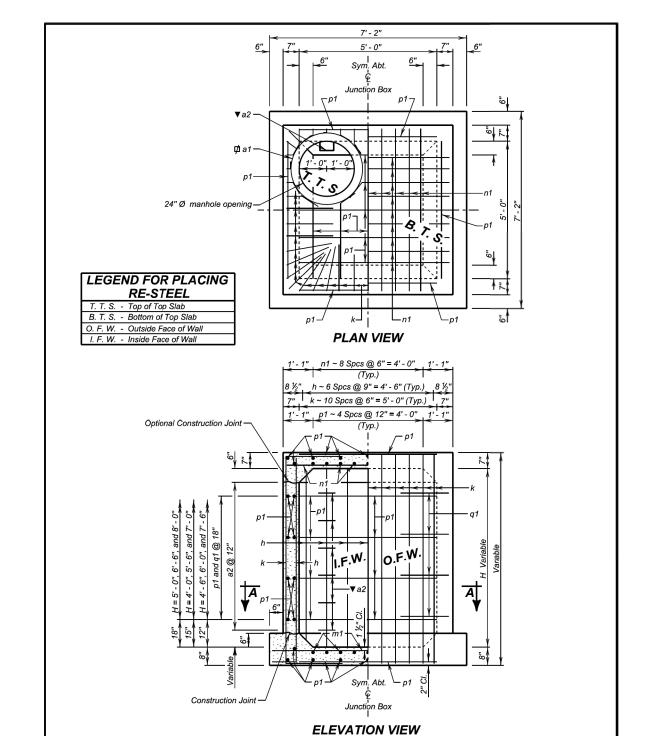
B54

SHEET

B51

Plotting Date: 06-18-2025





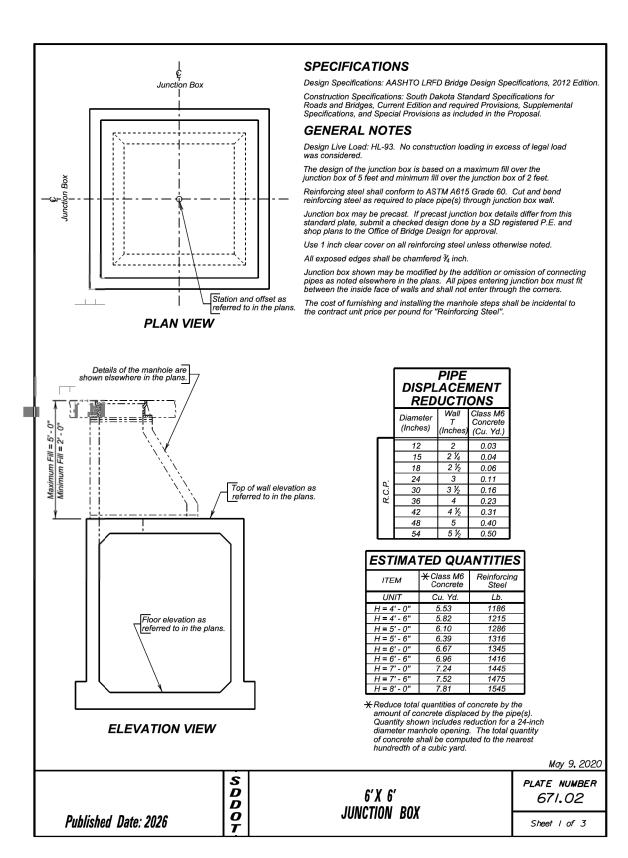
D

D O

Published Date: 2026

5' X 5'

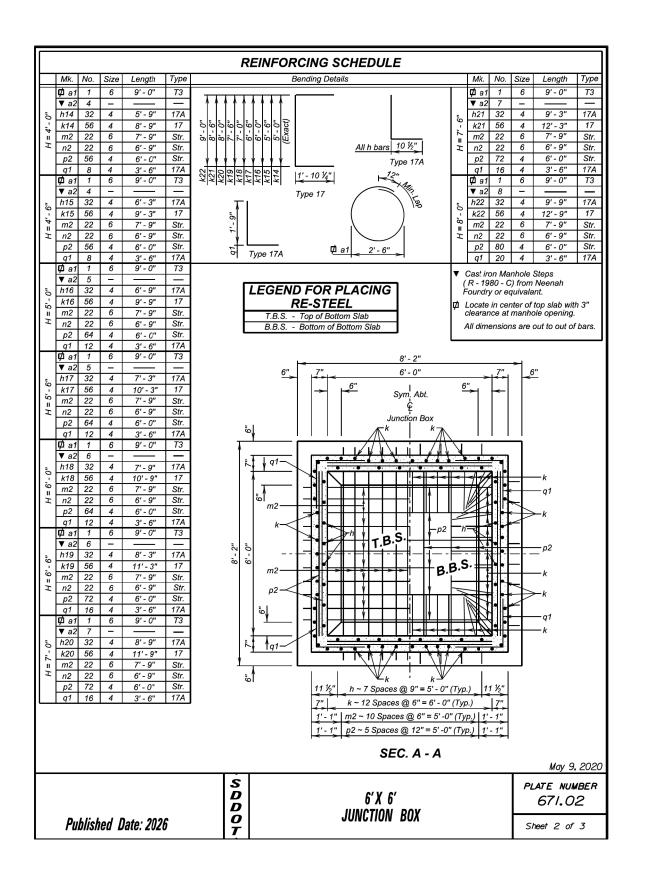
JUNCTION BOX

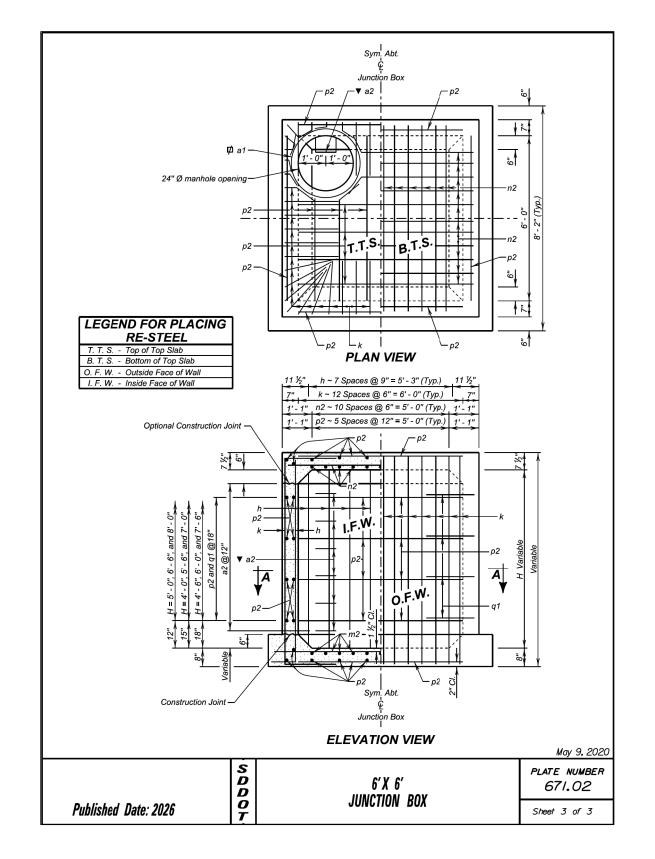


## Standard Plates

FOR BIDDING PURPOSES/8145 STATE OF SOUTH DAKOTA P 6542(04)

Plotting Date: 06-18-2025





TOTAL SHEETS

B54

SHEET

B52

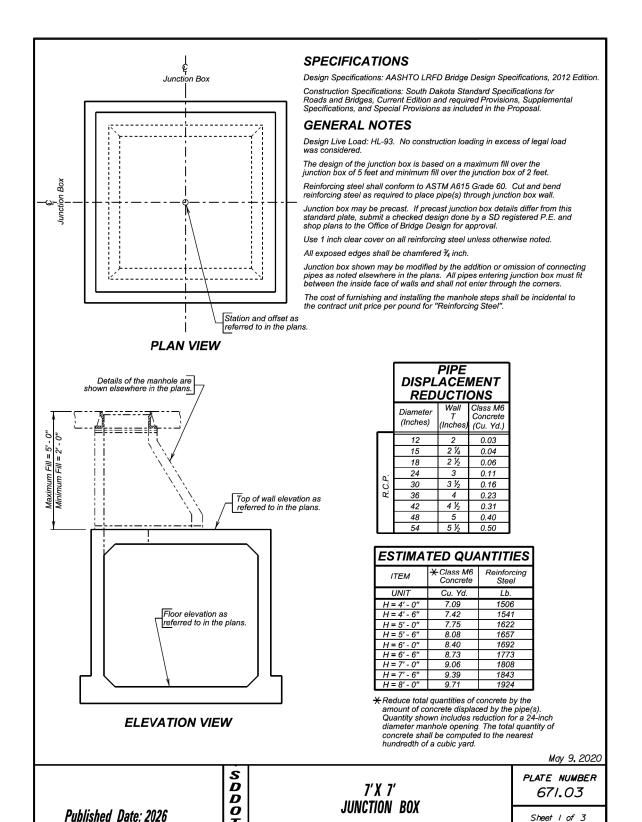
STATE OF PROJECT
SOUTH
DAKOTA P 6542(04)

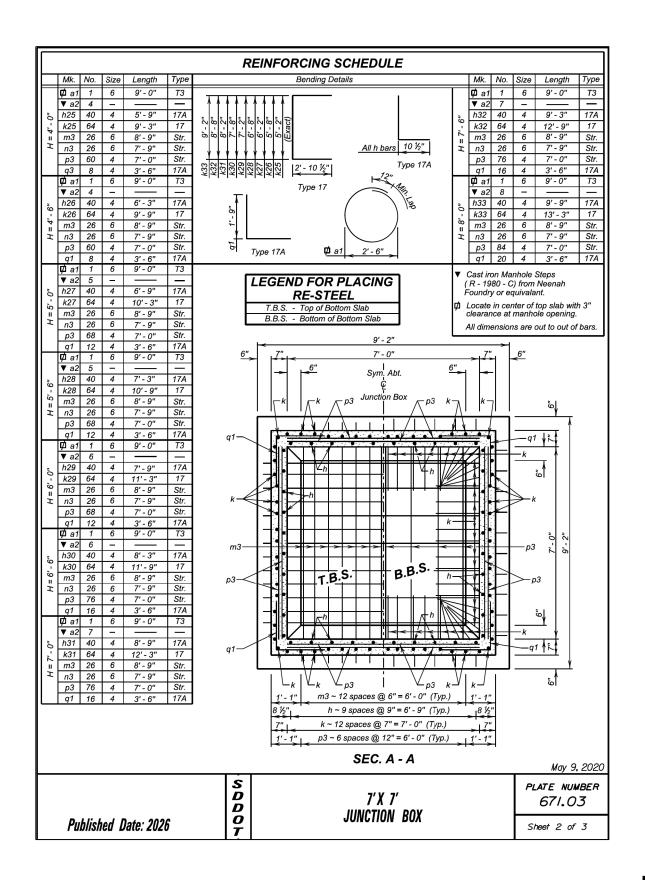
SHEET TOTAL SHEETS

B53 B54

Plotting Date: 06-18-2025

### **Standard Plates**





SHEET TOTAL SHEETS
B54 B54

Plotting Date: 06-18-2025

## **Standard Plates**

