

STATE OF	PROJECT	SHEET	TOTAL SHEETS
SOUTH DAKOTA	2011 YANKTON AREA CRCP & PCCP REPAIR	2	24

ESTIMATE OF QUANTITIES

BID ITEM NUMBER	ITEM	050W-292 PCN I23D QUANTITY	029S-291 PCN I23E QUANTITY	029N-291 PCN I23F QUANTITY	081-292 PCN I23G QUANTITY	081-292 PCN I23H QUANTITY	TOTAL QUANTITY	UNIT
009E0010	Mobilization	<		-Lump Sum		>	Lump Sum	LS
380E5030	Nonreinforced PCC Pavement Repair	30.0	12.0	146.1	8.0	201.7	397.8	SqYd
380E5100	Continuously Reinforced PCC Pavement Repair	-	18.6	-	-	-	18.6	SqYd
380E6000	Dowel Bar	50	18	104	12	92	276	Each
380E6110	Insert Steel Bar in PCC Pavement	80	92	156	20	108	456	Each
634E0010	Flagging	5	5	5	5	25	45	Hour
634E0100	Traffic Control	156	156	156	156	734	1358	Unit
634E0120	Traffic Control, Miscellaneous	<		-Lump Sum		>	Lump Sum	LS
634E0310	Temporary Road Markers	780	2340	1980	360	6000	11460	Ft
634E0420	Type C Advance Warning Arrow Panel	<		2 each		>	2	Each
634E0610	4" Temporary Pavement Marking Tape Type 2	-	-	-	-	432	432	Ft
998E0100	Railroad Protective Insurance	-	-	-	-	LS	Lump Sum	LS

TABLE FOR PCC PAVEMENT REPAIR ON 050W-292 PCN 123D

INSERT STEEL BAR IN PCC PAVEMENT

			WB DR LAN			NEW JOINT	No. 8 x 18" DEFORMED	No. 5 x 24" DEFORMED	DOWEL
	MRM	LANE	L Ft	W Ft	PCCP SqYds	CON- FIG.	TIE BARS Each	TIE BARS Each	BAR Each
3	93.023	WBDL	6	6	4.0	R	8	2	6
3	93.389	WBDL	6	6	4.0	R	8	2	6
3	93.412	WBDL	6	6	4.0	R	8	2	6
3	93.427	WBDL	6	6	4.0	R	8	2	6
3	93.432	WBDL	6	6	4.0	R	8	2	6
TO	TALS:				20.0		40	10	30
AD	DITIONA	AL QUANT	TITIES:		10.0		20	10	20
GR	AND TO	TALS:			30.0		60	20	50

TABLE FOR CRC & PCC PAVEMENT REPAIR ON 029S-291 PCN 123E

INSERT STEEL BAR IN PCC PAVEMENT

		SB DR LAI			NEW JOINT	No. 9 x 18" DEFORMED	No. 5 x 24" DEFORMED	DOWEL
MRM	LANE	L Ft	W Ft	PCCP SqYds	CON- FIG.	TIE BARS Each	TIE BARS Each	BAR Each
0.562	SBDL	6	6	4.0	R	8	4	6
2.054	SBDL	6	6	4.0	R	8	4	6
3.198	SBDL	6	6	4.0	R	8	4	6
TOTALS:				12.0		24	12	18

INSERT STEEL BAR IN PCC PAVEMENT

		SB DR LAI			NEW JOINT	No. 6 DEFORMED	No. 4 x 24" DEFORMED	
MRM	LANE	L Ft	W Ft	CRC SqYds	CON- FIG.	TIE BARS Each	TIE BARS Each	
28.097	SBDL	6	14	9.3		24	4	
28.118	SBDL	6	14	9.3		24	4	
TOTALS:				18.6		48	8	

STATE OF	PROJECT	SHEET	TOTAL SHEETS
SOUTH DAKOTA	2011 YANKTON AREA CRCP & PCCP REPAIR	3	24

TABLE FOR PCC PAVEMENT REPAIR ON 029N-291 PCN 123F

INSERT STEEL BAR IN PCC PAVEMENT

		NB DRIVING LANE			NEW JOINT	No. 9 x 18" DEFORMED	No. 5 x 24" DEFORMED	DOWEL
MRM	LANE	L Ft	W Ft	PCCP SqYds	CON- FIG.	TIE BARS Each	TIE BARS Each	BAR Each
46.950	NBDL	6	8	5.3	R	10	4	6
46.970	NBDL	6	14	9.3	R	16	4	12
47.450	NB ONRAMP	25	20	55.6	R	16 *	20	20
47.550	NB ONRAMP	24	20	53.3	R	16 *	18	36
47.556	NBDL	6	6	4.0	R	8	4	6
47.635	NBDL	6	14	9.3	R	16	4	12
47.654	NBDL	6	14	9.3	R	16	4	12
TOTALS:				146.1		98	58	104

NOTE

* NB ONRAMP Bars shall be No. 8 x 18".

TABLE FOR PCC PAVEMENT REPAIR ON 081-292 PCN 123G

INSERT STEEL BAR IN PCC PAVEMENT

			SB DRIVING LANE		SB PASSING LANE		NEW JOINT	No. 9 x 18" DEFORMED	No. 5 x 24" DEFORMED	DOWEL
MRM	LANE	L Ft	W Ft	L Ft	W Ft	PCCP SqYds	CON- FIG.	TIE BARS Each	TIE BARS Each	BAR Each
1.158 1.158	NBDL NBPL	6	6	6	6	4.0 4.0	R R	8 8	2 2	6 6
TOTALS:						8.0		16	4	12

TABLE FOR PCC PAVEMENT REPAIR ON 081-292 PCN 123H

INSERT STEEL BAR IN PCC PAVEMENT

		SB DR LAI		NB DR LAI			NEW JOINT	No. 9 x 18" DEFORMED	No. 5 x 24" DEFORMED	DOWEL
MRM	LANE	L Ft	W Ft	L Ft	W Ft	PCCP SqYds	CON- FIG.	TIE BARS Each	TIE BARS Each	BAR Each
38.111	NBL			6	14	9.3	R	16		12
38.111	SBL	6	14			9.3	R	16		12
45.554	SBL	12	14			18.7	Т	16	8	
49.330	NBL			40	14	62.2	R	16		24
49.330	SBL	40	14			62.2	R	16		24
TOTALS:						161.7		80	8	72
ADDITIONA	AL QUAN	TITIES:				40.0		20	0	20
GRAND TO	TALS:		•			201.7		100	8	92

JOINT REPAIR CONFIGURATION KEY

W = Two Working Joints (Use only if repair is full roadway width and uniform length (across <u>all</u> lanes))

T = Two Tied Joints

B = One Working & One Tied Joint

R = Two Tied Joints with Original Joint Restored with Dowel Bar Assembly

SPECIFICATIONS

Standard Specifications for Roads and Bridges, 2004 Edition and Required Special Provisions, Supplemental Specifications and/or Special Provisions as included in the Proposal.

UTILITIES

The Contractor shall contact the involved utility companies through South Dakota One Call (1-800-781-7474) prior to starting work. It shall be the responsibility of the Contractor to coordinate work with the utility owners to avoid damage to existing facilities.

Utilities are not planned to be affected on this project. If utilities are identified near the improvement area through the SD One Call Process as required by South Dakota Codified Law 49-7A and Administrative Rule Article 20:25, the Contractor shall contact the Project Engineer to determine modifications that will be necessary to avoid utility impacts.

SCOPE OF WORK

This project consists of full depth replacement of concrete pavement in areas where concrete pavement blowups or major failures have occurred. Full depth areas vary in length and width, however the minimum length is 6 feet.

Project 029S-291 also consists of full depth replacement of Continuously Reinforced Concrete (CRC) Pavement in areas where major failures have occurred. Full depth areas may vary in length and width, however the minimum length is 6 feet.

RAILROAD CROSSING

The Contractor is to coordinate work with the Railroad Company regarding any work to be done adjacent to the railroad tracks on US HWY 81 and give 30 days notice to the Railroad Company in advance of required flagging dates. See Special Provision for Working on Railroad Company Right of Way.

HISTORICAL PRESERVATION OFFICE CLEARANCES

To obtain State Historical Preservation Office (SHPO) clearance, a cultural resources survey may need to be conducted by a qualified archaeologist. In lieu of a cultural resources survey, the Contractor could request a records search from Jim Donohue, State Archaeological Research Center (SARC). Provide SARC with the following: a topographical map or aerial view on which the site is clearly outlined, site dimensions, project number, and PCN. If applicable, provide evidence that the site has been previously disturbed by farming, mining, or construction activities with a landowner statement that no artifacts have been found on the site. The Contractor shall arrange and pay for the cultural resource survey and/or records search.

If any earth disturbing activities occur within the current geographical or historic boundaries of any South Dakota reservation, the Contractor shall obtain Tribal Historical Preservation Office (THPO) clearance. If no THPO exists, the required SHPO clearance shall suffice, with documentation of Tribal contact efforts provided to SHPO.

To facilitate SHPO or THPO responses, the Contractor should submit a records search or cultural resources survey report to the DOT Environmental Engineer, 700 East Broadway Avenue, Pierre, SD 57501-2586 (605-773-3268). Allow 30 days from the date this information is submitted to the Environmental Engineer for SHPO/THPO approval. The Contractor is responsible for obtaining all required permits and clearances for staging areas, borrow sites, waste disposal sites, and all material processing sites. The Contractor shall provide the required permits and clearances to the Engineer at the preconstruction meeting.

WASTE DISPOSAL SITE

The Contractor will be required to furnish a site(s) for the disposal of construction/demolition debris generated by this project.

Construction/demolition debris may not be disposed of within the State ROW.

The waste disposal site(s) shall be managed and reclaimed in accordance with the following from the General Permit for Highway, Road, and Railway Construction/Demolition Debris Disposal under the South Dakota Waste Management Program issued by the Department of Environment and Natural Resources.

The waste disposal site(s) shall not be located in a wetland, within 200 feet of surface water, or in an area that adversely affects wildlife, recreation, aesthetic value of an area, or any threatened or endangered species, as approved by the Engineer.

If the waste disposal site(s) is located such that it is within view of any ROW, the following additional requirements shall apply:

- 1. Construction/demolition debris consisting of concrete, asphalt concrete, or other similar materials shall be buried in a trench completely separate from wood debris. The final cover over the construction/demolition debris shall consist of a minimum of 1 foot of soil capable of supporting vegetation. Waste disposal sites provided outside of the State ROW shall be seeded in accordance with Natural Resources Conservation Service recommendations. The seeding recommendations may be obtained through the appropriate County NRCS Office. The Contractor shall control the access to waste disposal sites not within the State ROW through the use of fences, gates, and placement of a sign or signs at the entrance to the site stating No Dumping Allowed.
- Concrete and asphalt concrete debris may be stockpiled within view of the ROW for a period of time not to exceed the duration of the project. Prior to project completion, the waste shall be removed from view of the ROW or buried and the waste disposal site reclaimed as noted above.

The above requirements will not apply to waste disposal sites that are covered by an individual solid waste permit as specified in SDCL 34A-6-58, SDCL 34A-6-1.13, and ARSD 74:27:10:06.

Failure to comply with the requirements stated above may result in civil penalties in accordance with South Dakota Solid Waste Law, SDCL 34A-6-1.31.

Cost associated with furnishing waste disposal site(s), disposing of waste, maintaining control of access (fence, gates and signs), and reclamation of the waste disposal site(s) shall be incidental to the various contract items.

EXISTING PCC & CRC PAVEMENT

The existing pavement thicknesses are as follows:

- 8" PCCP on 050W-292 PCN I23D, 081-292 PCN I23H & the On Ramp on 029N-291 - PCN I23F.
- 9.5" PCCP on 081-292 PCN I23G between MRM 1.0 to 1.2.
- 10" CRCP on 029S-291 PCN I23E between MRM 28.0 to 28.2.
- 10.5" PCCP on 029N-291 PCN I23F on the mainline areas.
- 11.5" PCCP on 029S-291 PCN I23E between MRM 0.4 to 4.0.

The aggregate in the existing PCC & CRC Pavement is quartzite.

RESTORATION OF GRAVEL CUSHION

An inspection of the gravel cushion subgrade shall be made after removing concrete from each pavement replacement area. Areas of excess moisture shall be dried to the satisfaction of the Engineer. Loose material shall be removed. Each replacement area shall be leveled and compacted to the satisfaction of the Engineer.

STATE

PROJECT

2011 YANKTON AREA

CRCP & PCCP REPAIR

TOTAL SHEETS

24

SHEET

If additional gravel cushion material is required, the Contractor shall place and compact gravel cushion to the satisfaction of the Engineer at no additional cost to the State. Additional gravel cushion can be obtained from the Department of Transportation Maintenance shops located in Beresford, Junction City, Menno or Yankton.

Cost for this work shall be incidental to the contract unit prices per square yard for Nonreinforced PCC Pavement Repair and Continuously Reinforced PCC Pavement Repair.

NONREINFORCED PCC PAVEMENT REPAIR - GENERAL

Locations and size (length or width) of concrete repair areas are subject to change in the field, at the discretion of the Engineer, at no additional cost to the state. Payment will be based on actual area replaced.

Existing concrete pavement shall be sawed full depth at the beginning and end of the PCCP repair areas. When either the beginning or end of a PCCP repair area falls close to an existing joint or crack, the PCCP repair area shall be extended to eliminate the existing joint or crack. Where possible, new working joints shall be adjacent to existing working joints.

Saw cuts that extend beyond the repair area shall be minimized and filled with a non-shrinkage mortar mix at the Contractor's expense.

Existing concrete pavement in the replacement areas shall be removed by the lift out method or by means that minimize damage to the base and sides of remaining in place concrete. All removed material shall be removed from within the right-of-way by the end of the workday. Damage to adjacent concrete caused by the Contractor's operations shall be removed and replaced at the Contractor's expense.

If the pavement replacement area is entirely on either side of the existing contraction joint, the location of one of the working joints will be at the original location. Any existing dowel bar assemblies shall be sawed off or removed.

Concrete placed adjacent to gravel and asphalt concrete shoulders shall be formed full depth to match the width of existing concrete pavement. Asphalt concrete shoulders adjacent to concrete pavement replacements shall be repaired with new hot-mix asphalt concrete.

At repair locations where the new working joint is not opposite the existing working joint, the Contractor shall place a ¼" preformed asphalt expansion joint material along the longitudinal joint from the existing working joint to the new working joint. The expansion joint material shall meet the requirements of AASHTO M33. Cost for this material shall be incidental to the contract unit price per square yard for Nonreinforced PCC Pavement Repair.

All joints (longitudinal and transverse) through and around the repair areas will be sawed and sealed in accordance with the details shown in these plans. Refer to Saw and Seal Joints notes.

NONREINFORCED PCC PAVEMENT REPAIR

New pavement thickness shall be a 1" thicker than the existing pavement.

Concrete shall meet the requirements of the Standard Specifications Section 380, except as modified by the following notes:

The fine aggregate shall be screened over a one-inch square-opening screen just prior to introduction into the concrete paving mix if required by the Engineer.

The slump requirement will be limited to 3" maximum after water reducer is added and the concrete shall contain 4.5% to 7.0% entrained air. Coarse aggregate shall be crushed ledge rock, Size No. 1. The Contractor is responsible for the mix design used. The Contractor shall submit a mix design and supporting documentation for approval at least 2 weeks prior to use. In lieu of submitting a mix design the Contractor may use one of the following dependent upon type of cement to be used:

	LB./CU.YD.	LB./CU.YD.
CEMENT	800 (TYPE I or II)	710 (TYPE III)
WATER	282	300
FINE AGGREGATE	1039	1114
COARSE AGGREGATE	1726	1668

The use of a water reducer at manufacturer's recommended dosage will be required.

Concrete shall be cured with white pigmented curing compound (AASHTO M148, Type 2) applied as soon as practical at a rate of 125 square feet per gallon. Concrete shall be cured for a minimum of 48 hours before opening to traffic. The 48 hours is based upon a concrete surface temperature of 60° F or higher throughout the cure period. If the concrete temperature falls below 60° F, the cure time shall be extended or other measures shall be taken, at no additional cost to the State. In addition to the curing requirements, a strength of 4,000 psi must be attained prior to opening to traffic.

Concrete shall be covered with suitable insulation blanket consisting of a layer of closed cell polystyrene foam protected by at least one layer of plastic. Insulation blanket shall have an R-value of at least 0.5, as rated by the manufacturer. Insulation blanket shall be left in place, except for joint sawing operations, until the 4000 psi is attained. Insulation blanket shall be overlapped on to the existing concrete by 4'. The initial contraction joint sawing shall be performed as soon as practical after placement to avoid random cracking.

Cost for performing the aforementioned work including sawing and removing concrete, furnishing and placing concrete, sawing and sealing joints, repairing gravel and asphalt concrete shoulders, labor, tools and equipment shall be included in the contract unit price per square yard for Nonreinforced PCC Pavement Repair.

CONTINUOUSLY REINFORCED PCC PAVEMENT REPAIR

Locations and size (length or width) of pavement repair areas are subject to change in the field, at the discretion of the Engineer, at no additional cost to the state. Payment will be based on actual area replaced.

The Engineer will mark the location of the area to be repaired on construction. Where repair crosses both lanes, the passing lane should be repaired first.

The Contractor shall saw the in place concrete transversely at four locations for each repair area. Two saw cuts shall be full depth. The other two saw cuts shall be partial depth saw cuts and shall be made to a depth just above the in place reinforcing steel (3"+), and be placed outside of the previous full depth saw cuts. The outside cuts shall be a minimum of 6" from the nearest tight crack outside of the patch.

The Contractor shall lift out or break out the center section (including reinforcing steel) and then use light chipping hammers (not exceeding 15 pounds) to remove the remaining concrete at each end of the repair area, leaving the reinforcing steel in place. Care shall be taken not to cut, bend or otherwise damage the in place reinforcing steel. Damage to in place reinforcing steel or to in place concrete beyond the repair area will be replaced at the Contractor's expense, to the satisfaction of the Engineer.

The Contractor shall remove and dispose of the in place concrete and in place asphalt concrete.

Existing exposed reinforcing steel and concrete faces shall be cleaned by sandblasting and compressed air to remove dirt and debris prior to placement of concrete.

Place reinforcing steel according to the notes for Reinforcing Steel and Steel Bar Insertion.

Concrete placed adjacent to asphalt concrete shoulders shall be formed full depth to match the width of existing concrete pavement. The excavated area of the asphalt concrete shoulder adjacent to repair areas shall be filled with asphalt concrete.

Concrete shall not be placed in the repair areas before 12:00 pm and should be placed in the late afternoon. Temperature of the concrete at the time of placement shall be between 50°F and 90°F. The temperature of the concrete shall be maintained above 40°F during the curing period.

Saw cuts that extend beyond the repair area shall be minimized and filled with a non-shrinkage mortar mix at the Contractor's expense.

Upon placement of the concrete, repair areas shall be straight edged to ensure a smooth riding surface and shall be textured longitudinally with the pavement by finishing with a stiff broom. Repair areas shall then be checked with a 10' foot straight edge. The permissible longitudinal and transverse surface deviation shall be 1/8" in 10'.

New pavement thickness shall be equal to existing pavement thickness.

Concrete shall meet the requirements of the Standard Specifications Section 380, except as modified by the following notes:

The fine aggregate shall be screened over a one-inch square-opening screen just prior to introduction into the concrete paving mix if required by the Engineer.

CONTINUOUSLY REINFORCED PCC PAVEMENT REPAIR (CONTINUED)

PROJECT

2011 YANKTON AREA

TOTAL SHEETS

SHEET

The slump requirement will be limited to 3" maximum after water reducer is added and the concrete shall contain 4.5% to 7.0% entrained air. Coarse aggregate shall be crushed ledge rock,

STATE

entrained air. Coarse aggregate shall be crushed ledge rock, Size No. 1. The Contractor is responsible for the mix design used. The Contractor shall submit a mix design and supporting documentation for approval at least 2 weeks prior to use. In lieu of submitting a mix design the Contractor may use one of the following dependent upon type of cement to be used:

	LB./CU.YD.	LB./CU.YD.
CEMENT	800 (TYPE I or II)	710 (TYPE III)
WATER	282	300
FINE AGGREGATE	1039	1114
COARSE AGGREGATE	1726	1668

The use of a water reducer at manufacturer's recommended dosage will be required.

Concrete shall be cured with white pigmented curing compound (AASHTO M148, Type 2) applied as soon as practical at a rate of 125 square feet per gallon. Concrete shall be cured a minimum of 48 hours before opening to traffic. The 48 hours is based upon a concrete surface temperature of 60° F or higher throughout the cure period. If the concrete temperature falls below 60° F, the cure time shall be extended or other measures shall be taken, at no additional cost to the State. In addition to the curing requirements, a strength of 4,000 psi must be obtained prior to opening to traffic.

Concrete shall be covered with suitable insulation blanket consisting of a layer of closed cell polystyrene foam protected by at least one layer of plastic. Insulation blanket shall have an R-value of at least 0.5, as rated by the manufacturer. Insulation blanket shall be left in place, except for joint sawing operations. Insulation blanket shall be overlapped on to the existing concrete by 4'.

Cost for performing the aforementioned work including sawing, chipping and removing concrete, sandblasting, cleaning, furnishing and placing concrete and reinforcing steel, finishing and curing, replacing gravel and asphalt concrete shoulders, labor and equipment shall be included in the contract unit price per square yard for Continuously Reinforced PCC Pavement Repair.

REINFORCING STEEL

After removal of the in place concrete and repair of the gravel cushion subgrade, new reinforcing steel shall be installed. Refer to the CRC Pavement Repair Area layouts for details.

- 1. New No. 6 longitudinal bars shall be lap spliced with the preserved in place longitudinal bars.
- 2. At full lane width repair areas, additional No. 6 longitudinal bars shall be centered between every other set of two spliced longitudinal bars throughout the width of the repair area. The additional longitudinal bars shall overlap into the existing concrete 9" on both sides of the repair area. Drilled holes will be required and the additional longitudinal bars shall be inserted in accordance with the notes for STEEL BAR INSERTION. The additional longitudinal bars shall then be lap spliced.
- 3. Additional No. 4 transverse bars shall be centered between the in place transverse bars throughout the length of the repair area. The spacing of transverse bars in the completed repair area should be half the spacing of the in place transverse reinforcing steel (New spacing will be 1.5' or 2').
 - For less than full lane width repair areas and repair areas adjacent to tied longitudinal joints, the additional transverse bars shall overlap into the existing concrete 9". Drilled holes will be required and the additional transverse bars shall be inserted according to the notes for STEEL BAR INSERTION.
 - For full roadway width repair areas, a keyway with factory bent No. 4 lap spliced transverse bars shall be constructed in the longitudinal joint to tie to the transverse bars that will be placed in the adjacent lane.

Cost for this work, including reinforcing steel, ties, labor and equipment shall be incidental to the contract unit price per square yard for Continuously Reinforced PCC Pavement Repair.

STEEL BAR INSERTION - CONTINUOUSLY REINFORCED CONCRETE PAVEMENT (CRCP)

Locations and quantities of concrete repair are subject to change in the field at the discretion of the Engineer. The Contractor will be responsible for ordering the actual quantity of steel bars necessary to complete the work.

Steel bars (No. 6 longitudinal deformed tie bars) shall be inserted 9 inches into the in place concrete at the transverse joint and centered between every other set of two spliced longitudinal bars throughout the width of the repair area. Steel bars (No. 4 transverse deformed tie bars) shall be inserted 9 inches into the in place concrete at the longitudinal joint throughout the length of the repair area. Refer to the notes for REINFORCING STEEL. An epoxy resin adhesive must be used to anchor the steel bar in the drilled hole.

Epoxy resin adhesive shall be of the type intended for horizontal applications, and shall conform to the requirements of ASTM C 881, Type IV, Grade 3 (equivalent to AASHTO M235, Type IV, Grade 3).

The diameter of the drilled holes in the existing concrete pavement for the steel bars shall not be less than 1/8 inch nor more than 3/8 inch greater than the overall diameter of the steel bar. Holes drilled into the existing concrete pavement shall be located at mid-depth of the slab and true and normal (Exception: In the transverse joints, the drilled in longitudinal steel bar angle will be slightly under 90° to allow for centering of the lap splice between existing longitudinal steel).

STEEL BAR INSERTION - CONTINUOUSLY REINFORCED CONCRETE PAVEMENT (CRCP) (CONTINUED)

The drilled holes shall be blown out with compressed air using a device that will reach to the back of the hole to ensure that all debris or loose material has been removed prior to epoxy injection.

A rigid frame or mechanical device will be required to guide the drill to ensure proper horizontal and vertical alignment of the steel bars in the drilled holes.

Mix the epoxy resin as recommended by the manufacturer and apply by an injection method approved by the Engineer. If an epoxy pump is utilized, it shall be capable of metering the components at the manufacturer's designated rate and be equipped with an automatic shut-off. The pump shall shut off when any of the components are not being metered at the designated rate.

Fill the drilled holes 1/3 to 1/2 full of epoxy, or as recommended by the manufacturer, prior to steel bar insertion. Care shall be taken to prevent epoxy from running out of the horizontal holes prior to steel bar insertion. Rotate the steel bar during insertion to eliminate voids and ensure complete bonding of the bar. Insertion of the bars by the dipping method will not be allowed.

Cost for steel bars shall be incidental to the contract unit price per square yard for Continuously Reinforced PCC Pavement Repair.

Cost for the epoxy resin adhesive, drilling of holes, applying the adhesive, inserting the steel bars into the drilled holes and all other items incidental to the insertion of the steel bars shall be incidental to the contract unit price per each for Insert Steel Bar in PCC Pavement.

SAW AND SEAL LONGITUDINAL JOINTS - CRCP

Longitudinal joints (in line with existing longitudinal joints) at concrete repair areas shall be sawed and sealed.

Joints shall not be sealed unless they are thoroughly clean and dry. Cleaning shall be accomplished by sand blasting and other tools as necessary. Just prior to sealing, each joint shall be blown out using a jet of compressed air to remove all traces of dust.

Longitudinal joints shall be sealed with Low Modulus Silicone Sealant or Hot Poured Elastic Joint Sealer.

Acceptance of the Low Modulus Silicone Sealant and Hot Poured Elastic Joint Sealer will be based on visual inspection by the Engineer.

Cost for sawing and sealing of the longitudinal construction joint shall be incidental to the contract unit price per square yard for Continuously Reinforced PCC Pavement Repair.

STEEL BAR INSERTION - NONREINFORCED PCC PAVEMENT

Locations and quantities of concrete repair are subject to change in the field at the discretion of the Engineer. The Contractor will be responsible for ordering the actual quantity of steel bars necessary to complete the work.

On 8" concrete repair areas:

The Contractor shall insert the steel bars (1" x 18" epoxy coated plain round dowel bars and No. 8 x 18" epoxy coated deformed tie bars for transverse joints and No. 5 x 24" epoxy coated deformed tie bars for longitudinal joints) into drilled holes in the existing concrete pavement. An epoxy resin adhesive must be used to anchor the steel bar in the drilled hole.

STEEL BAR INSERTION – NONREINFORCED PCC PAVEMENT (CONTINUED)

On 8.5" to 11.5" concrete repair areas:

The Contractor shall insert the steel bars (1½" x 18" epoxy coated plain round dowel bars and No. 9 x 18" epoxy coated deformed tie bars for transverse joints and No. 5 x 24" epoxy coated deformed tie bars for longitudinal joints) into drilled holes in the existing concrete pavement. An epoxy resin adhesive must be used to anchor the steel bar in the drilled hole.

Plain round dowel bars shall be cut to the specified length by sawing and shall be free from burring or other deformations. Shearing will not be permitted.

Epoxy resin adhesive shall be of the type intended for horizontal applications, and shall conform to the requirements of ASTM C 881, Type IV, Grade 3 (equivalent to AASHTO M235, Type IV, Grade 3).

Steel bars shall be inserted in the transverse joint on 18" centers. The first steel bar in the transverse joint shall be placed 9" from the edge of the slab closest to centerline. Steel bars shall be inserted in the longitudinal joint on 30" centers and shall be a minimum of 15" from either transverse joint. A typical one-lane patch 12' wide and 6' long will require 18 steel bars (8 in each transverse joint and 2 in the longitudinal joint).

The diameter of the drilled holes in the existing concrete pavement for the steel bars shall not be less than 1/8 inch nor more than 3/8 inch greater than the overall diameter of the steel bar. Holes drilled into the existing concrete pavement shall be located at mid-depth of the slab and true and normal. The drilled holes shall be blown out with compressed air using a device that will reach to the back of the hole to ensure that all debris or loose material has been removed prior to epoxy injection.

A rigid frame or mechanical device will be required to guide the drill to ensure proper horizontal and vertical alignment of the steel bars in the drilled holes.

Mix the epoxy resin as recommended by the manufacturer and apply by an injection method approved by the Engineer. If an epoxy pump is utilized, it shall be capable of metering the components at the manufacturer's designated rate and be equipped with an automatic shut-off. The pump shall shut off when any of the components are not being metered at the designated rate.

Fill the drilled holes 1/3 to 1/2 full of epoxy, or as recommended by the manufacturer, prior to insertion of the steel bar. Care shall be taken to prevent epoxy from running out of the horizontal holes prior to steel bar insertion. Rotate the steel bar during insertion to eliminate voids and ensure complete bonding of the bar. Insertion by the dipping method will not be allowed.

Cost for the epoxy resin adhesive, steel bars, drilling of holes, inserting the steel bars into the drilled holes and all other items incidental to the insertion of the steel bars shall be included in the contract unit price per each for Insert Steel Bar In PCC Pavement.

STATE OF	PROJECT	SHEET	TOTAL SHEETS
SOUTH DAKOTA	2011 YANKTON AREA CRCP & PCCP REPAIR	7	24

SAW AND SEAL JOINTS - NONREINFORCED PCC PAVEMENT

All longitudinal and transverse joints at concrete repair areas shall be sawed and sealed.

Joints shall not be sealed unless they are thoroughly clean and dry. Cleaning shall be accomplished by sand blasting and other tools as necessary. Just prior to sealing, each joint shall be blown out using a jet of compressed air to remove all traces of dust.

Longitudinal and transverse joints in urban sections shall be sealed with Hot Poured Elastic Joint Sealer. Transverse joints in rural sections shall be sealed with Low Modulus Silicone Sealant. Longitudinal joints in rural sections may be sealed with either Hot Poured Elastic Joint Sealer or Low Modulus Silicone Sealant.

Acceptance of the Low Modulus Silicone Sealant and Hot Poured Elastic Joint Sealer will be based on visual inspection by the Engineer.

Cost for sawing and sealing of the longitudinal construction joint and both transverse joints shall be incidental to the contract unit price per square yard for Nonreinforced PCC Pavement Repair.

TEMPORARY PAVEMENT MARKING

Temporary pavement marking on lane closure tapers shall consist of Temporary Road Markers. (Two workspaces with a 180' taper on 081-292 PCN I23G, one workspace with a 780' taper on 050W-292 PCN I23D, three workspaces with 780' tapers on 029S-291 PCN I23E, one workspace with a 780' taper on 029 N-291 PCN I23F, two workspaces with 600' tapers on 029N-291 PCN I23F). Three workspaces require 1000' double yellow on 081-292 PCN I23H for no passing zone marking.

Cost shall be included in the contract unit price per foot for Temporary Road Markers.

Temporary pavement marking for stop bars shall consist of 4" Temporary Pavement Marking Tape Type 2. Placement of each 24" white stop bar shall be accomplished by placing six pieces of 4" x 12' tape adjacent to one another. Each workspace requires two stop bars which is an equivalent of approximately 144' of 4" tape (three workspaces at 144' =432').

Cost shall be included in the contract unit price per foot for 4" Temporary Pavement Marking Tape Type 2.

GENERAL MAINTENANCE OF TRAFFIC

Removing, relocating, covering, salvaging and resetting of permanent traffic control devices, including delineation, shall be the responsibility of the Contractor. Cost for this work shall be incidental to the contract unit prices for the various items unless otherwise specified in the plans. Any delineators and signs damaged or lost shall be replaced by the Contractor at no cost to the State.

Storage of vehicles and equipment shall be outside the clear zone and as near as possible to the right-of-way line. Contractor's employees should mobilize at a location off the right-of-way and arrive at the work sites in a minimum number of vehicles necessary to perform the work.

Indiscriminate driving and parking of vehicles within the right-of-way will not be permitted. Any damage to the vegetation, surfacing, embankment, delineators and existing signs resulting from such indiscriminate use shall be repaired and/or restored by the Contractor, at no expense to the State, and to the satisfaction of the Engineer.

The Contractor shall provide documentation that all breakaway sign supports comply with FHWA NCHRP 350 or MASH crash-worthy requirements. The Contractor shall provide installation details at the preconstruction meeting for all breakaway sign support assemblies.

Sufficient traffic control devices have been included in these plans to sign two workspaces on a two-lane highway and two workspaces on a four-lane highway. If the Contractor elects to work on additional sites simultaneously, the cost for additional traffic control devices shall be incidental to the contract unit price per unit for Traffic Control.

MAINTENANCE OF TRAFFIC - PCC PAVEMENT REPAIR

A Type III Barricade shall be installed at the end of a lane closure taper as detailed in these plans. Additional Type III Barricades shall be installed facing traffic within the closed lane at a spacing of 1/4 mile.

Each mainline concrete repair location from which the in place concrete has been removed shall be marked with a minimum of two reflectorized cones (42" minimum height) or two reflectorized drums. In areas containing numerous concrete repair locations, two reflectorized drums should be installed at a spacing of 660' alternating with the Type III Barricades.

Signs may be mounted on portable supports.

Drivers in one lane two-way traffic workspaces must be able to see approaching traffic through and beyond the workspaces.

Construction workspaces on divided roadways shall be limited to 3 miles in length. Construction workspaces on undivided roadways shall be limited to 300 feet in length. The distance between the closest points of any two construction workspaces, including channeling devices, shall not be less than 3 miles. Drivers in two-way traffic workspaces must be able to see approaching traffic through and beyond the work zone.

Construction workspaces in urban areas shall be limited to 3 blocks in length. The minimum distance between workspaces shall be 3 blocks.

When work is in progress within an intersection, Flaggers will be required to direct traffic.

Holes adjacent to centerline in the lane open to traffic created during removal and replacement of PCC Pavement Repair areas shall be filled with gravel and cold-mix asphalt concrete prior to opening the lane to traffic. Gravel and cold-mix asphalt concrete can be obtained from the Department of Transportation Maintenance shops located at Tyndall, Yankton or Junction City.

MAINTENANCE OF TRAFFIC - PCC PAVEMENT REPAIR (CONTINUED)

Holes in the gravel and asphalt concrete shoulders created during removal and replacement of PCC Pavement Repair areas shall be filled with gravel and hot-mix asphalt concrete (to match the shoulder surfacing) prior to opening the lane to traffic. Gravel can be obtained from the Department of Transportation Maintenance shops located at Tyndall, Yankton or Junction City. Hot-mix asphalt concrete shall be furnished by the Contractor.

Cost for furnishing, hauling and placing gravel and asphalt concrete shall be incidental to the contract unit price per square yard for Nonreinforced PCC Pavement Repair and Continuously Reinforced PCC Pavement Repair.

Routing traffic onto the shoulders during any phase of the construction will not be allowed.

Damage to the shoulders, median or ditch due to the Contractor's operations shall be repaired by the Contractor, to the satisfaction of the Engineer, at no expense to the State. This includes the routing of traffic onto these shoulders around the work zones.

MAINTENANCE OF TRAFFIC (INTERSTATE HIGHWAYS)

Lane closures shall be limited to 3 miles in length. The distance between the closest points of any two-lane closures, excluding taper, shall not be less than 3 miles.

Work activities shall not be conducted simultaneously on the median and outside shoulders of the same directional set of lanes.

The use of interstate maintenance crossovers will not be permitted.

Traffic will be permitted on the ramp shoulders when necessary to allow traffic around a workspace.

STATE OF	PROJECT	SHEET	TOTAL SHEETS
SOUTH DAKOTA	2011 YANKTON AREA CRCP & PCCP REPAIR	8	24

ITEMIZED LIST FOR TRAFFIC CONTROL

SIGN CODE	SIGN SIZE	DESCRIPTION	NUMBER REQUIRED	UNITS PER SIGN	UNITS
E5-1	36" x 32"	EXIT GORE SIGN		24	
G20-2	36" x 18"	END ROAD WORK	8	17	136
R1-1	48" x 48"	STOP	4	34	136
R1-2	48" x 48"	YIELD		34	
R2-1	30" x 36"	SPEED LIMIT		23	
R2-6	24" x 24"	FINES DOUBLE		16	
R4-7	24" x 30"	KEEP RIGHT (SYMBOL)		18	
R5-1	48" x 48"	DO NOT ENTER		34	
R5-1a	48" x 36"	WRONG WAY		29	
R10-6	24" x 36"	STOP HERE ON RED		20	
R11-2	48" x 30"	ROAD CLOSED		27	
R11-3a	60" x 30"	ROAD CLOSED MILES AHEAD LOCAL TRAFFIC ONLY		30	
R11-4	60" x 30"	ROAD CLOSED TO THRU TRAFFIC		30	
SW12-1b	120" x 60"	HIGHWAY WORKERS GIVE'EM A BRAKE		80	
W1-1	48" x 48"	LEFT OR RIGHT TURN ARROW		34	
W1-2	48" x 48"	LEFT OR RIGHT CURVE ARROW		34	
W1-3	48" x 48"	REVERSE TURN SIGN (LEFT OR RIGHT)	2	34	68
W1-4	48" x 48"	REVERSE CURVE SIGN (LEFT OR RIGHT)		34	
W3-1	48" x 48"	STOP AHEAD (SYMBOL)	4	34	136
W3-2	48" x 48"	YIELD AHEAD (SYMBOL)		34	
W3-3	48" x 48"	SIGNAL AHEAD (SYMBOL)		34	
W3-4	48" x 48"	BE PREPARED TO STOP		34	
W3-5	48" x 48"	SPEED REDUCTION (MPH)		34	
W4-1	48" x 48"	MERGE (SYMBOL)		34	
W4-2	48" x 48"	LEFT OR RIGHT LANE ENDS (SYMBOL)	2	34	68
W5-2	48" x 48"	NARROW BRIDGE		34	
W5-4	48" x 48"	RAMP NARROWS	1	34	34
W7-3a	30" x 24"	NEXT MILES		18	
W8-1	36" x 36"	BUMP		27	
W8-6	48" x 48"	TRUCK CROSSING		34	
W8-7	36" x 36"	LOOSE GRAVEL		27	
W8-9a	48" x 48"	SHOULDER DROP-OFF		34	
W8-11	48" x 48"	UNEVEN LANES		34	
W13-1	24" x 24"	ADVISORY SPEED PLATE	3	16	48
W13-4	24" x 24"	ON RAMP	1	16	16
W20-1	48" x 48"	ROAD WORK AHEAD	6	34	204
W20-2	48" x 48"	DETOUR AHEAD		34	
W20-3	48" x 48"	ROAD CLOSED AHEAD		34	
W20-4	48" x 48"	ONE LANE ROAD AHEAD	4	34	136
W20-5	48" x 48"	LT. OR RT. LANE CLOSED AHEAD	2	34	68
W20-7a	48" x 48"	FLAGGER	2	34	68
W21-1a	48" x 48"	WORKERS (SYMBOL)		34	
W21-2	36" x 36"	FRESH OIL		27	
W21-3	48" x 48"	ROAD MACHINERY AHEAD		34	
W21-5	48" x 48"	SHOULDER WORK		34	
W21-5a	48" x 48"	RIGHT SHOULDER CLOSED		34	
W21-5b	48" x 48"	RIGHT SHOULDER CLOSED AHEAD		34	
****	12" x 36"	TYPE III OBJECT MARKER		15	
	****	TYPE III BARRICADE - 8 FT. SINGLE SIDED	6	40	240
****		I I F E III DANNODE - O I I. SINGLE SIDED	Ö	40	

TOTAL UNITS 1358

	Posted	Spacing of	Spacing of
	Speed	Advance Warning	Channelizing
ΙP	rior to	Signs	Devices
	Work	(Feet)	(Feet)
	(M.P.H.)	(A)	(G)
	0 - 30	200	25
Г	35 - 40	350	25
	45 - 50	500	50
	55	750	50
[6	60 - 65	1000	50

■ Flagger

■ Channelizing Device

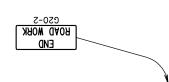
For low-volume traffic situations with short work zones on straight roadways where the flagger is visible to road users approaching from both directions, a single flagger may be used.

The ROAD WORK AHEAD and the END ROAD WORK signs may be omitted for short duration operations (I hour or less).

For tack and/or flush seal operations, when flaggers are not being used, the FRESH OIL sign (W21-2) shall be displayed in advance of the liquid asphalt

Flashing warning lights and/or flags may be used to call attention to the advance warning signs.

The channelizing devices shall be drums or type II barricades if traffic control must remain overnight or longer. During daylight hours, 42" cones may be used in lieu of drums or type II barricades along the centerline.



Channelizing devices are not required along the centerline adjacent to work area when pilot cars are utilized for escorting traffic through the work area.

Channelizing devices and flaggers shall be used at intersecting roads to control intersecting road traffic as required.

The buffer space shall be a sufficient length so that the channelizing devices are visible to approaching traffic.

DDO

Warning sign sequencein opposite direction same as below. Sort One Tr XXX FEET (Optional) ROAD AHEAD ROAD WORK June 26, 2006

PLATE NUMBER 634.23

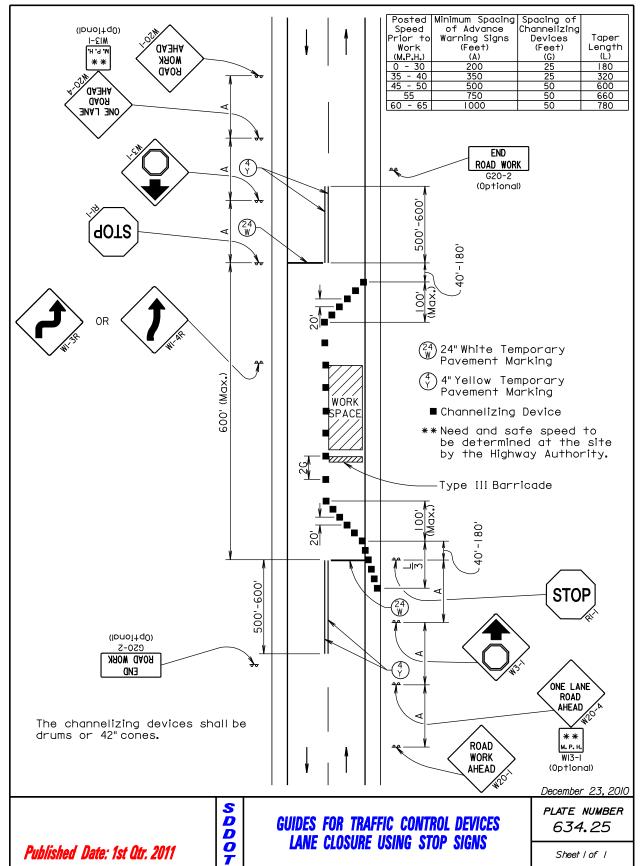
Published Date: 1st Qtr. 2011

GUIDES FOR TRAFFIC CONTROL DEVICES LANE CLOSURE WITH FLAGGER PROVIDED

Sheet I of I

PROJECT SHEET TOTAL SHEETS STATE OF 2011 YANKTON AREA 24 DAKOTA 9 CRCP & PCCP REPAIR

Plotting Date: 21-MAR-2011

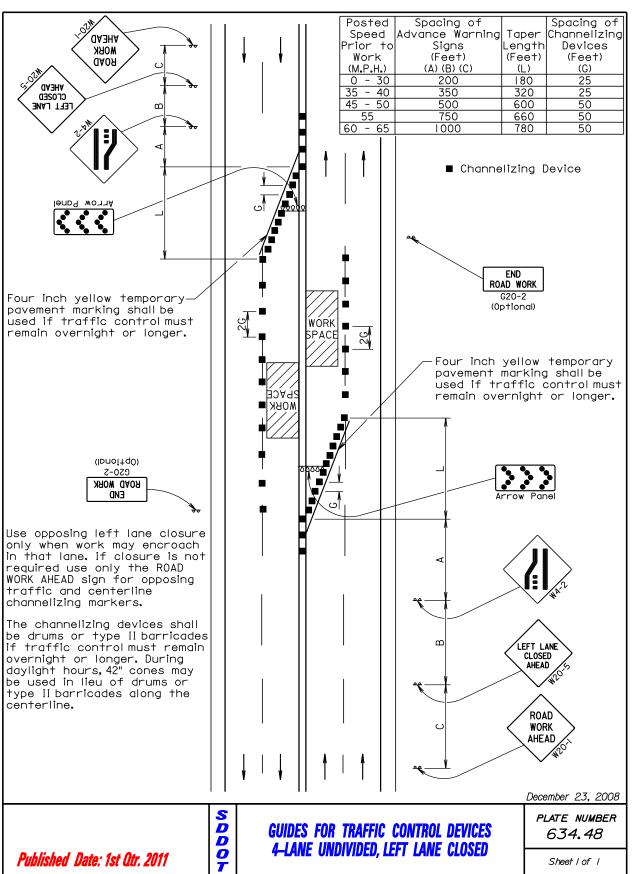


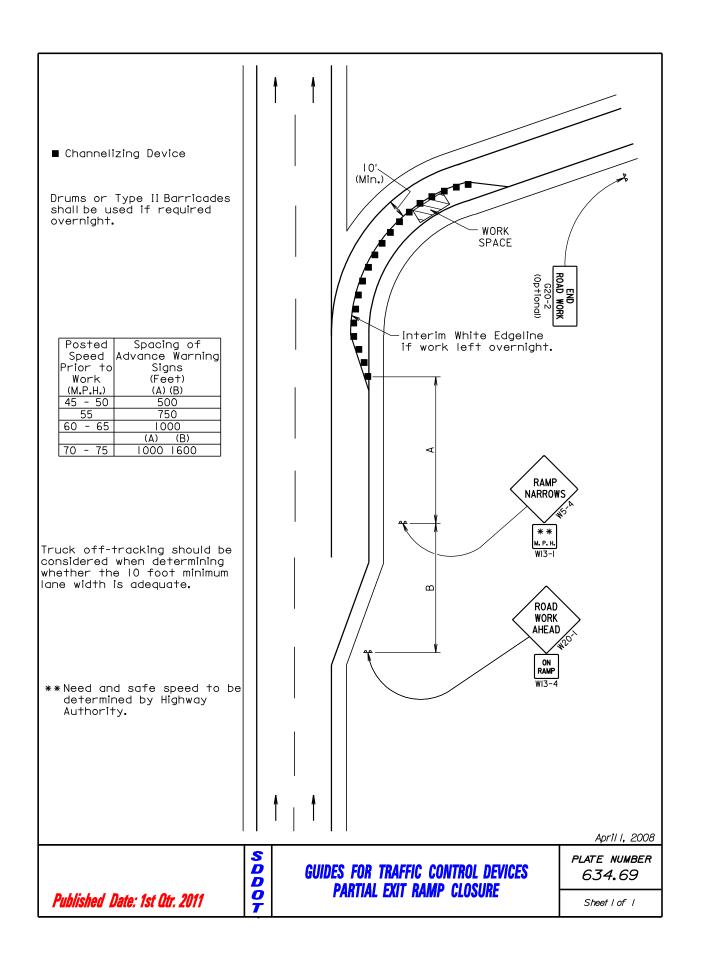
Posted Spacing of Taper Speed Advance Warning Taper Length Work (Feet) (Feet) (Feet) (Feet) (Feet) (I) (I)	n Device	ting s	 			END ROAD WORK G20-2 (Optional)
■ Channelizing Device Drums or Type II Barricades shall be used if required overnight. 42" cones may be used along centerline Longitudinal dimensions may be to fit project conditions such horizontal curves, vertical curv other site restrictions.	as			WORK SPACE		
Four inch white temporary pav marking shall be used if traffi must remain overnight or longe	c control				00000 00000	Arrow Panel
						RIGHT LANE CLOSED AHEAD WORK AHEAD
Published Date: 1st Qtr. 2011				FFIC CONTROL D, RIGHT LAN		April I, 2008 PLATE NUMBER 634.47 Sheet I of I

STATE OF PROJECT SHEET TOTAL SHEETS

SOUTH 2011 YANKTON AREA
DAKOTA CRCP & PCCP REPAIR 10 24

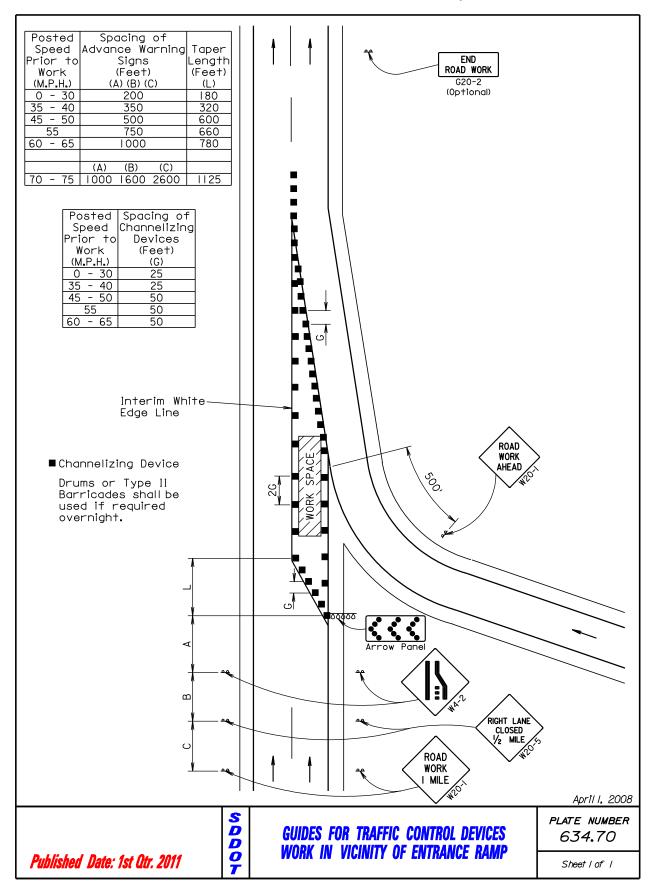
Plotting Date: 21-MAR-2011





٦	STATE OF	PROJECT	SHEET	TOTAL SHEETS
١	SOUTH	2011 YANKTON AREA		SHEE 13
١	DAKOTA	CRCP & PCCP REPAIR	11	24

Plotting Date: 21-MAR-2011



-Anchor Post or Slip Base

120" Diameter

clearance checks)

(Perimeter of stub height



The top of anchor posts and slip bases SHALL NOT extend above a 60" chord line within a 120" diameter circle around the post with ends 4" above the ground.

ELEVATION VIEW

PLAN VIEW

(Examples of stub height clearance checks)

At locations where there is curb and gutter adjacent to the breakaway sign support, the stub height shall be a maximum of 4" above the ground line at the localized area adjacent to the breakaway support stub.

The 4" stub height clearance is not necessary for U-channel lap splices where the support is designed to yield (bend) at the base.

July I, 2005 PLATE NUMBER *634.*99

Chord Line

GENERAL NOTES:

Examples of

60" Chord Line Clearance Checks

S D D O T

Breakaway Support Stub Clearance

Ground Line-

Sheet I of I

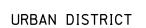
ublished Date: 1st Qtr. 2011

Top of Anchor Post or Slip Base-

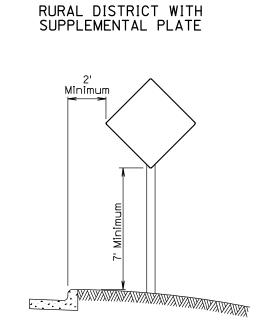
√ Walkway

RURAL DISTRICT

6' to 12'



Published Date: 1st Qtr. 2011



Paved Shoulder

6' Minimum

URBAN DISTRICT

DDO

Sheet I of I

December 23, 2003

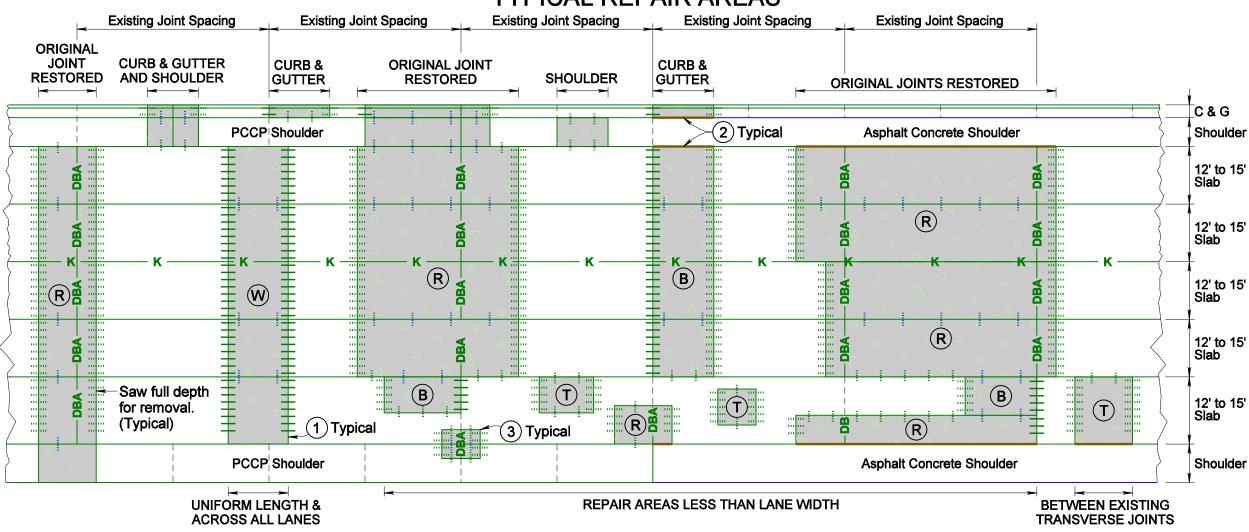
PLATE NUMBER

634.85

NONREINFORCED PCC PAVEMENT REPAIR

STATE OF SOUTH DAKOTA CRCP & PCCP REPAIR 13 24

UP TO FOUR LANE ROADWAY WITH CENTER TURN LANE OR UP TO TEN LANE DIVIDED ROADWAY TYPICAL REPAIR AREAS



KEY:



PCC Pavement Repair Area

PCC PAVEMENT REPAIR AREA TYPES:

- W Two Working Joints (Use only if repair is full roadway width and uniform length (across all lanes))
- Two Tied Joints
- (B) One Working & One Tied Joint
- R Two Tied Joints with Original Joint Restored with Dowel Bar Assembly

Longitudinal Keyway Joints Without Bars

 K — Where a repair area intersects an existing longitudinal keyway joint without tie bars, the newly constructed joint should also be a keyway without tie bars.

Steel Bars for Transverse Joints

Pavement Thickness >= 8.5"

- _ Drilled in 1¼" x 18" epoxy coated deformed tie bars spaced 18" center to center.
- Drilled in No. 9 x 18" epoxy coated deformed tie bars spaced 18" center to center.

Pavement Thickness < 8.5"

- Drilled in 1" x 18" epoxy coated deformed tie bars spaced 18" center to center.
- Drilled in No. 8 x 18" epoxy coated deformed tie bars spaced 18" center to center.
- Dowel Bar Assembly

Steel Bars for Longitudinal Joints

- No. 5 x 30" epoxy coated deformed tie bars. Sawed Joint - spaced 48" center to center. Construction Joint - spaced 48" center to center.
- No. 5 x 24" epoxy coated deformed tie bars. Drilled In spaced 30" center to center.

NOTES: Saw around repair areas full depth for removal.

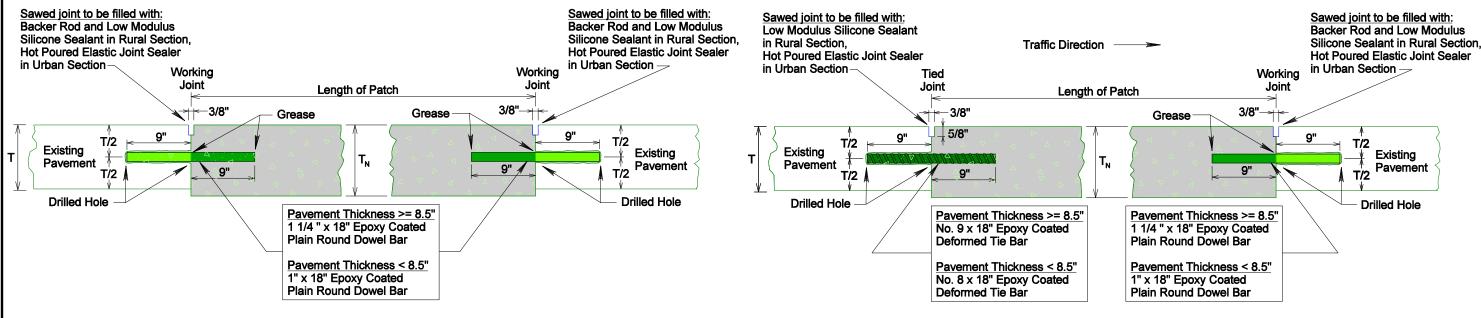
- (1) Where possible, transverse joints shall be constructed/maintained full roadway width.
- 2 Edges of repair areas shall be formed to match the width of the existing concrete pavement.
- (3) Need for bars in small repair areas on/near the shoulder to be determined on a case-by-case basis, on construction by the Engineer.

TOTAL SHEETS **PROJECT** SHEET 2011 YANKTON AREA 14 24 **CRCP & PCCP REPAIR**

NONREINFORCED PCC PAVEMENT REPAIR

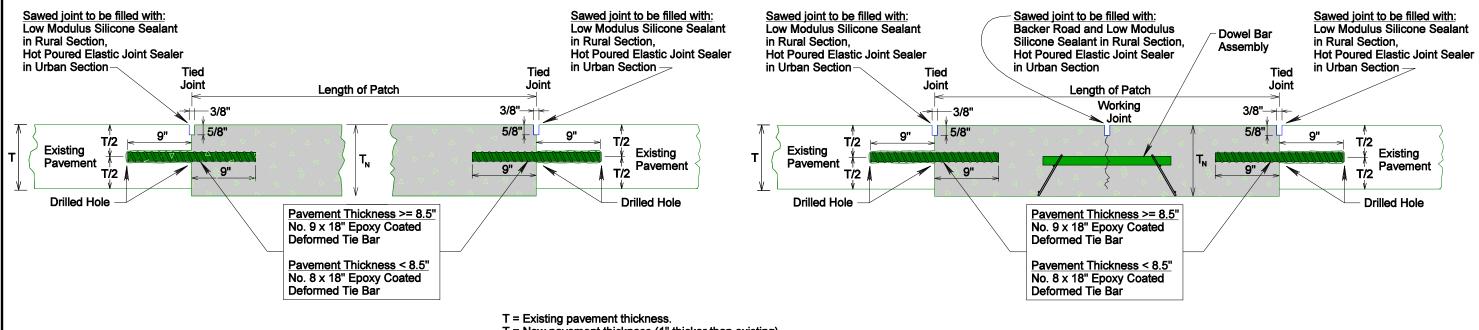
PLAIN ROUND DOWEL BAR INSERTION TYPE W - (TWO WORKING JOINTS)

DEFORMED TIE BAR AND PLAIN ROUND DOWEL BAR INSERTION TYPE B - (ONE TIED JOINT AND ONE WORKING JOINT)



DEFORMED TIE BAR INSERTION TYPE T - (TWO TIED JOINTS)

DEFORMED TIE BAR INSERTION WITH DOWEL BAR ASSEMBLY TYPE R - (TWO TIED JOINTS AND ONE WORKING JOINT - ORIGINAL JOINT RESTORED)



T_N= New pavement thickness (1" thicker than existing).

Bar embedded to a minimum depth of 9 inches into the existing pavement by utilizing an epoxy resin adhesive.

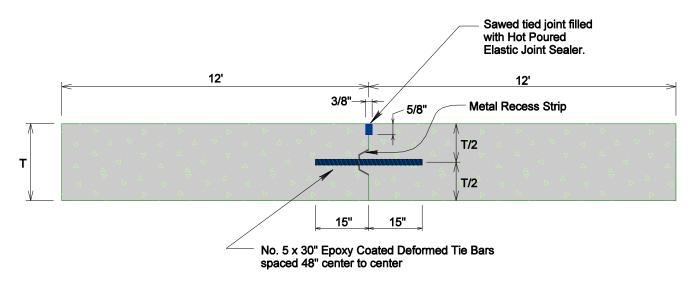
Cost for furnishing and inserting steel bars (deformed tie and plain round dowel) shall be included in the contract unit price per each for Insert Steel Bar in PCC Pavement.

Cost for furnishing and installing dowel bar assembly shall be included in the contract unit price per each for Dowel Bar.

NONREINFORCED PCC PAVEMENT REPAIR

STATE OF	PROJECT	SHEET	TOTAL SHEETS
SOUTH DAKOTA	2011 YANKTON AREA CRCP & PCCP REPAIR	15	24

LONGITUDINAL CONSTRUCTION JOINT WITH TIE BARS & KEYWAY

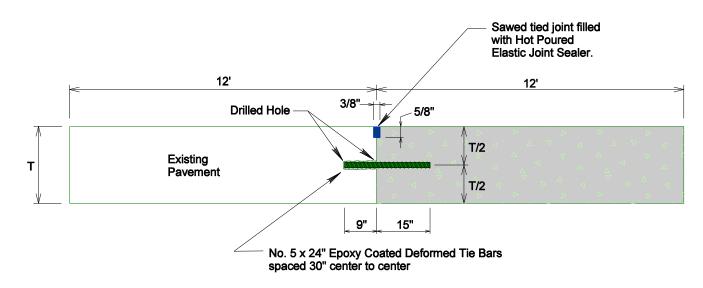


T = New pavement thickness.

Deformed tie bars will only be inserted on centerline when there is full width pavement removal.

Cost for furnishing and inserting centerline tie bars shall be incidental to the contract unit price per square yard for Nonreinforced PCC Pavement Repair.

LONGITUDINAL CONSTRUCTION JOINT WITH DRILLED IN TIE BARS



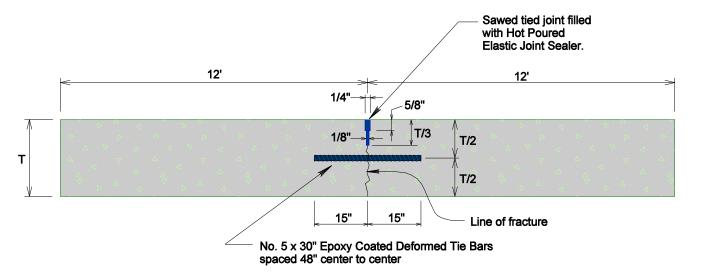
T = Existing and new pavement thickness.

Bar embedded a minimum depth of 9 inches into the existing pavement by utilizing an epoxy resin adhesive.

Bars shall be placed a minimum of 15 inches from existing transverse contraction joints.

Cost for furnishing and inserting drilled in centerline tie bars shall be included in the contract unit price per each for Insert Steel Bar in PCC Pavement.

SAWED LONGITUDINAL JOINT

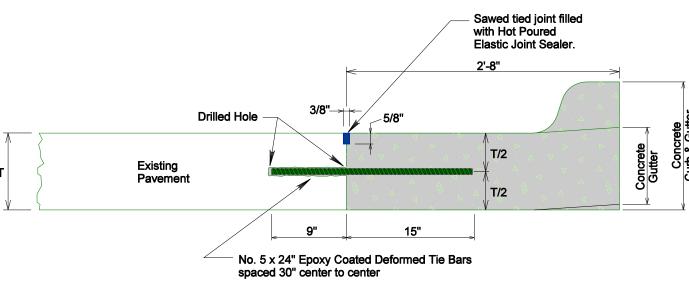


T = New pavement thickness.

The first saw cut to control cracking shall be a minimum of 1/3 the depth of the pavement. Additional sawing for widening the saw cut will be necessary.

Cost for furnishing and inserting centerline tie bars shall be incidental to the contract unit price per square yard for Nonreinforced PCC Pavement Repair.

LONGITUDINAL CONSTRUCTION JOINT WITH DRILLED IN TIE BARS

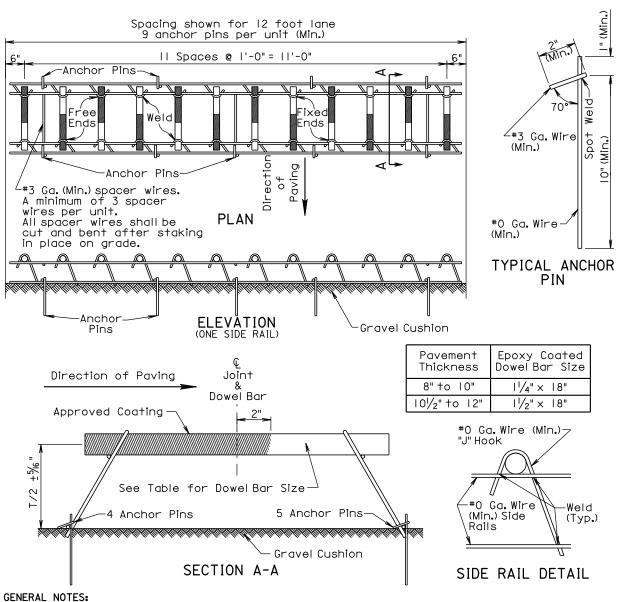


T = Existing and new pavement thickness.

Bar embedded a minimum depth of 9 inches into the existing pavement by utilizing an epoxy resin adhesive.

Bars shall be placed a minimum of 15 inches from existing transverse contraction joints.

Cost for furnishing and inserting drilled in tie bars shall be included in the contract unit price per each for Insert Steel Bar in PCC Pavement.



Longitudinal construction joint tie bars shall be placed a minimum of 15 inches from the transverse contraction joint.

Centerline of individual dowel bars shall be parallel to top of subgrade $\pm 1/8$ inch in 18 inches and to all other dowel bars in the assembly $\pm 1/16$ inch in 18 inches.

Centerline of individual dowel bars shall be parallel to the centerline of the roadway ±1/2 inch in 18 inches.

The transverse contraction joints shall be sawed perpendicular to the centerline of the roadway and the dowel bars shall be centered on the sawed joint \pm 1 inch.

Supporting devices of the type shown on this sheet, or equivalent as approved by the Engineer, shall be used to maintain proper horizontal and vertical alignment of the dowel

December 23, 2007

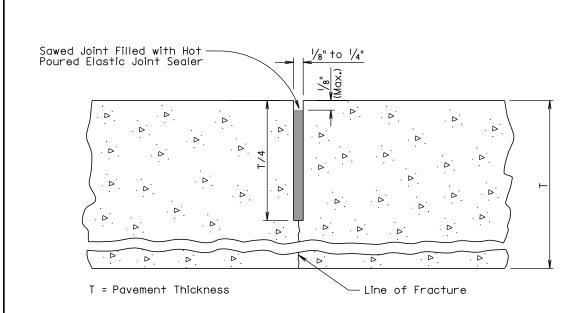
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PCC PAVEMENT DOWEL BAR ASSEMBLY FOR TRANSVERSE CONTRACTION JOINTS PLATE NUMBER 380.01

Sheet | of |

STATE OF	PROJECT	SHEET	TOTAL SHEETS
SOUTH DAKOTA	2011 YANKTON AREA CRCP & PCCP REPAIR	16	24

Plotting Date: 21-MAR-2011



GENERAL NOTES:

The saw cut to control cracking shall be a minimum of $\frac{1}{4}$ the thickness of the pavement.

All hot poured elastic joint sealer material spilled on the surface of the concrete pavement shall be removed as soon as the material has cooled. The extent of removal of material shall be to the satisfaction of the Engineer. All costs for removal of the spilled joint sealer material shall be borne by the Contractor.

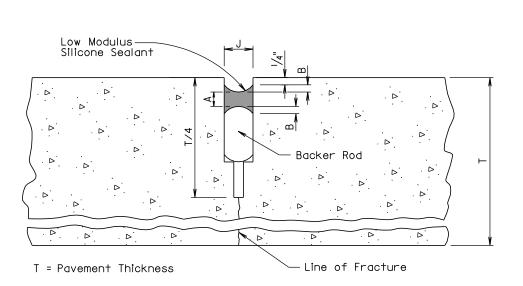
December 23, 2007

D D O T PCC PAVEMENT TRANSVERSE CONTRACTION IOINT WITH OR WITHOUT DOWEL BAR ASSEMBLY PLATE NUMBER 380.03

Sheet | of |

ublished Date: 1st Qtr. 2011

blished Date: 1st Qtr. 2011



LOW MODULUS SILICONE SEALANT ALLOWABLE CONSTRUCTION TOLERANCES							
	J = 3/8"						
A (Min.) A (Max.) B (Min.) B (Max.) (In) (In) (In)							
3/16	1/4						
	J =	1/2"					
A (Min.) (In)	A (Max.) (In)	B (Min.) (In)	B (Max.) (In)				
3/16	3/8	1/8	1/4				
			•				
J = 5/8"							
A (Min.) (In)	A (Max.) (In)	B (Min.) (in)	B (Max.) (In)				
1/4	7/16	1/8	5/16				

GENERAL NOTE:

Published Date: 1st Qtr. 2011

The backer rod shall be a nonmoisture absorbing resilient material approximately 25% larger in diameter than the width of the joint to be sealed.

June 26, 2009

S D D O T

RESEAL PCC PAVEMENT JOINT (SILICONE)

PLATE NUMBER 380./3

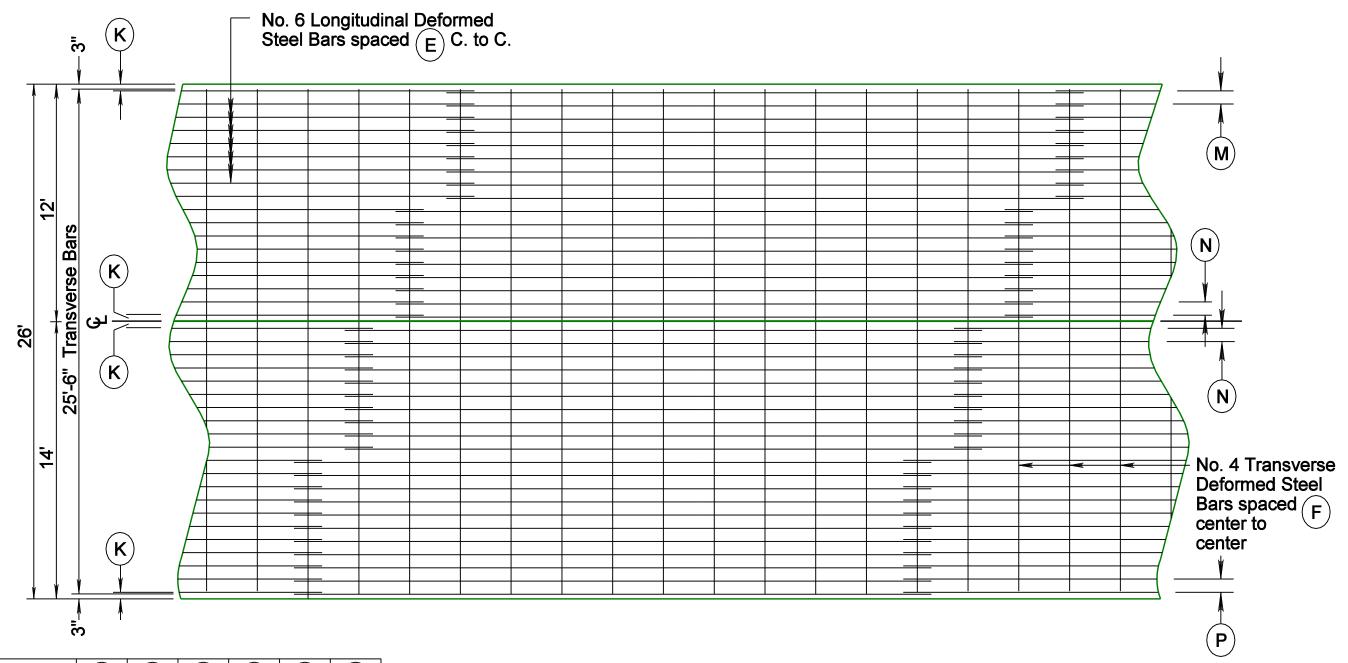
Sheet I of I

STATE OF SOUTH 2011 YANKTON AREA CRCP & PCCP REPAIR 17 24

Plotting Date: 21-MAR-2011

Username - +rmiln+

26' CONTINUOUSLY REINFORCED PCC PAVEMENT - IN PLACE



E	F	K	M	N	P
8"	36"	4"	8"	8"	8"
7½"	36"	4"	4"	4½"	5½"
7"	36"	4"	5"	5"	8"
6½"	48"	3¾"	6½"	6½"	4½"
6½"	48"	3¾"	6½"	6½"	41/2"
6"	48"	4"	5"	5"	5"
6"	48"	4"	5"	5"	5"
	8" 7½" 7" 6½" 6½" 6"	8" 36" 7½" 36" 7" 36" 6½" 48" 6½" 48" 6" 48"	8" 36" 4" 7½" 36" 4" 7" 36" 4" 6½" 48" 3¾" 6½" 48" 3¾" 6" 48" 4"	8" 36" 4" 8" 7½" 36" 4" 4" 7" 36" 4" 5" 6½" 48" 3¾" 6½" 6½" 48" 3¾" 6½" 6" 48" 4" 5"	8" 36" 4" 8" 8" 7½" 36" 4" 4" 4½" 7" 36" 4" 5" 5" 6½" 48" 3¾" 6½" 6½" 6½" 48" 3¾" 6½" 6½" 6" 48" 4" 5" 5"

*Exception for Southbound Lanes on I29 from MRM 62.1 to MRM 72.8.

E = 6 1/2"

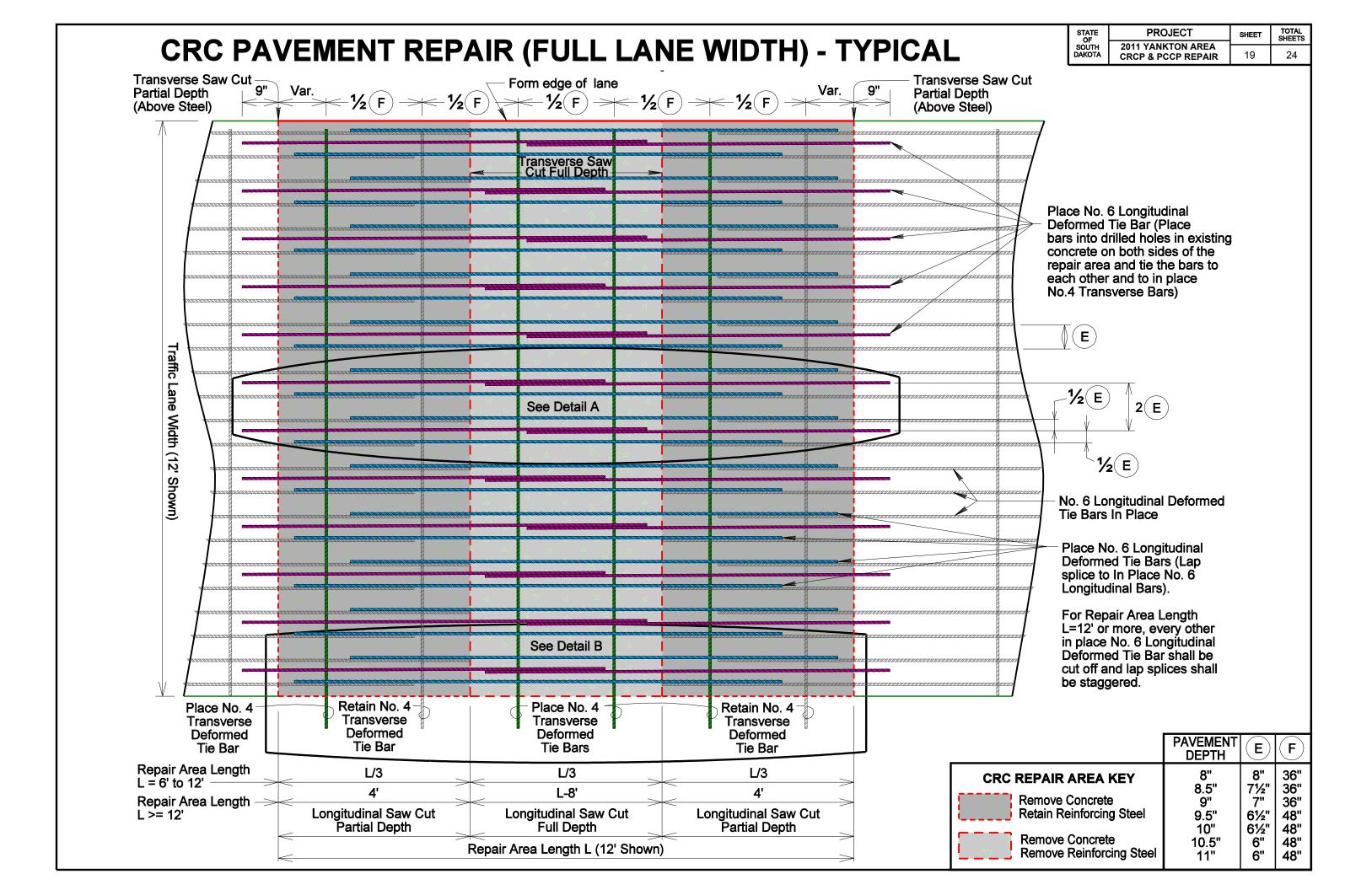
F = 48"

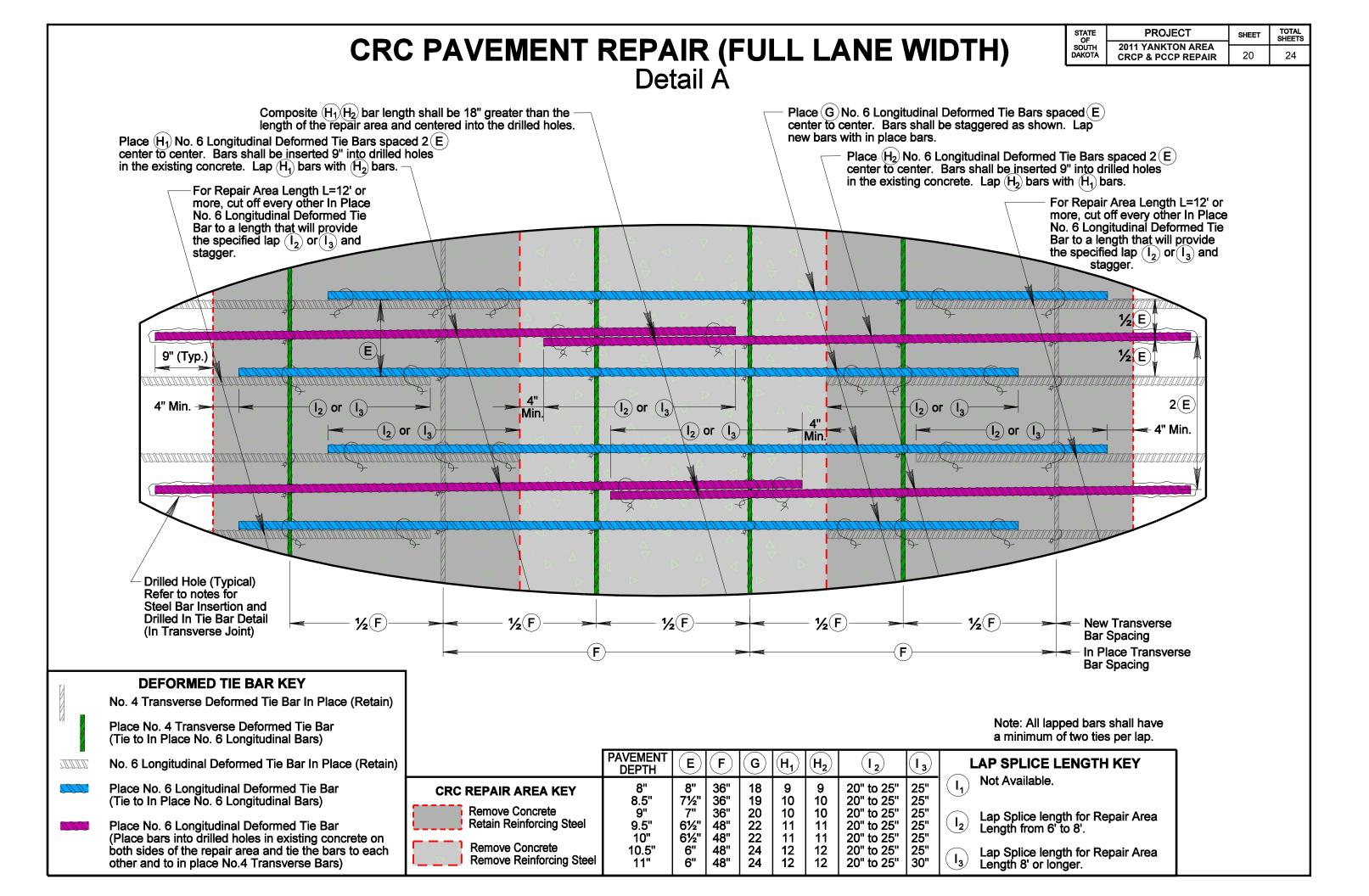
K = 3 3/4"

M = 6 1/2"

N = 6 1/2"

P = 4 1/2"

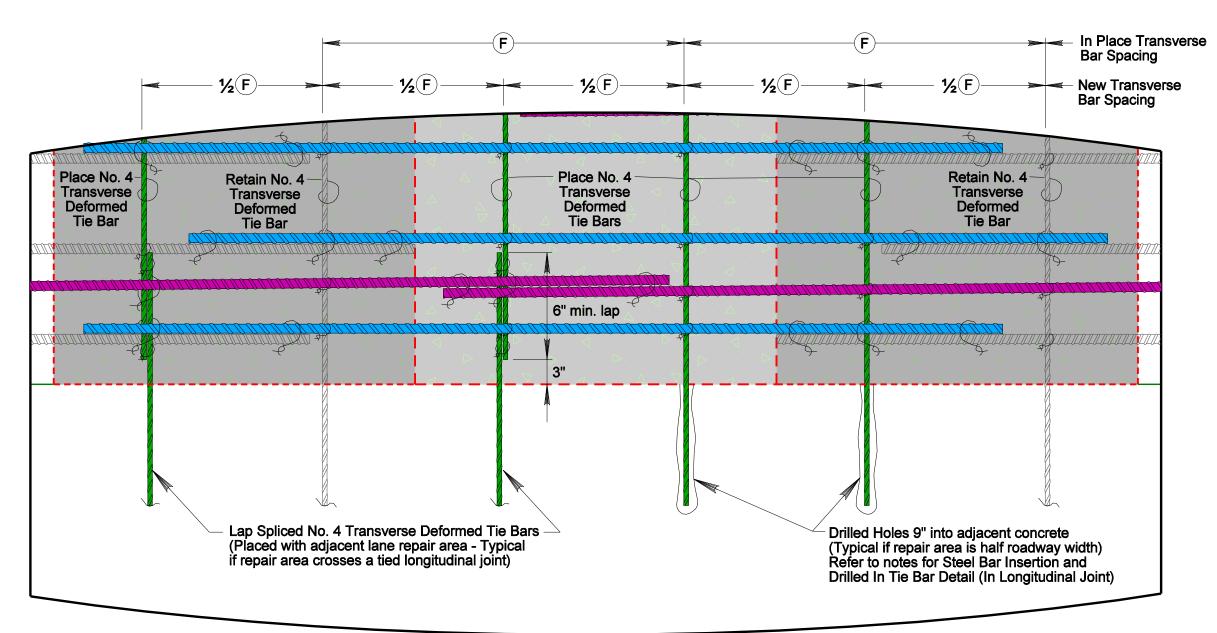




CRC PAVEMENT REPAIR (FULL LANE WIDTH)







DEFORMED TIE BAR KEY

No. 4 Transverse Deformed Tie Bar In Place (Retain)

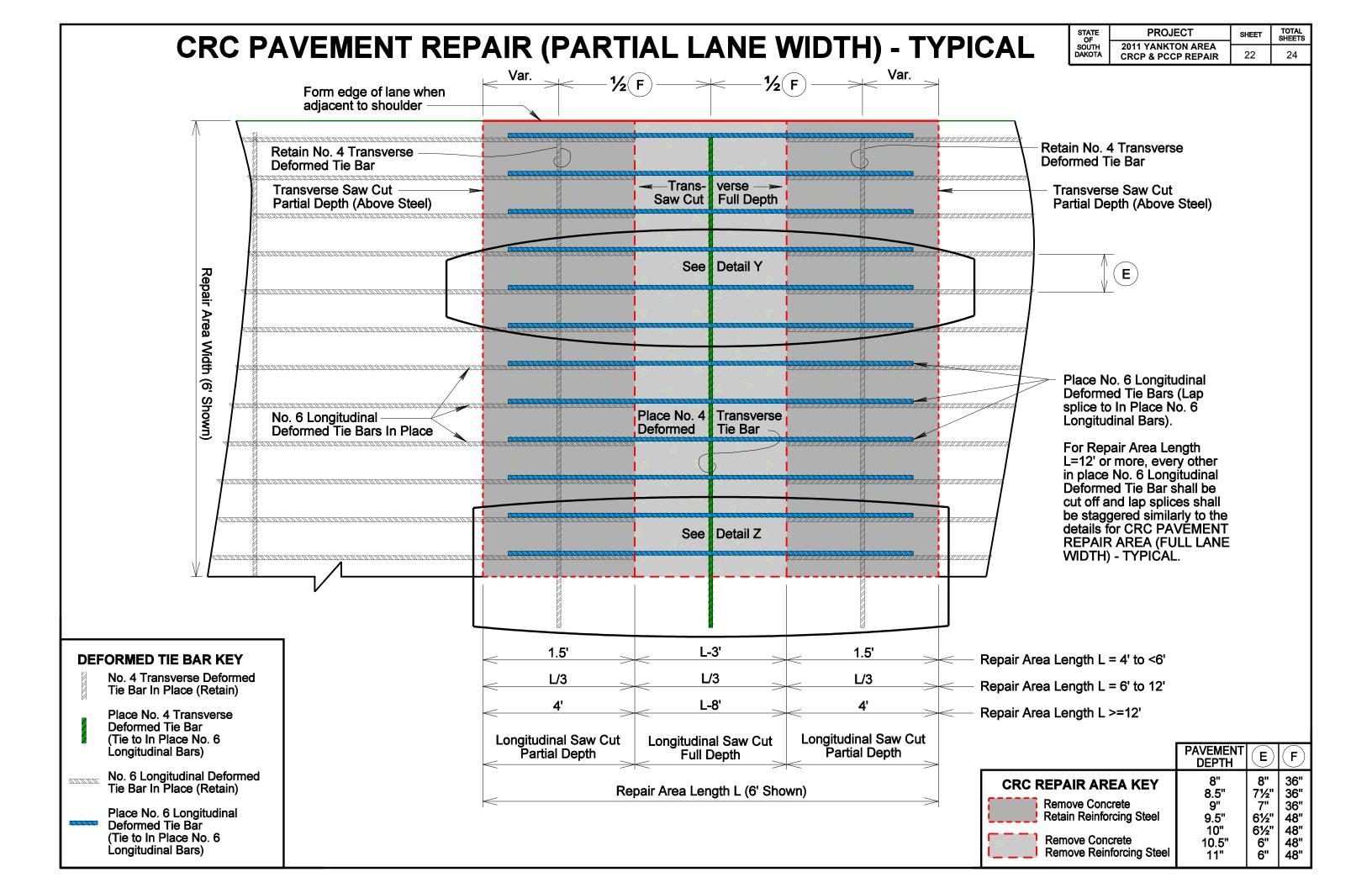
Place No. 4 Transverse Deformed Tie Bar (Tie to In Place No. 6 Longitudinal Bars)

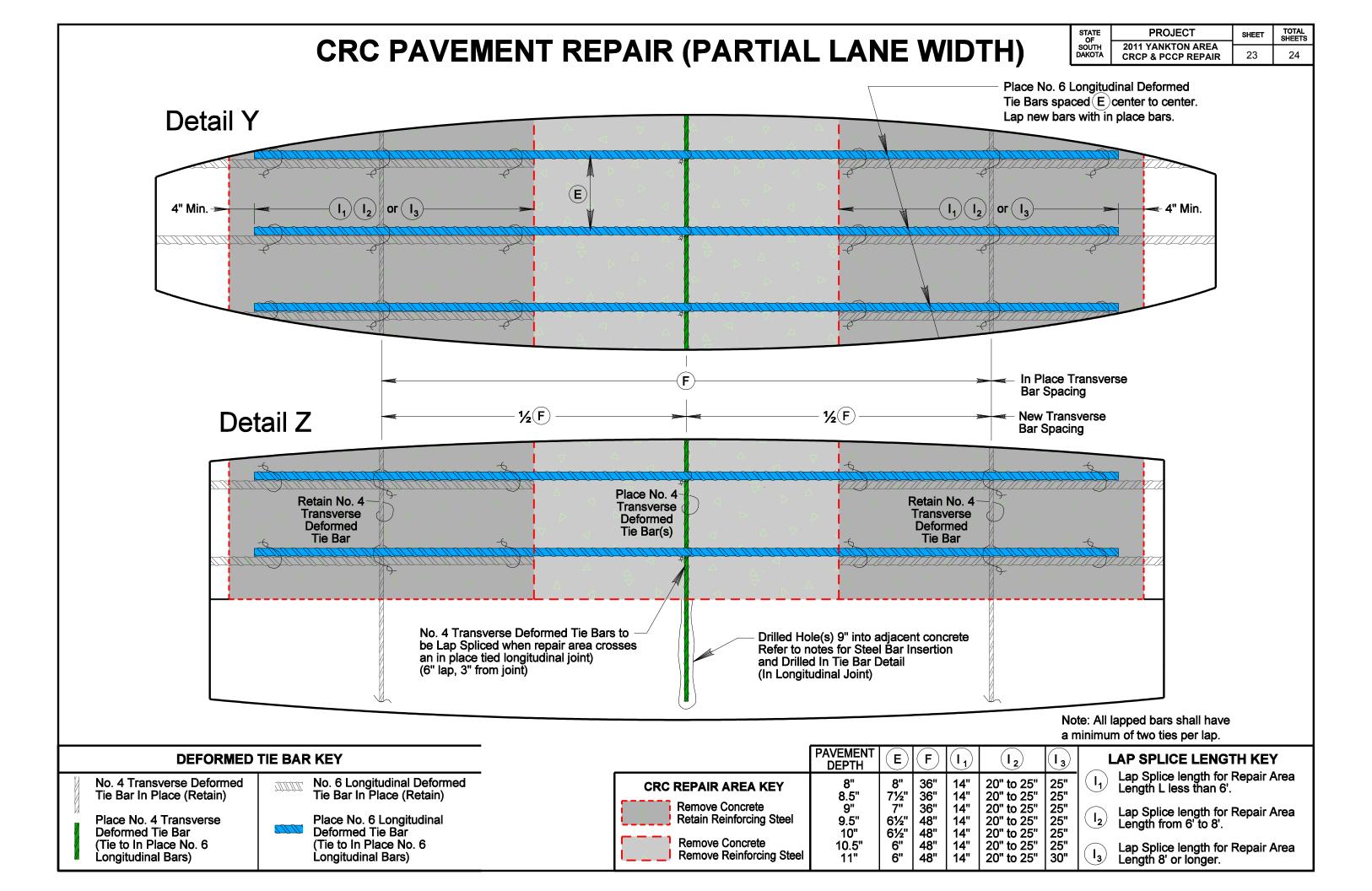
No. 6 Longitudinal Deformed Tie Bar In Place (Retain)

Place No. 6 Longitudinal Deformed Tie Bar (Tie to In Place No. 6 Longitudinal Bars)

Place No. 6 Longitudinal Deformed Tie Bar (Place bars into drilled holes in existing concrete on both sides of the repair area and tie the bars to each other and to in place No.4 Transverse Bars) Note: All lapped bars shall have a minimum of two ties per lap.

	PAVEMENT DEPTH	E	F
CRC REPAIR AREA KEY Remove Concrete Retain Reinforcing Steel Remove Concrete Remove Reinforcing Steel	8" 8.5" 9" 9.5" 10" 10.5"	8" 7½" 7" 6½" 6½" 6" 6"	36" 36" 36" 48" 48" 48" 48"

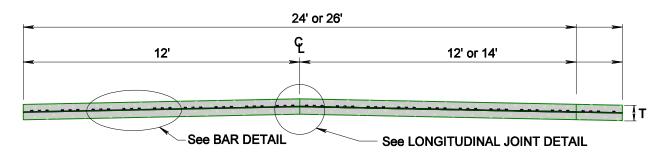


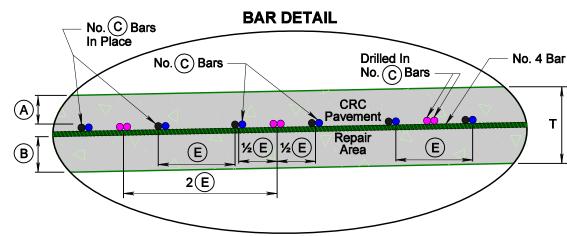


CRC PAVEMENT REPAIR

STATE OF	PROJECT	SHEET	TOTAL SHEETS
SOUTH DAKOTA	2011 YANKTON AREA CRCP & PCCP REPAIR	24	24

TRANSVERSE SECTION SHOWING STEEL PLACEMENT

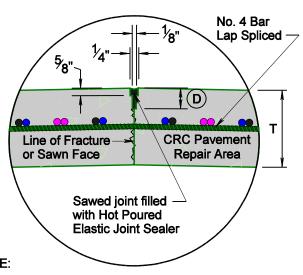




Placement of longitudinal steel bars may vary from +1/2" to -1/2" vertically and 3/4" horizontally. Placement of transverse steel bars may vary from +1/2" to -1/2" vertically and 2" horizontally.

The transverse deformed steel bars will be positioned on acceptable chairs.

LONGITUDINAL JOINT DETAIL



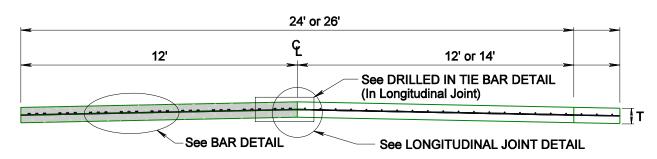
Depth of CRC (T)	A	В	©	D	Œ
*8"	3"	33/4"	6	2"	8
8.5"	31/4"	4"	6	21/8"	7½"
9"	3½"	41/4"	6	21/4"	7"
9.5"	3½"	43/4"	6	23/8"	61/2"
10"	3½"	5½"	6	21/2"	61/2"
10.5"	3 ³ / ₄ "	5½"	6	2 ⁵ / ₈ "	6
11"	4"	5 ³ / ₈ "	7	23/4"	6"
11.5"	3½"	67/8"	5	21/8"	-
11.5"	4"	57/8"	7	21/8"	-
12"	4"	6¾"	7	3"	ا "

* Exception for I29 SBL MRM 83.8 to MRM 97.8 $(A) = 3\frac{1}{4}$ " $(B) = 3\frac{1}{2}$ "

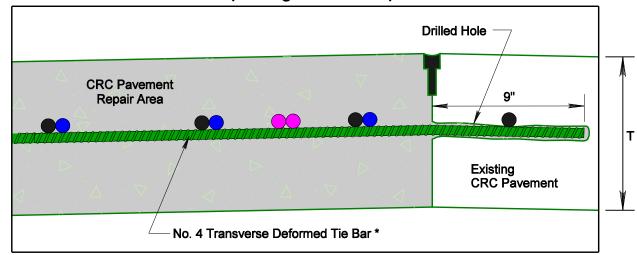
NOTE:

Steel bars for concrete reinforcement shall conform to the requirements of Specification M31 (Grade 60) of the AASHTO Standard Specifications for Deformed Billet Steel Bars for Concrete Reinforcement.

TRANSVERSE SECTION SHOWING STEEL PLACEMENT



DRILLED IN TIE BAR DETAIL (In Longitudinal Joint)



* Transverse Deformed Tie Bar shall be bent slightly to fit into the drilled hole

LONGITUDINAL SECTION SHOWING STEEL PLACEMENT

DRILLED IN TIE BAR DETAIL (In Transverse Joint)

