

(None required)



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#### **ESTIMATE OF QUANTITIES**

#### PCN I6XU

BID ITEM NUMBER	ITEM	QUANTITY	UNIT
009E0010	Mobilization	Lump Sum	LS
320E1200	Asphalt Concrete Composite	200.0	Ton
380E5030	Nonreinforced PCC Pavement Repair	424.4	SqYd
380E5100	Continuously Reinforced PCC Pavement Repair	200.7	SqYd
380E6000	Dowel Bar	801	Each
380E6110	Insert Steel Bar in PCC Pavement	1,329	Each
380E6310	Seal Random Cracks in PCC Pavement	50	Ft
390E0200	Repair Type A Spall	10.0	SqFt
634E0010	Flagging	100.0	Hour
634E0110	Traffic Control Signs	423.6	SqFt
634E0120	Traffic Control, Miscellaneous	Lump Sum	LS
634E0275	Type 3 Barricade	20	Each
634E0420	Type C Advance Warning Arrow Board	2	Each
634E0600	4" Temporary Pavement Marking Tape Type I	2,688	Ft
634E0640	Temporary Pavement Marking	3,840	Ft
634E0900	Portable Temporary Traffic Control Signal	2	Unit
634E1215	Contractor Furnished Portable Changeable Message Sign	2	Each

#### **PCN 177W**

BID ITEM NUMBER	ITEM	QUANTITY	UNIT
009E0197	Mobilization 1	10	Each
380E5030	Nonreinforced PCC Pavement Repair	240.0	SqYd
380E5100	Continuously Reinforced PCC Pavement Repair	100.0	SqYd
380E6000	Dowel Bar	240	Each
380E6110	Insert Steel Bar in PCC Pavement	360	Each
634E0010	Flagging	50.0	Hour
634E0110	Traffic Control Signs	422.0	SqFt
634E0126	Traffic Control for Pavement Repair	10	Site
634E0275	Type 3 Barricade	10	Each
634E0420	Type C Advance Warning Arrow Board	2	Each
634E0640	Temporary Pavement Marking	2,640	Ft
634E1215	Contractor Furnished Portable Changeable Message Sign	2	Each

#### SPECIFICATIONS

Standard Specifications for Roads and Bridges, 2015 Edition and Required Provisions, Supplemental Specifications and Special Provisions as included in the Proposal.

#### **ENVIRONMENTAL COMMITMENTS**

The SDDOT is committed to protecting the environment and uses Environmental Commitments as a communication tool for the Engineer and Contractor to ensure that attention is given to avoid, minimize, and/or mitigate an environmental impact. Environmental commitments to various agencies and the public have been made to secure approval of this project. An agency with permitting authority can delay a project if identified environmental impacts have not been adequately addressed. Unless otherwise designated, the Contractor's primary contact regarding matters

#### ENVIRONMENTAL COMMITMENTS (CONTINUED)

associated with these commitments will be the Project Engineer. During construction, the Project Engineer will verify that the Contractor has met Environmental Commitment requirements. These environmental commitments are not subject to change without prior written approval from the SDDOT Environmental Office.

Additional guidance on SDDOT's Environmental Commitments can be accessed through the Environmental Procedures Manual found at: https://dot.sd.gov/media/documents/EnvironmentalProceduresManual.pdf

For questions regarding change orders in the field that may have an effect on an Environmental Commitment, the Project Engineer will contact the Environmental Engineer at 605-773-3180 or 605-773-4336 to determine whether an environmental analysis and/or resource agency coordination is necessary.

Once construction is complete, the Project Engineer will review all environmental commitments for the project and document their completion.

#### **COMMITMENT C: WATER SOURCE**

The Contractor will not withdraw water with equipment previously used outside the State of South Dakota or previously used in aquatic invasive species (AIS) positive waters within South Dakota without prior approval from the SDDOT Environmental Office. To prevent and control the introduction and spread of invasive species into the project vicinity, all equipment will be power washed with hot water (≥140 °F) and completely dried for a minimum of 7 days prior to subsequent use. South Dakota administrative rule 41:10:04:02 forbids the possession and transport of AIS; therefore, all attached dirt, mud, debris and vegetation must be removed and all compartments and tanks capable of holding standing water must be drained. This includes, but is not limited to, all equipment, pumps, lines, hoses and holding tanks.

The Contractor will not withdraw water directly from streams of the James, Big Sioux, and Vermillion watersheds without prior approval from the SDDOT Environmental Office.

#### Action Taken/Required:

The Contractor will obtain the necessary permits from the regulatory agencies such as the South Dakota Department of Agriculture and Natural Resources (DANR) and the United States Army Corps of Engineers (USACE) prior to water extraction activities.

Additional information and mapping of water sources impacted by Aquatic Invasive Species in South Dakota can be accessed at: http://sdleastwanted.com/maps/default.aspx

#### COMMITMENT E: STORM WATER

Construction activities constitute less than 1 acre of disturbance.

#### Action Taken/Required:

At a minimum and regardless of project size, appropriate erosion and sediment control measures must be installed to control the discharge of pollutants from the construction site.

## COMMITMENT H: WASTE DISPOSAL SITE

The Contractor will furnish a site(s) for the disposal of construction and/or demolition debris generated by this project.

## Action Taken/Required:

Construction and/or demolition debris may be disposed of within the Public ROW.

The waste disposal site(s) will be managed and reclaimed in accordance with the following from the General Permit for Construction/Demolition Debris Disposal Under the South Dakota Waste Management Program issued by the Department of Agriculture and Natural Resources.

The waste disposal site(s) will not be located in a wetland, within 200 feet of surface water, or in an area that adversely affects wildlife, recreation, aesthetic value of an area, or any threatened or endangered species, as approved by the Environmental Office and the Project Engineer.

If the waste disposal site(s) is located such that it is within view of any ROW, the following additional requirements will apply:

- No Dumping Allowed.

The above requirements will not apply to waste disposal sites that are covered by an individual solid waste permit as specified in SDCL 34A-6-58, SDCL 34A-6-1.13. and ARSD 74:27:10:06.

Failure to comply with the requirements stated above may result in civil penalties in accordance with South Dakota Solid Waste Law, SDCL 34A-6-1.31.

Cost associated with furnishing waste disposal site(s), disposing of waste, maintaining control of access (fence, gates and signs), and reclamation of the waste disposal site(s) will be incidental to the various contract items.

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SOUTH DAKOTA	0009-271 & 0009-271	2	40

1. Construction and/or demolition debris consisting of concrete, asphalt concrete, or other similar materials will be buried in a trench separate from wood debris. The final cover over the construction and/or demolition debris will consist of a minimum of 1 foot of soil capable of supporting vegetation. Waste disposal sites provided outside of the Public ROW will be seeded in accordance with Natural Resources Conservation Service recommendations. The seeding recommendations may be obtained through the appropriate County NRCS Office. The Contractor will control the access to waste disposal sites not within the Public ROW with fences, gates, and placement of a sign or signs at the entrance to the site stating,

2. Concrete and asphalt concrete debris may be stockpiled within view of the ROW for a period not to exceed the duration of the project. Prior to project completion, the waste will be removed from view of the ROW or buried, and the waste disposal site reclaimed as noted above.

#### COMMITMENT I: HISTORIC PRESERVATION OFFICE CLEARANCES

State Historic Preservation Office (SHPO or THPO) concurrence has not been obtained for this project.

#### Action Taken/Required:

All earth disturbing activities require a cultural resource review prior to scheduling the pre-construction meeting. This work includes but is not limited to: Contractor furnished material sources, material processing sites, stockpile sites, storage areas, plant sites, and waste areas.

The Contractor will arrange and pay for a record search and when necessary, a cultural resource survey. The Contractor has the option to contact the state Archaeological Research Center (ARC) at 605-394-1936 or another qualified archaeologist, to obtain either a records search or a cultural resources survey. A record search might be sufficient for review if the site was previously surveyed; however, a cultural resources survey may need to be conducted by a qualified archaeologist.

The Contractor will provide ARC with the following: a topographical map or aerial view in which the site is clearly outlined, site dimensions, project number, and PCN. If applicable, provide evidence that the site has been previously disturbed by farming, mining, or construction activities with a landowner statement that artifacts have not been found on the site.

The Contractor will submit the cultural resources survey report to SDDOT Environmental Office, 700 East Broadway Avenue, Pierre, SD 57501-2586. SDDOT will submit the information to the appropriate SHPO/THPO. Allow **30 Days** from the date this information is submitted to the Environmental Engineer for SHPO/THPO review.

In the event of an inadvertent discovery of human remains, funerary objects, or if evidence of cultural resources is identified during project construction activities, then such activities within 100 feet of the inadvertent discovery will

immediately cease and the Project Engineer will be immediately notified. The Project Engineer will contact the SDDOT Environmental Office, who will contact the appropriate SHPO/THPO within 48 hours of the discovery to determine an appropriate course of action.

The Contractor is responsible for obtaining any additional permits and clearances for Contractor furnished material sources, material processing sites, stockpile sites, storage areas, plant sites, and waste areas that affect wetlands, threatened and endangered species, or waterways. The Contractor will not utilize a site known or suspected of having contaminated soil or water. The Contractor will provide the required permits and clearances to the Project Engineer at the preconstruction meeting.

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#### **UTILITIES**

The Contractor will contact the involved utility companies through South Dakota One Call (1-800-781-7474) prior to starting work. It will be the responsibility of the Contractor to coordinate work with the utility owners to avoid damage to existing facilities.

Utilities are not planned to be affected on this project. If utilities are identified near the improvement area through the SD One Call process as required by South Dakota Codified Law 49-7A and Administrative Rule Article 20:25; the Contractor will contact the Project Engineer to determine if project changes are necessary to avoid utility impacts.

#### SCOPE OF WORK

This project consists of nonreinforced and continuously reinforced PCCP repair as well as on-call nonreinforced and continuously reinforced PCCP pavement repair. Once a repair need is identified, the Contractor will be given notification of the needed repair and the Contractor will have 21 calendar days from the date of notification to fully complete the identified repair. On call repair needs may exceed estimated quantities.

Full depth areas may vary in length and width; however, the minimum length is 4 feet for partial lane width repair areas and the minimum length is 4.5 feet for full lane width repair areas. Minimum size for small repair areas – existing steel maintained, is 1 foot x 1 foot.

#### MOBILIZATION

Mobilization for nonreinforced and continuously reinforced PCCP repair as included in PCN i6xu will be paid at the contract lump sum price for the project.

Mobilization for the on-call nonreinforced and continuously reinforced PCCP repair as included in PCN i77w will be measured by each mobilization. A mobilization will be considered each time the Contractor has to mobilize to a route or location on which they are not currently working. Only one mobilization will be measured at each new location regardless of the amount of pavement repair needed unless the Contractor completes work and must return at a later date to complete additional repairs. Pavement repair locations on the same route within 5 miles of each other will be considered one mobilization.

#### **EXISTING NRC PAVEMENT – SD 11**

The existing pavement is 8.5" x 20' NRC Pavement. Pavement dimensions at other locations will vary.

Existing contraction joints are spaced at approximately 20'. Longitudinal joints are reinforced with No. 4 x 30" deformed tie bars spaced 48" center to center. Transverse joints are reinforced with  $1\frac{1}{4}$ " x 18" plain round dowel bars spaced 12" center to center.

The aggregate in the existing NRC Pavement is quartzite.

#### **EXISTING CRC PAVEMENT – 129 SB**

The existing pavement is 11" x 26' CRC Pavement. The longitudinal reinforcing steel consists of No. 6 deformed bars spaced 6" center to center, and the transverse reinforcing steel consists of No. 4 deformed bars spaced 48" center to center. Pavement dimensions at other locations will vary.

The aggregate in the existing CRC Pavement is quartzite.

#### **RESTORATION OF GRAVEL CUSHION**

An inspection of the gravel cushion will be made after removing concrete from each pavement replacement area. Areas of excess moisture will be dried to the satisfaction of the Engineer. Loose material will be removed. Each replacement area will be leveled and compacted to the satisfaction of the Engineer.

If additional gravel cushion material is required, the Contractor will furnish, place and compact gravel cushion to the satisfaction of the Engineer at no additional

Cost for this work will be incidental to the contract unit prices per square yard for Nonreinforced PCC Pavement Repair and Continuously Reinforced PCC Pavement Repair.

#### **GRAVEL CUSHION**

If quarried ledge rock is used in the Gravel Cushion, a maximum blend of 40% quarried ledge rock will be allowed.

#### ASPHALT CONCRETE COMPOSITE

200 tons of asphalt concrete composite has been included in the plans to repair damage to interstate shoulders adjacent to PCCP repair locations. It is expected that the Contractor will place traffic control devices on the shoulder adjacent to repair locations to discourage traffic from driving on the shoulder.

#### NONREINFORCED PCC PAVEMENT REPAIR - GENERAL

New pavement thickness will equal existing pavement thickness ( $T_N = T$ ).

Locations and size (length or width) of concrete repair areas are subject to change in the field, at the discretion of the Engineer, at no additional cost to the state. Payment will be based on actual area replaced.

Existing concrete pavement will be sawed full depth at the beginning and end of the NRCP repair areas. When either the beginning or end of a NRCP repair area falls close to an existing joint or crack, the NRCP repair area will be extended to eliminate the existing joint or crack. Where possible, new working joints will be adjacent to existing working joints.

Saw cuts that extend beyond the repair area will be minimized and filled with a non-shrinkage mortar mix at the Contractor's expense.

Existing concrete pavement in the replacement areas will be removed by the lift out method or by means that minimize damage to the base and sides of remaining in place concrete. Removed material will be removed from within the right-of-way by the end of the workday. Damage to adjacent concrete caused by the Contractor's operations will be removed and replaced at the Contractor's expense.

If the pavement replacement area is entirely on either side of the existing contraction joint, the location of one of the working joints will be at the original location. Any existing dowel bar assemblies/steel bars will be sawed off and removed.

At full roadway width repairs and when specified, a working joint will be reconstructed at both ends of each pavement replacement area as shown in these plans. Use only for full roadway width repairs (across all lanes)

Concrete placed adjacent to gravel and asphalt concrete shoulders will be formed full depth to match the width of existing concrete pavement. Asphalt concrete shoulders adjacent to concrete pavement replacements will be repaired with new hot-mix asphalt concrete.

At repair locations where the new working joint is not opposite the existing working joint, the Contractor will place a  $\frac{1}{4}$ " preformed asphalt expansion joint material along the longitudinal joint from the existing working joint to the new

#### NONREINFORCED PCC PAVEMENT REPAIR - GENERAL (CONTINUED)

working joint. The expansion joint material will meet the requirements of AASHTO M33. Cost for this material will be incidental to the contract unit price per square yard for Nonreinforced PCC Pavement Repair.

The initial contraction joint sawing will be performed as soon as practical after placement to avoid random cracking.

Joints (longitudinal and transverse) through and around the repair areas will be sawed and sealed in accordance with the details shown in these plans. Refer to Saw and Seal Joints notes.

## NONREINFORCED PCC PAVEMENT REPAIR

Concrete will meet the requirements stated in Section 380 of the specifications, except as modified by the following notes:

The fine aggregate will be screened over a one-inch square-opening screen just prior to introduction into the concrete paving mix if required by the Engineer.

The slump requirement will be limited to 3" maximum after water reducer is added and the concrete will contain 4.5% to 7.0% entrained air. The concrete will contain a minimum of 50% coarse aggregate by weight. Coarse aggregate will be crushed ledge rock, Size No. 1 unless an alternative gradation is approved by the Concrete Engineer as part of the mix design submittal. The mix design will contain at least 650 lbs of Type I or II cement or 600 lbs of Type III cement per cubic yard. The minimum 28 day compressive strength will be 4,000 psi. The Contractor is responsible for the mix design used. The Contractor will submit a mix design and supporting documentation for approval at least 2 weeks prior to use.

The use of a v be required.

Concrete will be cured with white pigmented curing compound (AASHTO M148, Type 2) applied as soon as practical at a rate of 125 square feet per gallon. Concrete will be cured for a minimum of 48 hours before opening to traffic. The 48 hours is based upon a concrete surface temperature of 60°F or higher throughout the cure period. If the concrete temperature falls below 60°F, the cure time will be extended, or other measures taken, at no additional cost to the State. A strength of 3,500 psi must be attained prior to opening to traffic.

Upon placement of the concrete, repair areas will be straight edged to ensure a smooth riding surface and will be textured longitudinally with the pavement by finishing with a stiff broom. Repair areas will then be checked with a 10' foot straight edge. The permissible longitudinal and transverse surface deviation will be 1/8" in 10'.

Concrete will be covered with suitable insulation blanket consisting of a layer of closed cell polystyrene foam protected by at least one layer of plastic. Insulation blanket will have an R-value of at least 0.5, as rated by the manufacturer. Insulation blanket will be left in place, except for joint sawing operations, until the 3,500 psi is attained. Insulation blanket will be overlapped on to the existing concrete by 4'. This requirement for covering repair areas with insulation blankets may be waived during periods of hot weather upon approval of the Engineer.

Cost for performing the aforementioned work including sawing and removing concrete, furnishing and placing concrete, sawing and sealing joints, repairing gravel and asphalt concrete shoulders, labor, tools and equipment will be included in the contract unit price per square yard for Nonreinforced PCC Pavement Repair.

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The use of a water reducer at manufacturer's recommended dosage will

#### CONTINUOUSLY REINFORCED PCC PAVEMENT REPAIR

New pavement thickness will equal existing pavement thickness ( $T_N = T$ ).

Locations and size (length or width) of pavement repair areas are subject to change in the field, at the discretion of the Engineer, at no additional cost to the state. Payment will be based on actual area replaced.

The Engineer will mark the location of the area to be repaired on construction. Where repair crosses both lanes, the passing lane should be repaired first.

#### Full Lane Width Repair and Partial Lane Width Repair

The Contractor will saw the in place concrete transversely at four locations for each repair area. Two saw cuts will be full depth. The other two saw cuts will be partial depth saw cuts and will be made to a depth just above the in place reinforcing steel and be placed outside of the previous full depth saw cuts. The outside cuts will be a minimum of 6" from the nearest tight crack outside of the patch.

The Contractor will lift out or break out the center section (including reinforcing steel). In the salvaged rebar sections of the repair areas, the use of 30 or 60 pound hammers will be allowed outside of one foot from the newly created header joint. To prevent damage to the joint and surrounding concrete, only light chipping hammers (not exceeding 15 pounds) will be allowed within the last foot adjacent to the newly created header joint to remove the remaining concrete at each end of the repair area, leaving the reinforcing steel in place.

#### Small Repair – Existing Steel Retained

The Contractor will saw the in place concrete around the periphery of each repair area to a depth of 2" (above the in place reinforcing steel). The cuts will be a minimum of 6" from the nearest tight crack outside of the patch.

Light chipping hammers (not exceeding 15 pounds) will be used to remove the concrete from the repair area, leaving the reinforcing steel in place.

Saw cuts that extend beyond the repair area will be minimized and filled with a non-shrinkage mortar mix at the Contractor's expense.

Care will be taken not to cut, bend or otherwise damage the in place reinforcing steel. Damage to in place reinforcing steel or to in place concrete beyond the repair area will be replaced at the Contractor's expense, to the satisfaction of the Engineer.

The Contractor will remove and dispose of the in place concrete and in place asphalt concrete.

Existing exposed reinforcing steel and concrete faces will be cleaned by sandblasting and compressed air to remove dirt and debris prior to placement of concrete.

Place reinforcing steel according to the notes for REINFORCING STEEL (CRCP) and STEEL BAR INSERTION (CRCP).

Concrete placed adjacent to asphalt concrete shoulders will be formed full depth to match the width of existing concrete pavement. The excavated area of the asphalt concrete shoulder adjacent to repair areas will be filled with asphalt concrete.

Concrete will not be placed in the repair areas before 12:00pm and should be placed in the late afternoon. Temperature of the concrete at the time of placement will be between 50°F and 90°F. The temperature of the concrete will be maintained above 40°F during the curing period.

Concrete will meet the requirements stated in Section 380 of the specifications, except as modified by the following notes:

#### CONTINUOUSLY REINFORCED PCC PAVEMENT REPAIR (CONTINUED)

The fine aggregate will be screened over a one-inch square-opening screen just prior to introduction into the concrete paving mix if required by the Engineer.

The slump requirement will be limited to 3" maximum after water reducer is added and the concrete will contain 4.5% to 7.0% entrained air. The concrete will contain a minimum of 50% coarse aggregate by weight. Coarse aggregate will be crushed ledge rock, Size No. 1 unless an alternative gradation is approved by the Concrete Engineer as part of the mix design submittal. The mix design will contain at least 650 lbs of Type I or II cement or 600 lbs of Type III cement per cubic yard. The minimum 28 day compressive strength will be 4,000 psi. The Contractor is responsible for the mix design used. The Contractor will submit a mix design and supporting documentation for approval at least 2 weeks prior to use.

The use of a water reducer at manufacturer's recommended dosage will be required.

Concrete will be cured with white pigmented curing compound (AASHTO M148, Type 2) applied as soon as practical at a rate of 125 square feet per gallon. Concrete will be cured a minimum of 48 hours before opening to traffic. The 48 hours is based upon a concrete surface temperature of 60°F or higher throughout the cure period. If the concrete temperature falls below 60°F, the cure time will be extended, or other measures taken, at no additional cost to the State. A strength of 3,500 psi must be attained prior to opening to traffic.

Concrete will be covered with suitable insulation blanket consisting of a layer of closed cell polystyrene foam protected by at least one layer of plastic. Insulation blanket will have an R-value of at least 0.5, as rated by the manufacturer. Insulation blanket will be left in place, except for joint sawing operations until 3,500 psi is attained. Insulation blanket will be overlapped on to the existing concrete by 4'. This requirement for covering repair areas with insulation blankets may be waived during periods of hot weather upon approval of the Engineer.

Upon placement of the concrete, repair areas will be straight edged to ensure a smooth riding surface and will be textured longitudinally with the pavement by finishing with a stiff broom. Repair areas will then be checked with a 10' foot straight edge. The permissible longitudinal and transverse surface deviation will be 1/8" in 10'.

Cost for performing the aforementioned work including sawing, chipping and removing concrete, sandblasting, cleaning, furnishing and placing concrete and reinforcing steel, finishing and curing, replacing asphalt concrete shoulders, labor and equipment will be included in the contract unit price per square yard for Continuously Reinforced PCC Pavement Repair.

#### **REINFORCING STEEL (CRCP)**

Reinforcing steel will conform to Section 1010.

After removal of the in place concrete and repair of the gravel cushion, new reinforcing steel will be installed. Refer to the CRC Pavement Repair Area lavouts for details.

#### At full lane and partial lane width repair areas:

New longitudinal bars will be lap spliced with the preserved in place longitudinal bars (New bar diameter to match in place bar diameter).

Additional transverse bars will be centered between the in place transverse bars throughout the length of the repair area. The spacing of transverse bars

### **REINFORCING STEEL (CRCP) (CONTINUED)**

in the completed repair area should be half the spacing of the in place transverse reinforcing steel.

The additional transverse bars will be lap spliced with No. 5 x 24" epoxy coated deformed tie bars inserted 9" into the existing concrete. Drilled holes will be required. Tie bars will be inserted according to the notes for STEEL BAR INSERTION (CRCP).

At full lane width repair areas: Additional longitudinal bars will be centered between every other set of two spliced longitudinal bars throughout the width of the repair area. These additional bars will extend 9" into the existing concrete on both sides of the repair area. Drilled holes will be required and the additional longitudinal bars will be inserted in accordance with the notes for STEEL BAR INSERTION (CRCP). The additional longitudinal bars will then be lap spliced.

Cost for this work, including reinforcing steel, ties, labor and equipment will be incidental to the contract unit price per square yard for Continuously Reinforced PCC Pavement Repair.

### **STEEL BAR INSERTION (CRCP)**

Steel bars will conform to Section 1010.

Locations and quantities of concrete repair are subject to change in the field at the discretion of the Engineer. The Contractor will be responsible for ordering the actual quantity of steel bars necessary to complete the work.

Longitudinal deformed tie bars will be inserted 9 inches into the in place concrete at the transverse joint and centered between every other set of two spliced longitudinal bars throughout the width of the repair area. Transverse deformed bars will be lap spliced with deformed tie bars which are inserted 9 inches into the in place concrete at the longitudinal joint throughout the length of the repair area. Refer to the notes for REINFORCING STEEL (CRCP). An epoxy resin adhesive must be used to anchor the steel bar in the drilled hole as per Section 380.3 C.1.

Holes drilled into the existing concrete pavement will be located at mid-depth of the slab and true and normal except that in transverse joints, the drilled in longitudinal steel bar angle will be slightly under 90° to allow for centering of the lap splice between existing longitudinal steel.

A rigid frame or mechanical device will be required to guide the drill to ensure proper horizontal and vertical alignment of the steel bars in the drilled holes.

Cost for reinforcing steel (except the inserted No. 5 x 24" epoxy coated deformed tie bars) will be incidental to the contract unit price per square vard for Continuously Reinforced PCC Pavement Repair.

Cost for drilling holes, furnishing and applying epoxy resin adhesive, furnishing and inserting No. 5 x 24" epoxy coated deformed tie bars into the drilled holes, inserting reinforcing steel bars into the drilled holes, and any incidentals necessary to complete the work will be included in the contract unit price per each for Insert Steel Bar in PCC Pavement.

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#### SAW AND SEAL LONGITUDINAL JOINTS (CRCP)

Longitudinal joints (in line with existing longitudinal joints) at concrete repair areas will be sawed and sealed.

Joint sealing will conform to Section 380.3 P.

Longitudinal joints will be sealed with Low Modulus Silicone Sealant or Hot Poured Elastic Joint Sealer.

Cost for sawing and sealing of the longitudinal construction joint will be incidental to the contract unit price per square yard for Continuously Reinforced PCC Pavement Repair.

#### **STEEL BAR INSERTION (NRCP)**

Steel bars will conform to Section 1010.

Locations and quantities of concrete repair are subject to change in the field at the discretion of the Engineer. The Contractor will be responsible for ordering the actual quantity of steel bars necessary to complete the work.

#### For existing pavement thickness greater than or equal to 10.5" (T >= 10.5"):

The Contractor will insert the steel bars  $(1\frac{1}{2}^{"} \times 18^{"})$  epoxy coated plain round dowel bars and No. 11 x 18" epoxy coated deformed tie bars for transverse joints and No. 5 x 24" epoxy coated deformed tie bars for longitudinal joints) into drilled holes in the existing concrete pavement. An epoxy resin adhesive must be used to anchor the steel bar in the drilled hole as per Section 380.3 C.1.

# For existing pavement thickness greater than or equal to 8.5" and less than 10.5" ( $T \ge 8.5$ " and T < 10.5"):

The Contractor will insert the steel bars  $(1\frac{1}{4}^{"} \times 18^{"})$  epoxy coated plain round dowel bars and No. 9 x 18" epoxy coated deformed tie bars for transverse joints and No. 5 x 24" epoxy coated deformed tie bars for longitudinal joints) into drilled holes in the existing concrete pavement. An epoxy resin adhesive must be used to anchor the steel bar in the drilled hole as per Section 380.3 C.1.

#### For existing pavement thickness less than 8.5" (T < 8.5"):

The Contractor will insert the steel bars (1" x 18" epoxy coated plain round dowel bars and No. 8 x 18" epoxy coated deformed tie bars for transverse joints and No. 5 x 24" epoxy coated deformed tie bars for longitudinal joints) into drilled holes in the existing concrete pavement. An epoxy resin adhesive must be used to anchor the steel bar in the drilled hole as per Section 380.3 C.1.

Steel bars will be inserted in the transverse joint on 18" centers. The first steel bar in the transverse joint will be placed 9" from the edge of the slab closest to centerline. Steel bars will be inserted in the longitudinal joint on 30" centers and will be a minimum of 15" from either transverse joint. A typical one-lane patch 12' wide and 6' long will require 18 steel bars (8 in each transverse joint and 2 in the longitudinal joint). It will be necessary to laterally adjust the location of some of the inserted steel bars when the dimensions above interfere with existing steel bar locations.

A rigid frame or mechanical device will be required to guide the drill to ensure proper horizontal and vertical alignment of the steel bars in the drilled holes.

#### SAW AND SEAL JOINTS (NRCP)

Longitudinal and transverse joints at concrete repair areas will be sawed and sealed.

Joint sealing will conform to Section 380.3 P.

Longitudinal and transverse joints in urban sections will be sealed with Hot Poured Elastic Joint Sealer. Transverse joints in rural sections will be sealed with Low Modulus Silicone Sealant. Longitudinal joints in rural sections may be sealed with either Hot Poured Elastic Joint Sealer or Low Modulus Silicone Sealant.

Cost for sawing and sealing of the longitudinal construction joint and both transverse joints will be incidental to the contract unit prices per square yard for Nonreinforced PCC Pavement Repair.

#### **REPAIR TYPE A SPALLS**

Concrete Patch Material will be Type III conforming to Section 390.2 B.3.

As an alternative, the Contractor may remove concrete by milling, provided it produces results similar to the sawing and chipping process described in the Specifications.

It is anticipated that a number of locations scheduled for Type A Spall Repair will have deteriorated to the point of needing full depth repair. Additional Quantities are included in the Table(s) for NRC Pavement Repair for this work. The Engineer will determine these locations on construction.

Spalls which are repaired according to plans and specifications and exhibit partial respalling or cracking, will be repaired to the satisfaction of the Engineer at no additional cost to the State.

#### SEAL RANDOM CRACKS IN PCC PAVEMENT (NRCP)

Random cracks will be repaired in accordance with the detail for Sealing Random Cracks. Reservoir dimensions may vary slightly from the details, due to the nature of this operation. However, any variance due to Contractor negligence will be repaired at the Contractor's expense.

Only those random cracks in the existing concrete pavement that are open and accept water and incompressible materials as selected by the Engineer will be prepared and sealed with either Low Modulus Silicone Sealant or Hot Poured Elastic Joint Sealer.

Prior to sealing, each random crack will be routed and thoroughly cleaned with compressed air or by other methods satisfactory to the Engineer. Routing will be performed with a saw designed for that purpose.

Random cracks narrower than  $\frac{1}{2}$  inch will be routed and sealed  $\frac{1}{2}$  inch wide by  $\frac{1}{2}$  inch deep.

Random cracks wider than  $\frac{1}{2}$  inch may require the placement of a backer rod prior to sealing. Use of backer rod should be limited to locations where, once placed, the top of the backer rod will be a minimum of  $2\frac{1}{4}$  inches below the top surface of the pavement. The hot pour in cracks wider than  $\frac{1}{2}$ " should be placed 2 inch thick with the final surface of the hot pour remaining recessed  $\frac{1}{4}$  inch below the top surface of the pavement.

Sealant will be placed in the routed reservoir with equipment and by methods that insure complete and uniform filling. Hot Poured Elastic Joint Sealer will be placed level with the driving surface of the concrete for cracks  $\frac{1}{2}$ " or narrower. Low Modulus Silicone Sealant will have a tooled surface with the top middle portion of the sealant recessed. Any excess or overrun of sealant will be removed by the Contractor at no additional cost to the State.

Acceptance of the Low Modulus Silicone Sealant and Hot Poured Elastic Joint Sealer will be based on visual inspection by the Engineer.

## SEAL RANDOM CRACKS IN PCC PAVEMENT (NRCP) (CONTINUED)

Seal Random Cracks in PCC Pavement will be measured by the foot to the nearest 0.1 foot of random cracks sealed and accepted and will be paid for at the contract unit price per foot measured for payment. Payment will be full compensation for labor, equipment, material and incidentals required for crack routing, cleaning, furnishing and installing backer rod when necessary, furnishing and placing sealant and removing routed and foreign material from the roadway.



STATE OF	PROJECT	SHEET	TOTAL SHEETS
SOUTH DAKOTA	0009-271 & 0009-271	6	40

## SEALING RANDOM CRACKS (NRCP)

#### SEAL RANDOM CRACKS IN PCC PAVEMENT (CRCP)

Random cracks that exhibit minor spalling will be routed, sealed and overbanded in accordance with the detail for Sealing Random Cracks. Reservoir dimensions may vary slightly from the details, due to the nature of this operation. However, any variance due to Contractor negligence will be repaired at the Contractor's expense.

Only those random cracks in the existing concrete pavement that are open and accept water and incompressible materials as selected by the Engineer will be prepared, sealed and overbanded with Hot Poured Elastic Joint Sealer. Typically, patterned cracks associated with the underlying steel reinforcement should not be routed and sealed.

Prior to sealing, each random crack will be routed and thoroughly cleaned with compressed air or by other methods satisfactory to the Engineer. Routing will be performed with a saw designed for that purpose.

Random cracks narrower than  $\frac{1}{2}$  inch will be routed and sealed  $\frac{1}{2}$  inch wide by  $\frac{1}{2}$  inch deep.

Random cracks wider than  $\frac{1}{2}$  inch may require the placement of a backer rod prior to sealing. Use of backer rod should be limited to locations where, once placed, the top of the backer rod will be 2 inches below the top surface of the pavement, resulting in a maximum hot pour depth of 2 inches.

Sealer will be placed in the routed reservoir with equipment and by methods that insure complete and uniform filling. Sealer will be overbanded to prohibit further respalling of the joint.

Acceptance of the sealer will be based on visual inspection by the Engineer.

Seal Random Cracks in PCC Pavement will be measured by the foot to the nearest 0.1 foot of random cracks sealed and accepted and will be paid for at the contract unit price per foot measured for payment. Payment will be full compensation for labor, equipment, material and incidentals required for crack routing, cleaning, furnishing and installing backer rod when necessary, furnishing and placing sealant, overbanding and removing routed and foreign material from the roadway.

#### SEALING RANDOM CRACKS (CRCP)



	STATE OF	PROJECT	SHEET	TOTAL
	SOUTH DAKOTA	0009-271 & 0009-271	7	40
I				

## TEMPORARY PAVEMENT MARKING

Cost for temporary pavement marking for stop bars and individual lane closure tapers that must remain overnight will be incidental to the contract unit price per site for Traffic Control for Pavement Repair.

Temporary payement marking on centerline will consist of temporary flexible vertical markers (tabs) and will be used as depicted on Standard Plate 634.25 when the stop condition must remain in place during nighttime hours 9:00PM to 6:00AM.

Temporary pavement marking for stop bars will consist of 4" Temporary Pavement Marking Tape Type I. Placement of each 24" white stop bar may be accomplished by placing six pieces of 4" x 12' tape adjacent to one another. Each workspace requires two stop bars which is an equivalent of approximately 144' of 4" tape.

#### CONTACTOR FURNISHED PORTABLE CHANGEABLE MESSAGE SIGN

One week prior to starting work affecting the traveling public, portable changeable message signs (PCMS) will be installed at locations detailed in the plans to notify drivers of the upcoming construction. The Contractor will program the portable changeable message signs with the following message:

ROAD WORK STARTS (Date)

ONE LANE AHEAD

REDUCE SPEED

When work begins that will affect traffic patterns, the Contractor will re-program the PCMS with the messages as detailed in the plans.

#### LANE CLOSURES

Interstate lane closures shorter than 5 miles will be used if 5 miles is greater than the length of work that can be accomplished in one day's production. More than one lane closure may be permitted; however, there will be a minimum of a threemile section between lane closures, excluding the tapers.

Interstate lane closures will be removed when work will not be occurring for a period of 3 or more calendar days. Activities that do not involve workers being present, such as curing time for concrete, constitute work. Lane closures will not be set up on a Friday if no work will be occurring on Saturday or Sunday. In these cases, the lane closure will be installed on Monday.

# See Addendum 2

#### MAINTENANCE OF TRAFFIC – PCC PAVEMENT REPAIR

Quantities for Traffic Control for Pavement Repair per site have been included for the on-call pavement repair PCN i77w. A site will be measured for each traffic control lane closure set-up necessary for pavement repairs. A site will not be measured for payment for modifications to lane closures such as switching lanes or changing the length. All specifications associated with the item Traffic Control Miscellaneous as per section 634 will apply to Traffic Control for Pavement Repair. All costs included in the item Traffic Control Miscellaneous as per section 634 will be included in the cost for Traffic Control for Pavement Repair per each site.

Additional Type 3 Barricades will be installed facing traffic within the closed lane at a spacing of 1/4 mile. At intersecting roadways, two additional Type 3 Barricades will be used to block the entire closed lane and shoulder.

Each mainline concrete repair location from which the in-place concrete has been removed will be marked with a minimum of two reflectorized drums.

Construction workspaces on undivided roadways will be limited to 300 feet in Length for lane closures using stop signs. Drivers in two-way traffic workspaces must be able to see approaching traffic through and beyond the work zone. If traffic cannot see approaching vehicles, 24-hour flagging will be required.

Construction workspaces in urban areas will be limited to 3 blocks in length. The minimum distance between workspaces will be 3 blocks.

When work is in progress within an intersection, Flaggers will be required to direct traffic.

Holes adjacent to centerline in the lane open to traffic created during removal and replacement of PCC Pavement Repair areas will be filled with gravel cushion material and cold-mix asphalt concrete prior to opening the lane to traffic. Gravel cushion material and cold-mix asphalt concrete will be furnished by the Contractor.

Holes in the asphalt concrete shoulders created during removal and replacement of PCC Pavement Repair areas will be filled with gravel cushion material and hotmix asphalt concrete (to match the shoulder surfacing) prior to opening the lane to traffic. Hot-mix asphalt concrete will be furnished by the Contractor at no additional cost to the State.

Cost for furnishing, hauling and placing asphalt concrete will be incidental to the contract unit price per square yard for Nonreinforced PCC Pavement Repair.

Routing traffic onto the mainline shoulders during any phase of the construction will not be allowed.

Damage to the shoulders, median or ditch due to the Contractor's operations will be repaired by the Contractor, to the satisfaction of the Engineer, at no expense to the State. This includes the apparent routing of traffic onto these shoulders around the work zones.

Traffic approaching the project from intersecting roadways, streets and approaches must be adequately accommodated. Major intersections or large commercial entrances may require additional signing, flaggers and other traffic control devices on a temporary basis until work activities pass these areas.

Cost for material (except signs, corresponding supports and hardware), labor, delivery, set up and maintenance of all necessary traffic control devices for each

Specification.



#### PORTABLE TEMPORARY TRAFFIC CONTROL SIGNAL

The Contractor will furnish, install, operate, and maintain a portable temporary traffic control signal during construction phases as determined by the Engineer. There will be one controller and one slave unit per location.

The portable temporary traffic control signal will be set up to dwell in red. Detection will be video, microwave, or radar. The green time may be adjusted as needed. The initial timings for the construction sites are given below:

PHASING AND SEQUENCING										
INT SIGNAL HEADS	ERVAL	1	2	3	4	5	6	FLASH DISPLAY		
SD11 NB	φA	G	Y	R	R	R	R	R		
SD11 SB	φB	R	R	R	G	Y	R	R		
TIMINGS BASED ON N BETWEEN OPPOSING ST OF	TIMINGS BASED ON MAXIMUM 1000 FT* DISTANCE BETWEEN OPPOSING STOP LINES AND TRAFFIC SPEED OF 35 MPH.							FLASH TIME		
CYCLE LENGTH = 120 Seconds										
PHASES					_		_			
PHASES			φA			φB				
PHASES			φ A		•	φ B		FAILURE OR		
PHASES MOVEMENTS MIN. GREEN (SEC)			¢Α 		•	фВ  15		FAILURE OR EMERGENCY ONLY		
PHASES MOVEMENTS MIN. GREEN (SEC) EXTENSION GREEN (SE	EC)		¢ A → 15 5		•	φ B 15 5		FAILURE OR EMERGENCY ONLY		
PHASES MOVEMENTS MIN. GREEN (SEC) EXTENSION GREEN (SE MAX. GREEN(SEC)	EC)		♦ A 15 5 36		•	фВ 15 5 36		FAILURE OR EMERGENCY ONLY		
PHASES MOVEMENTS MIN. GREEN (SEC) EXTENSION GREEN (SE MAX. GREEN(SEC) YELLOW (SEC)	EC)		♦ A 15 5 36 4		•	фВ 15 5 36 4		FAILURE OR EMERGENCY ONLY		

#### Temporary Signal Timing Plans for PCN I6XU - SD11

\* - The timings may be adjusted if the length between the stop lines varies from the 1000 ft value used in calculations.

The all red times may be recalculated as follows:

All Red = t + 
$$\frac{V}{2a}$$
 +  $\frac{W + L}{V}$  - Y

Where W = The distance between stop lines (ft).

Green times may be adjusted accordingly, however the total cycle length shall not exceed 120 seconds.

The portable temporary traffic control signal will be set up to dwell in red.

Both signals must be maintained in line of sight.

#### PORTABLE TEMPORARY TRAFFIC CONTROL SIGNAL (CONTINUED)

The portable temporary traffic control signal will be set up to dwell in red. Detection will be video, microwave, or radar. The green time may be adjusted as needed. The Engineer will contact the Region Traffic Engineer one week prior to activation to obtain the appropriate signal timings.

All vehicle signal heads will have backplates with retroreflective border. The vehicle signal head backplates will have a factory applied 3-inch wide yellow retroreflective border. Sheeting for the border will be Type IX or Type XI in conformance with ASTM D4956.

Signal backplates will be polycarbonate, aluminum, or aluminum-composite. Minimum material thicknesses are:

Polycarbonate, 0.10-inch Aluminum, 0.06-inch Aluminum-Composite, 0.08-inch

Signal backplates will extend not less than 5 inches from the edge of the signal head at the top, bottom, and sides.

All traffic signal equipment and materials will meet the requirements of Sections 635 and 985 of the Specifications except the controller requirements.

All costs involved with constructing the portable temporary traffic control signal as specified above and on the plans, will be included in the contract unit price per unit for "Portable Temporary Traffic Control Signal".

STATE OF	PROJECT	SHEET	TOTAL
SOUTH DAKOTA	0009-271 & 0009-271	9	SHEETS 40
		Ű	10

# I6XU

# ITEMIZED LIST FOR TRAFFIC CONTROL SIGNS

		CONVENTIONAL ROAD					EXPRESSWAY / INTERSTATE			
SIGN CODE	SIGN DESCRIPTION	NUMBER	SIGN SIZE	SQFT PER SIGN	SQFT	NUMBER	SIGN SIZE	SQFT PER SIGN	SQFT	
R2-1	SPEED LIMIT 45		24" x 30"	5.0		2	36" x 48"	12.0	24.0	
R2-1	SPEED LIMIT 65		24" x 30"	5.0		3	36" x 48"	12.0	36.0	
R2-1	SPEED LIMIT 80		24" x 30"	5.0		1	36" x 48"	12.0	12.0	
R2-6aP	FINES DOUBLE (plaque)		24" x 18"	3.0		1	36" x 24"	6.0	6.0	
R10-6	STOP HERE ON RED	2	24" x 36"	6.0	12.0					
W1-3	REVERSE TURN (L or R)	2	48" x 48"	16.0	32.0		48" x 48"	16.0		
W1-4	REVERSE CURVE (L or R)		48" x 48"	16.0		1	48" x 48"	16.0	16.0	
W3-3	SIGNAL AHEAD (symbol)	2	48" x 48"	16.0	32.0		48" x 48"	16.0		
W3-5	SPEED REDUCTION AHEAD (45 MPH)		48" x 48"	16.0		1	48" x 48"	16.0	16.0	
W3-5	SPEED REDUCTION AHEAD (65 MPH)		48" x 48"	16.0		2	48" x 48"	16.0	32.0	
W4-2	LEFT or RIGHT LANE ENDS (symbol)		48" x 48"	16.0		2	48" x 48"	16.0	32.0	
W13-1P	ADVISORY SPEED (plaque)	2	30" x 30"	6.3	12.6		30" x 30"	6.3		
W20-1	ROAD WORK AHEAD	2	48" x 48"	16.0	32.0	2	48" x 48"	16.0	32.0	
W20-4	ONE LANE ROAD AHEAD	2	48" x 48"	16.0	32.0		48" x 48"	16.0		
W20-5	LEFT or RIGHT LANE CLOSED AHEAD		48" x 48"	16.0		2	48" x 48"	16.0	32.0	
W20-7	FLAGGER (symbol)		48" x 48"	16.0		1	48" x 48"	16.0	16.0	
G20-2	END ROAD WORK	2	36" x 18"	4.5	9.0	1	48" x 24"	8.0	8.0	
		CONVENTIONAL ROAD TRAFFIC CONTROL SIGNS SQFT		161.6	EXPRES TRAFFIC	SWAY / INTE	RSTATE	262.0		

# 177W

# ITEMIZED LIST FOR TRAFFIC CONTROL SIGNS

			CONVENTIO	ONAL ROAD		E)	<b>(PRESSWAY</b>	/ INTERSTA	TE
SIGN CODE	SIGN DESCRIPTION	NUMBER	SIGN SIZE	SQFT PER SIGN	SQFT	NUMBER	SIGN SIZE	SQFT PER SIGN	SQFT
R1-1	STOP	2	30"	5.2	10.4		36"	7.5	
R2-1	SPEED LIMIT 45		24" x 30"	5.0		2	36" x 48"	12.0	24.0
R2-1	SPEED LIMIT 65		24" x 30"	5.0		3	36" x 48"	12.0	36.0
R2-1	SPEED LIMIT 80		24" x 30"	5.0		1	36" x 48"	12.0	12.0
R2-6aP	FINES DOUBLE (plaque)		24" x 18"	3.0		1	36" x 24"	6.0	6.0
W1-3	REVERSE TURN (L or R)	2	48" x 48"	16.0	32.0		48" x 48"	16.0	
W1-4	REVERSE CURVE (L or R)		48" x 48"	16.0		1	48" x 48"	16.0	16.0
W3-1	STOP AHEAD (symbol)	2	48" x 48"	16.0	32.0		48" x 48"	16.0	
W3-5	SPEED REDUCTION AHEAD (45 MPH)		48" x 48"	16.0		1	48" x 48"	16.0	16.0
W3-5	SPEED REDUCTION AHEAD (65 MPH)		48" x 48"	16.0		2	48" x 48"	16.0	32.0
W4-2	LEFT or RIGHT LANE ENDS (symbol)		48" x 48"	16.0		2	48" x 48"	16.0	32.0
W13-1P	ADVISORY SPEED (plaque)	2	30" x 30"	6.3	12.6		30" x 30"	6.3	
W20-1	ROAD WORK AHEAD	2	48" x 48"	16.0	32.0	2	48" x 48"	16.0	32.0
W20-4	ONE LANE ROAD AHEAD	2	48" x 48"	16.0	32.0		48" x 48"	16.0	
W20-5	LEFT or RIGHT LANE CLOSED AHEAD		48" x 48"	16.0		2	48" x 48"	16.0	32.0
W20-7	FLAGGER (symbol)		48" x 48"	16.0		1	48" x 48"	16.0	16.0
G20-2	END ROAD WORK	2	36" x 18"	4.5	9.0	1	48" x 24"	8.0	8.0
		CONVENTIONAL ROAD TRAFFIC CONTROL SIGNS SOFT			160.0	EXPRES TRAFFIC	SWAY / INTE CONTROL S	ERSTATE IGNS SQFT	262.0

STATE OF	PROJECT	SHEET	TOTAL
SOUTH DAKOTA	0009-271 & 0009-271	10	40

## TABLE FOR NRC PAVEMENT REPAIR ON SD 11 - PCN I6XU SB & NB

							INSER	T STEEL BAR		
							PUCPA	VENENT (NRC	JP)	
	SB		NB						INSERI	
	DRIVIN	NG	DRIVI	NG _		NEW			STEEL	
	LANE	=	LAN	E		JOINT	No. 9 x 18"	No. 5 x 24"	BAR IN	
					NRCP	CON-	DEFORMED	DEFORMED	NRCP	DOWEL
	L	w	L	w	REPAIR	FIG.	TIE BARS	TIE BARS	TOTAL	BAR
MRM DISP	Ft	Ft	Ft	Ft	SqYds	(NRCP)	Each	Each	Each	Each
79.00 0.530			4	4	1.8	R	4	4	8	4
79.00 0.527	4	4			1.8	R	4	2	6	4
79.00 0.519	4	4			1.8	R	4	2	6	4
79.00 0.504	5	4			2.2	R	4	2	6	4
79.00 0.490	4	4			1.8	R	4	2	6	4
79.00 0.487	4	4			1.8	R	4	2	6	4
79.00 0.479	4	4			1.8	R	4	2	6	4
79.00 0.476	4	4			1.8	R	4	2	6	4
79.00 0.468			4	8	3.6	R	10	4	14	8
79.00 0.442	4	4			1.8	R	4	2	6	4
79.00 0.439			4	6	2.7	R	8	4	12	6
79.00 0.428	4	6	4	4	1.8	R	4	4	8	4
79.00 0.303	4	16			2.1	R D	0	2	10	12
79.00 0.377	4	10			1.1	R	10	2	10	12
79.00 0.343	4	10			8.0	R	12	2	14	10
79.00 0.291	0	12	6	20	13.3	R	16	<u> </u>	20	12
79.00 0.257	4	6	•	20	2.7	R	8	2	10	6
79.00 0.166	6	6			4.0	R	8	2	10	6
79.00 0.158	4	6			2.7	R	8	2	10	6
78.00 0.621	4	4			1.8	R	4	2	6	4
78.00 0.561			4	12	5.3	R	16	4	20	12
78.00 0.558	4	14	4	14	12.4	R	32		32	24
78.00 0.524	6	14	6	14	18.7	R	32		32	24
78.00 0.510	4	14	4	14	12.4	R	32		32	24
78.00 0.499	4	12			5.3	R	16	2	18	12
78.00 0.496			4	14	6.2	R	16	2	18	12
78.00 0.482	4	12	4	4	7.1	R	20	6	26	16
78.00 0.476	4	6			2.7	R	8	2	10	6
78.00 0.465	4	14	4	14	12.4	R	32	0	32	24
78.00 0.450	4	12			5.3	R	10	2	18	12
78.00 0.433	4	12	6	6	0.3	R	24	6	30	12
78.00 0.391	4	12	0	0	6.2	R	16	2	18	10
78.00 0.388	4	12			5.3	R	16	2	18	12
78.00 0.382	4	12	4	4	7.1	R	20	6	26	16
78.00 0.377	4	6			2.7	R	8	2	10	6
78.00 0.368	4	4	4	4	3.6	R	8	6	14	8
78.00 0.365	6	14	6	14	18.7	R	32		32	24
78.00 0.348	4	4			1.8	R	4	2	6	4
78.00 0.311	4	8			3.6	R	10	2	12	8
78.00 0.283	4	4			1.8	R	4	2	6	4
78.00 0.257	4	4			1.8	R	4	2	6	4
78.00 0.212	4	4			1.8	R	4	2	6	4
78.00 0.203	4	4			1.8	R	4	2	6	4
78.00 0.121	4	4			1.8	ĸ	4	2	6	4
78.00 0.000	4	4			1.0	ĸ	4	2	0	4
78.00 0.090	4	6			2.2	R	0 8	2	10	6
77 00 0.073	4	6			2.1	R	8	2	10	6
77.00 0.973	4	6			27	R	8	2	10	6
77.00 0.956	4	12	4	6	8.0	R	24	6	30	18
77.00 0.931	4	10			4.4	R	12	2	14	10
77.00 0.925	4	12			5.3	R	16	2	18	12
77.00 0.902	4	6			2.7	R	8	2	10	6
77.00 0.874	4	4			1.8	R	4	2	6	4

									STATE OF		PROJECT		SHEET	Γ
									SOUTH DAKOTA	0009-	271 & 000	9-271	11	Γ
		TABLE	EFOR	NRC P	AVEM	ENT REP	AIR ON S	SD 11 - PC	N 16XL	J SB & I	NB			
								INS	ERT STI		IN CD)			
		Se Drivi Lan	NG E	NB DRIVII LAN	NG E	NRCP	NEW JOINT CON-	No. 9 x 18 DEFORM	8" No. ED DEF	5 x 24" ORMED	INSERT STEEL BAR IN NRCP	DOWEL		
		L	W	L	W	REPAIR	FIG.	TIE BAR	S TIE	BARS	TOTAL	BAR		
MRM	DISP	Ft	Ft	Ft	Ft	SqYds	(NRCP)	Each	E	Each	Each	Each		
77.00	0.860	4	12			5.3	R	16		2	18	12		
77.00	0.848	4	6	4	4	4.4	R	12		6	18	10	_	
77.00	0.043	4	4			1.0	rt P	4		2	18	4	_	
77.00	0.828	4	4			1.8	R	4		2	6	4	-	
77.00	0.823	4	4			1.0	R	4		2	6	4	-	
77.00	0.820	4	6			2.7	R	8		2	10	6		
77.00	0.808	4	6			2.7	R	8		2	10	6		
77.00	0.806	4	14			6.2	R	16		2	18	12		
77.00	0.803	4	6			2.7	R	8		2	10	6	-	
77.00	0.797	4	4			1.8	R	4		2	6	4	-	
77.00	0.786	4	4			1.8	R	4		2	6	4		
77.00	0.783	4	14			6.2	R	16		2	18	12		
77.00	0.777	4	10			4.4	R	12		2	14	10		
77.00	0.766	4	8			3.6	R	10		2	12	8		
77.00	0.760	4	4			1.8	R	4		2	6	4	_	
77.00	0.757	4	4			1.8	R	4		2	6	4		
77.00	0.752	4	4			1.8	R	4		2	6	4	-	
77.00	0.740	4	12			2.1	R	16		2	10	10	-	
77.00	0.740	4	12	1	1	<u> </u>	R	10		2	8	12	-	
77.00	0.715			6	8	5.3	R	10		4	14	4		
77.00	0.709	4	4	U	0	1.8	R	4		2	6	4	-	
77.00	0.575	4	14	4	4	8.0	R	20		2	22	16		
77.00	0.553	4	14	4	4	8.0	R	20		2	22	16		
77.00	0.479			4	14	6.2	R	16		2	18	12		
77.00	0.465	4	12			5.3	R	16		2	18	12		
77.00	0.448	4	8	4	4	5.3	R	14		6	20	12		
77.00	0.436	4	12			5.3	R	16		2	18	12		
77.00	0.254	4	4			1.8	R	4		2	6	4	_	
//.00	0.243	4	6			2.7	R	8		2	10	6		
TALS:						384.4		956	2	208	1164	761	-	
JANTITI	AL ES:					40.0		50		10	60	40	=	
						424.4		1006	2	218	1224	801		

NRC PAVEMENT REPAIR AREA TYPESW = Two Working Joints (Use only if repair is full roadway width and uniform length (across <u>all</u> lanes))

T = Two Tied Joints

B = One Working & One Tied Joint

R = Two Tied Joints with Original Joint Restored with Dowel Bar Assembly

### TABLE FOR CRC PAVEMENT REPAIR ON I29 - PCN I6XU SB

#### TABLE FOR CRC PAVEMENT REPAIR ON 129 - PCN 16XU SB

			SE PASS LAN	ing IE	SB DRIVI LAN	NG E	CRCP
			1.1	w		w	REPAIR
MRM	DISP	DMI	Ft	Ft	Ft	Ft	SaYds
65.00	0.226	65 226			4	4	1.8
65.00	0.209	65.209			4	4	1.8
65.00	0.206	65.206			4	4	1.8
65.00	0.195	65.195			4	4	1.8
65.00	0.186	65.186			4	4	1.8
65.00	0.166	65.166			4	4	1.8
65.00	0.164	65.164			4	4	1.8
65.00	0.076	65.076			4	4	1.8
65.00	0.050	65.050	4	4			1.8
64.00	0.752	64.752			4	4	1.8
64.00	0.732	64.732			4	4	1.8
64.00	0.706	64.706			10	14	15.6
64.00	0.698	64.698	4	4			1.8
64.00	0.686	64.686			4	4	1.8
64.00	0.666	64.666			4	4	1.8
64.00	0.664	64.664			4	4	1.8
64.00	0.658	64.658			6	14	9.3
64.00	0.544	64.544			4	4	1.8
64.00	0.507	64.507			4	4	1.8
64.00	0.354	64.354			20	4	8.9
64.00	0.252	64.252	4	4			1.8
64.00	0.127	64.127			4	4	1.8
64.00	0.104	64.104			4	4	1.8
64.00	0.098	64.098			4	4	1.8
64.00	0.095	64.095			4	4	1.8
64.00	0.090	64.090			15	4	6.7
64.00	0.081	64.081			4	4	1.8
64.00	0.078	64.078			4	4	1.8
64.00	0.070	64.070			4	4	1.8
63.00	0.854	63.854			4	4	1.8
TOTALS:							87.3
	IAL						10.0
GRAND TOTALS							97.3

			SE PASS LAN	ing IE	SB DRIVING LANE		
							CRCP
			L	W	L	W	REPAIR
MRM	DISP	DMI	Ft	Ft	Ft	Ft	SqYds
67.00	0.952	67.952			4	4	1.8
67.00	0.946	67.946			4	4	1.8
67.00	0.940	67.940			4	4	1.8
67.00	0.932	67.932			4	4	1.8
67.00	0.929	67.929			4	4	1.8
67.00	0.926	67.926			4	4	1.8
67.00	0.923	67.923			4	4	1.8
67.00	0.912	67.912			4	4	1.8
67.00	0.909	67.909			4	4	1.8
67.00	0.906	67.906			4	4	1.8
67.00	0.903	67.903			4	4	1.8
67.00	0.901	67.901			4	4	1.8
67.00	0.898	67.898			4	4	1.8
67.00	0.895	67.895			4	4	1.8
67.00	0.892	67.892			4	4	1.8
67.00	0.878	67.878			4	4	1.8
67.00	0.872	67.872			4	4	1.8
67.00	0.858	67.858			4	4	1.8
67.00	0.582	67.582	4	-	4	4	1.8
67.00	0.386	67.386	4	4			1.8
67.00	0.304	67.304	4	4			1.8
67.00	0.031	67.031	4	4			1.8
66.00	0.906	66.906	4	4	4	4	1.8
66.00	0.895	00.895	4	4	4	4	1.8
66.00	0.010	00.010	4	4	4	4	1.0
66.00	0.013	00.013	4	4	4	4	1.0
66.00	0.790	00.790	4	4	4	4	1.0
66.00	0.770	66 526	1	1	4	4	1.0
66.00	0.520	66 514	4	4	1	1	1.0
66.00	0.014	66 496			4	4	1.0
66.00	0.400	66 103			4	4	1.0
66.00	0.193	66 190			4	4	1.0
66.00	0.130	66 173				4	1.0
66.00	0.173	66 162				4	1.0
66.00	0.102	66 088			4	4	1.0
65.00	0.000	65.878			4	4	1.0
65.00	0.872	65 872			4	4	1.8
65.00	0.869	65 869			4	4	1.8
65.00	0.866	65 866	4	4	•		1.8
65.00	0.778	65.778	4	4			1.8
65.00	0.699	65.699			4	4	1.8
65.00	0.693	65.693			4	4	1.8
65.00	0.676	65.676			4	4	1.8
65.00	0.517	65.517			4	4	1.8
65.00	0.401	65.401			4	4	1.8
TOTALS:							82.8
	IAL ES:						10.0
GRAND							
TOTALS							92.8

STATE OF	PROJECT	SHEET	TOTAL
SOUTH DAKOTA	0009-271 & 0009-271	12	40

									PROJECT		τοται
								STATE OF SOUTH	FROJECT	SHEF	T SHEETS
								DAKOTA	0009-271 & 000	)9-271   13	40 ا
										I	
			TABLE FO	R CRC PAVEMEN	FREPAIR ON 129 - PCI	I I6XU SB					
			R	EINFORCING STEEL (C	RCP) FOR SB DRIVING LA	NE			INSERT STEEL	BAR IN	
			(STEEL FOR	CRCP IS NOT A BID I	TEM - ACTUAL STEEL QUAN	TITIES WILL			PCC PAVEMENT		
						R AREAS)					
			VARTEO	E TO ECOATION AND		(AREAO)			INSERT		
	No. 6 Longituding	al Bara Lan	No. 6 Longitudinal Bars to	bo Lon	No. 6 Longitudinal Bars to	ho lon	No. 4 Transverse Bars	Now		INCEPT	
	to be len aplic	ai Dais Lap	apliced together between of	verv Len Steager	apliced together between	vonu Lon Staggor	to be lon onlined with	Tropo Doinforoing			
	to be tap split	Lap Stagger	spliced together between e	very Lap Stagger	spliced logeliner between a	ber Splice ?	No. 5 x 24" hore	Par Steel			
		bars Splice &		bai Splice &					DARS HE DAR	S IUTAL	
MRM DISP DMI	# bars @ length	Length Length Cutoff	# bars @ length Le	ength Length Cutoff	# bars @ length L	ength Length Cutoff	# bars @ length Length	Spacing Lbs	Each Each	Each	
65.00 0.226 65.226											
65.00 0.209 65.209											
65.00 0.206 65.206											
65.00 0.195 65.195											
65.00 0.186 65.186											
65.00 0.166 65.166											
65.00 0.164 65.164											
64.00 0.752 64.752											
64.00 0.732 64.732											
64.00 0.706 64.706	26 hars @ 106" =	229 67' 30" 6"	13 bars @ 82" = 8	88.83' 30" 7"	13 bars @ 86" =	93 17' 30" 7"	4 bars @ 162" = 54.00'	2' 654 400	26 4	30	
64 00 0 686 64 686	20 5015 @ 100	220.01 00 0		0.00 00 1		00 1	4 5013 (2 102 04.00	2 004.400	20 4	00	
64.00 0.666 64.666											
64.00 0.664 64.664											
64.00 0.658 64.658	26 bars @ 62" =	134.33' 19" -	13 bars @ 55" = 5	59.58' 19" -	13 bars @ 55" =	59.58' 19" -	3 bars @ 162" = 40.50'	2' 407.796	26 3	29	
64.00 0.544 64.544	Ŭ		<u> </u>		Ŭ		Ŭ				
64.00 0.507 64.507											
64.00 0.354 64.354											
64.00 0.127 64.127											
64.00 0.104 64.104											
64.00 0.098 64.098											
64.00 0.095 64.095											
64.00 0.090 64.090											
05.00 0.054 05.854											
TOTALS:	52 bars	364'	26 bars	148'	26 bars	153'	7 bars 95'	1062 Lbs	52 7	59	
QUANTITIES	10 bars	40'	_	10'		20'	- 10'	110 l be	10 -	10	
CRAND	10 0010					-*	10	110 203	••		
	60 harra	40.41		4501	00 hours	4701	7 hore (07)	447011			
IUIALO	62 Dars	404	20 Dars	061	26 Dars	1/3	<i>i</i> bars 105 <sup>°</sup>	11/2 LDS	62 /	63	

### TABLE FOR CRC PAVEMENT REPAIR ON 129 - PCN 16XU SB

			SB DRIVI LAN	NG E	CRCP
			L	W	REPAIR
MRM	DISP	DMI	Ft	Ft	SqYds
62.00	0.449	62.449	4.5	14	7.0
62.00	0.446	62.446	4	4	1.8
62.00	0.227	62.227	4	4	1.8
TOTALS:					10.6

#### TABLE FOR CRC PAVEMENT REPAIR ON I29 - PCN I6XU SB

								REINFO	ORCING	STEEL (C	RCP) FOR SB DRIVIN	G LANE				
							(STEE	L FOR CRC	P IS NO	T A BID IT	EM - ACTUAL STEEL (	QUANTITIES	S WILL			
							` VAI	RY DUE TO	LOCAT	ION AND S	SIZE OF INDIVIDUAL R	EPAIR ARE	AS)			
													,			
			No. 6 Longitudinal	Bars		Lap	No. 6 Longitudinal E	ars to be		Lap	No. 6 Longitudinal B	ars to be		Lap	No. 4 Transvers	e Bars
			to be lap splice	ed	Lap	Stagger	spliced together betw	een every	Lap	Stagger	spliced together betw	een every	Lap	Stagger	to be lap splice	d with
			with existing ba	ars	Splice	&	other existing longit	udinal bar	Splice	&	other existing longitu	udinal bar	Splice	&	No. 5 x 24" b	ars
MRM	DISP	рмі	# bars @ length	Length	l enath	Cutoff	# bars @ length	Length	l enath	Cutoff	# hars @ length	l enath	l enath	Cutoff	# bars @ length	Lenc
				Longar	Longar	outon		Longar	Longa	outon		Longar	Longar	outon		Long
62.00	0.449	62.449	26 bars @ 46" =	99.67'	14"	-	13 bars @ 44" =	47.67'	14"	-	13 bars @ 44" =	47.67'	14"	-	2 bars @ 162" =	27.0
TOTALS:			26 bars	100'			13 bars	48'			13 bars	48'			2 bars	2
ADDITION																
QUANTIT	IES:		10 bars	20'			-	10'			-	10'			-	1
GRAND																
TOTALS			36 bars	120'			13 bars	58'			13 bars	58'			2 bars	3

		STATE OF		SHEET	TOTAL SHEETS		
		SOUTH DAKOTA	0009-27	71	14	40	
						••	
			INSE				
			PCCI			)	
				DRIVING LA			
	New		No 6	INSERT	INS	FRT	
,	Trans	Reinforcing	LONG	No. 5 x 24"	B		
	Bar	Steel	BARS	TIE BARS	то	TAL	
ength	Spacing	Lbs	Each	Each	Ea	ch	
27.00'	2'	310.941	26	0	2	26	
					_		
27'		311 Lbs	26	-	2	26	
10'		60 Lbs	10	-	1	0	
37'		371 Lbs	36	-	3	86	

# 24', 36' & 48' \* CRC PAVEMENT - IN PLACE

\* And other miscellaneous widths (may include CRC shoulders)



MITCHELL REGION INTERSTATE CRC PAVEMENT KEY & DIMENSIONS	Underlying Plans	CRC Depth	CRC Width	Lon Size	gitudinal Steel Spacing
Location	PCN	т	W	С	E
I90E/W MRM 263.53 +0.010 to 265.00 +0.428	3781	9.5"	24'/36'	6	6½"
I90E MRM 251.09 +0.506 to 259.52 & MRM 259.60 to 259.90	3028	10"	24'	6	6½"
I90W MRM 251.09 +0.509 to 259.52 & MRM 259.60 to 259.88	4766	10"	24'	6	6½"
I229N/S Approach Slabs for 57th St Tunnel (Double Matte Steel)	0549	10"	52'	4&8	18"&6"
I229N/S Approach Pavement and Pavement over 57th St Tunnel	0549	10"	52'	7	6"
I229N/S MRM 2.08 to 5.32 +0.067 & MRM 5.68 +0.090 to 8.28 +0.687	1231	10.5"	24'/36'	6	6"
I29S MRM 83.00 +0.790 to 84.39	3785	10.5"	40'/42'	6	6"
I29N/S MRM 73.38 to MRM 73.38 +0.634	1948	11"	34'/36'	7	7"
I29N MRM 79.26 +0.246 to 80.29 +0.246 & I29S MRM 79.26 +0.246 to 80.29 +0.251	A443 & 02P3	12"	36'/48'	7	6½"
I29N MRM 80.29 +0.246 to 83.00 +0.790 & I29S MRM 80.29 +0.251 to 83.00 +0.790	1177 & 02P3	12"	36'/48'	7	6½"

	STATE OF		PROJEC		SHEET	SHEETS
	SOUTH DAKOTA	0009	-271 & 0	009-271	15	40
	Plotting	) Date: 08	/21/2019			
		Dote: 08	)			
		P	)			
Tra	nsverse Steel Spacing	F	Perime	ter Ba	r	i
		$\frown$			$\frown$	
	$(\mathbf{F})$	$(\mathbf{K})$	(M)		$(\mathbf{P})$	
6	48"	<b>3<sup>3</sup>⁄</b> 4"	6½"	6½"	6½"	
6	48"	3 <sup>3</sup> / <sub>4</sub> "	6½"	61⁄2"	6½"	
4	48"	3¾"	6½"	61⁄2"	61/2"	
 4&6	18"&12"	3"	6"	6"	6"	
 4	16"	3"	6"	6"	6"	
4	48"	4"	5"	5"	5"	
4	48 <sup>°</sup>	4" 6"	5	5 <sup></sup>	5	
4 1	30 26"	0 /''	0	/ 61/"	0 6"	
 4 1	26"	4 ∕/"	0 6"	072 61/."	0 6"	
4	30	4	0	U/2	0	



	I29S MRM 27.00 + 0.058 to 37.32 +0.138 & I90W MRM 353.07 +0.006 to 362.00 +0.045	5886 & 5359	10"	26'	6	6½"
NT 15	I90E/W MRM 334.54 +0.004 to 353.07 +0.006	5363, 4431,	10.5"	26'	6	6"
RMI 1		5365 & 5364				
- TF	I29S MRM 84.39 to 84.00 +0.910	3785	10.5"	26'	6	6"
FROM	I29N MRM 61.00 +0.888 to 72.00 +0.866 & I29S MRM 61.00 +0.888 to 62.00 +0.443	5360	11"	26'	6	6"
TED	I29S MRM 62.00 +0.443 to MRM 72.00 +0.875	5367	11"	26'	6	6½"
PLOT	I29N MRM 72.00 +0.866 to 73.38 & I29S MRM 72.00 +0.875 to 73.38	1948	11"	26'	7	7"
	I29N MRM 4.35 +0.463 to 17.00 +0.406 & I29S MRM 37.32 +0.138 to 46.31 +0.600	6176 & 6181	11.5"	26'	5	41/2"

		STATE O SOUTH	F	PROJEC1	Γ	SHEET	TOTAL SHEETS	
		DAKOTA	0009	-271 & 0	009-271	16	40	
			0009	N N N	<u>009-271</u>	16	40	DI OT NAME 2
				P V				VEDE DEDATOVEDE EVICTINE DEN
Tra Size	nsve Stee Spa	erse I acing	F	<sup>p</sup> erime Spa	ter Ba	r		
L	(	F	K	M	N	P		
4		36"	4"	8"	8"	8"		

3"

**3**<sup>3</sup>⁄4"

**3**<sup>3</sup>/<sub>4</sub>"

3"

4"

4"

**3**<sup>3</sup>⁄4"

6"

3"

42"

42"

48"

42"

48"

48"

48"

36"

42"

4

4

4

4

4

4

4

4

4

6<sup>3</sup>⁄4"

61⁄2"

61⁄2"

6"

5"

5"

61⁄2"

6"

3"

5¾"

4½"

4½"

6"

5"

5"

4½"

9"

41⁄2"

6¼"

6½"

6½"

6"

5"

5"

6½"

7"

4½"

# **CRC PAVEMENT REPAIR (FULL LANE WIDTH) - TYPICAL**



STATE OF	PROJECT	SHEET	TOTAL SHEETS
SOUTH DAKOTA	0009-271 & 0009-271	17	40
Plotting I	)ate: 08/21/2019		



![](_page_18_Figure_0.jpeg)

Place No. (C) Longitudinal Deformed Tie Bar <u>UUU</u> (Place bars into drilled holes in existing concrete on both sides of the repair area and tie the bars to each other and to No.(L)Transverse Bars)

Deformed Tie Bar

Bars)

(Tie to No. (C) Longitudinal

Note: All lapped bars will have a minimum of two ties per lap.

![](_page_18_Picture_4.jpeg)

**Remove Concrete** Retain Reinforcing Steel

Remove Concrete **Remove Reinforcing Steel** 

# **CRC PAVEMENT REPAIR (PARTIAL LANE WIDTH) - TYPICAL**

![](_page_19_Figure_1.jpeg)

Under

lying

PCN

CRC

Т

STATE OF	PROJECT	SHEET	TOTAL
SOUTH DAKOTA	0009-271 & 0009-271	20	40
Disting			

- Retain No. (L) Transverse
- Transverse Saw Cut Partial Depth (Above Steel)

Place No. C Longitudinal Deformed Tie Bars (Lap splice to In Place No. (C)Longitudinal Bars).

For Repair Area Length L = 8' or more - every other in place No. (C) Longitudinal Deformed Tie Bar will be cut off and lap splices will be staggered similarly to the details for CRC PAVEMENT **REPAIR AREA (FULL LANE** 

![](_page_19_Picture_9.jpeg)

**Remove Concrete** Retain Reinforcing Steel

![](_page_19_Picture_10.jpeg)

Remove Concrete Remove Reinforcing Steel

![](_page_20_Figure_1.jpeg)

![](_page_21_Figure_1.jpeg)

However, a Repair Width W exceeding 4' might be used when doing a small repair adjacent to an existing repair, in order to match the width of the existing repair.

![](_page_21_Figure_3.jpeg)

![](_page_21_Figure_5.jpeg)

T = Existing pavement thickness.

# **CRC REPAIR AREA KEY**

![](_page_21_Picture_10.jpeg)

Remove Concrete Retain Reinforcing Steel

 $T_N = New pavement thickness.$ 

# **CRC PAVEMENT REPAIR - REINFORCING STEEL DETAILS**

# TRANSVERSE SECTION SHOWING STEEL PLACEMENT

![](_page_22_Figure_2.jpeg)

![](_page_22_Figure_4.jpeg)

![](_page_22_Figure_5.jpeg)

![](_page_22_Figure_6.jpeg)

See DRILLED IN TIE BAR DETAIL (In Longitudinal Joint)

# **CRC PAVEMENT IN PLACE & CRC PAVEMENT REPAIR KEY & DIMENSIONS**

	Under					Long	gitudinal	Saw	Trar	nsverse		Long	gitudina	l Bar C	ount		La	p Sp
	lying	CRC	CRC	Clea	rance		Steel	Cut	5	Steel		(full	lane w	idth rep	bair)		(for	Rep
	Plans	Depth	Width	Тор	Bottom	Size	Spacing	Depth	Size	Spacing	12'	Wide S	lab	14'	Wide S	lab	L<4.5'	L=4
Location	PCN	Т	W	A	B	C	E	D		F	G	H	H <sub>2</sub>	G	H	H2	$(\mathbf{b})$	

![](_page_23_Figure_1.jpeg)

# ANY SINGLE LANE ROADWAY (RAMPS, ETC.)

![](_page_24_Figure_1.jpeg)

# TYPICAL REPAIR AREAS

![](_page_25_Figure_1.jpeg)

### KEY:

PCC Pavement Repair Area

### PCC PAVEMENT REPAIR AREA TYPES:

W Two Working Joints (Use only if repair is full roadway width and uniform length (across all lanes))

(T) Two Tied Joints

(B) One Working & One Tied Joint

R Two Tied Joints with Original Joint Restored with <sup>/</sup> Dowel Bar Assembly

#### Steel Bars for Transverse Joints

- Pavement Thickness >= 10.5" \_\_\_\_\_ Drilled in 1½" x 18" epoxy coated plain round dowel bars spaced 18" center to center.
- Drilled in No. 11 x 18" epoxy coated deformed tie bars spaced 18" center to center.

- Pavement Thickness >= 8.5" and < 10.5" \_\_\_\_\_ Drilled in 1¼" x 18" epoxy coated plain round dowel bars spaced 18" center to center.
- Drilled in No. 9 x 18" epoxy coated deformed tie bars spaced 18" center to center.

- Pavement Thickness < 8.5" \_\_\_\_ Drilled in 1" x 18" epoxy coated plain round dowel bars spaced 18" center to center.
- Drilled in No. 8 x 18" epoxy coated deformed tie bars spaced 18" center to center.
- Dowel Bar Assembly

# Steel Bars for Longitudinal Joints

NOTES: Saw around repair areas full depth for removal.

No. 5 x 30" epoxy coated deformed tie bars. Sawed Joint - spaced 48" center to center. Construction Joint - spaced 48" center to center.

No. 5 x 24" epoxy coated deformed tie bars. Drilled In - spaced 30" center to center.

(1) Where possible, transverse joints will be constructed/maintained full roadway width.

(2) Edges of repair areas will be formed to match the width of the existing concrete pavement.

(3) Need for bars in small repair areas on/near the shoulder to be determined on a case-by-case basis, on construction by the Engineer.

# **TYPICAL REPAIR AREAS**

![](_page_26_Figure_1.jpeg)

# KEY:

PCC Pavement Repair Area

# PCC PAVEMENT REPAIR AREA TYPES:

- W Two Working Joints (Use only if repair is full roadway width and uniform length (across all lanes))
- (T) Two Tied Joints
- (B) One Working & One Tied Joint
- R Two Tied Joints with Original Joint Restored with <sup>/</sup> Dowel Bar Assembly

# Longitudinal Keyway Joints Without Bars

 $-\kappa$  – Where a repair area intersects an existing longitudinal keyway joint without tie bars, the newly constructed joint should also be a keyway without tie bars.

# Steel Bars for Transverse Joints

- Pavement Thickness >= 10.5" \_\_\_\_\_ Drilled in 1½" x 18" epoxy coated plain round dowel bars spaced 18" center to center.
- Drilled in No. 11 x 18" epoxy coated deformed tie bars spaced 18" center to center.
- Pavement Thickness >= 8.5" and < 10.5" \_\_\_\_\_ Drilled in 1¼" x 18" epoxy coated plain round dowel bars spaced 18" center to center.
- Drilled in No. 9 x 18" epoxy coated deformed tie bars spaced 18" center to center.

- Pavement Thickness < 8.5" \_\_\_\_\_ Drilled in 1" x 18" epoxy coated plain round dowel bars spaced 18" center to center.
- Drilled in No. 8 x 18" epoxy coated deformed tie bars spaced 18" center to center.
- Dowel Bar Assembly

# Steel Bars for Longitudinal Joints

No. 5 x 30" epoxy coated deformed tie bars. Sawed Joint - spaced 48" center to center. Construction Joint - spaced 48" center to center.

No. 5 x 24" epoxy coated deformed tie bars. Drilled In - spaced 30" center to center.

NOTES: Saw around repair areas full depth for removal.

(1) Where possible, transverse joints will be constructed/maintained full roadway width.

(2) Edges of repair areas will be formed to match the width of the existing concrete pavement.

(3) Need for bars in small repair areas on/near the shoulder to be determined on a case-by-case basis, on construction by the Engineer.

# **TYPICAL REPAIR AREAS**

![](_page_27_Figure_1.jpeg)

# PLAIN ROUND DOWEL BAR INSERTION

![](_page_28_Figure_3.jpeg)

### LONGITUDINAL CONSTRUCTION JOINT WITH TIE BARS & KEYWAY

![](_page_29_Figure_3.jpeg)

### LONGITUDINAL SHOULDER CONSTRUCTION JOINT WITH TIE BARS & KEYWAY

![](_page_30_Figure_3.jpeg)

![](_page_31_Figure_1.jpeg)

\*\* The saw cut to control cracking will be a minimum of 1/4 the thickness of the pavement.

![](_page_32_Figure_0.jpeg)

![](_page_33_Figure_0.jpeg)

![](_page_33_Figure_1.jpeg)

![](_page_33_Figure_2.jpeg)

![](_page_34_Figure_0.jpeg)

![](_page_34_Figure_1.jpeg)

November 19, 2022

	S D D	RESEAL PCC PAVEMENT JOINT (SILICONE)	plate number 380.16
Published Date: 1st Qtr. 2023	0 T		Sheet I of I

STATE OF	PROJECT	SHEET	TOTAL	1
SOUTH DAKOTA	0009-271 & 0009-271	35	40	
Plotting	Date: 04/27/2023			1
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		S D D	LANE	CLOSURE V	VITH FLAGGER PROVIDED	PLATE NUMBER 634.23
The leng fit field c	oth of A may be adj onditions.	usted to		▼		January 22 2021
The buff so that the placed be curve to distance of stoppe	er space should be he two-way traffic to pefore a horizontal of provide adequate for the flagger and ed vehicles.	e extended aper is or vertical sight I queue				
Channel be used control ir required	izing devices and f at intersecting road ntersecting road tra	laggers will ds to iffic as	<del>∛</del>			OAD ORK JEAD
Channel along the area who escorting area.	lizing devices are n e centerline adjace en pilot cars are uti g traffic through the <u>z-oze</u> X80M QYON ONJ	ot required nt to work lized for e work				LANE DAD HEAD VIC
advance The cha or 42" co	warning signs. nnelizing devices wones.	vill be drums				XXX EET 16-2P
Flashing may be	y warning lights and used to call attentio	l/or flags on to the			Traffi	T
For tack when fla FRESH in advan	and/or flush seal o lggers are not being OIL sign (W21-2) v lice of the liquid asp	perations, g used, the vill be displayed bhalt areas.			(100' S hax.) c Taper	
The RO/ WORK s duration	AD WORK AHEAD signs may be omitte operations (1 hour	and the END R ed for short or less).			See See	
vith sho roadway to road u direction	rt work zones on st vs where the flagge users approaching t is, a single flagger	raight r is visible from both may be used.	/	27		Butter
For low-	volume traffic situa	tions			S S AN	Johat.
	Flagger	vice		/		
55 60 - 65	750 1000	50 50			│ / <sup>⊗</sup> ∕∳◆◆◆	Roger
45 50	500	25 50				X
35 - 40	350	25				
(M.P.H.)	(A)	(G)				
Work	(Feet)	(Feet)	l á	n opposite d as below.	irection same	
Speed	Advance Warning	Channelizing	N.	Narning sigr	n sequence	

STATE OF	PROJECT	SHEET	TOTAL SHEETS	1
SOUTH DAKOTA	0009-271 & 0009-271	36	40	
Plotting	Date: 04/27/2023			
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![](_page_36_Figure_0.jpeg)

![](_page_36_Figure_1.jpeg)

![](_page_37_Figure_0.jpeg)

	WORK ZONE SPEED REDUCTION FOR INTERSTATE AND HIGH	plate number 634.63
	MODE ZORED DEDUCTION	EED T LANE DSED T LANE T L
PostedSpacing ofSpeedAdvance WarningPrior toSignsWork(Feet)(M.P.H.)(A) (B) (C)		

![](_page_37_Figure_5.jpeg)

![](_page_38_Figure_1.jpeg)

STATE OF	PROJECT	SHEET	TOTAL SHEETS
SOUTH DAKOTA	0009-271 & 0009-271	39	40
Plotting (	)ate: 04/27/2023		
-			

![](_page_39_Figure_1.jpeg)

![](_page_39_Figure_2.jpeg)